



OHIO VALLEY ELECTRIC CORPORATION

3932 U. S. Route 23
P. O. Box 468
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WRITER'S DIRECT DIAL NO:
740-289-7259

June 29, 2023

Delivered Electronically

Mr. Brian Rockensuess
Commissioner
Indiana Department of Environmental Management
100 N. Senate Avenue
Mail Code 50-01
Indianapolis, IN 46204-2251

**Re: Indiana-Kentucky Electric Corporation- Clifty Creek Station
Revision to the Landfill Runoff Collection Pond Closure Plan**

Dear Mr. Rockensuess:

As required by 40 CFR 257.106(i)(4), Indiana-Kentucky Electric Corporation is providing notification to the Commissioner of the Indiana Department of Environmental Management (IDEM) that the Closure Plan for Clifty Creek Station's Landfill Runoff Collection Pond has been revised. The newly revised plan will be placed in the facility's operation record as well as the publicly accessible internet site, which can be viewed at <http://www.ovec.com/CCRCompliance.php>

If you have any questions, or require any additional information, please call me at (740) 289-7259, or you can contact Tim Fulk at (740) 897-7768.

Sincerely,

A handwritten signature in black ink that reads "Jeremy Galloway". The signature is written in a cursive, flowing style.

Jeremy Galloway
Environmental Specialist

JDG: tlf



Stantec Consulting Services Inc.
9200 Shelbyville Road, Suite 800, Louisville KY 40222-5136

June 27, 2023

Project/File: 175531036

Mr. Gabriel Coriell
Indiana-Kentucky Electric Corporation
3932 U.S. Route 23
P.O. Box 468
Piketon, Ohio 45661

**Reference: Closure Plan
Clifty Creek Station
Landfill Runoff Collection Pond
Madison, Jefferson County, Indiana**

Dear Mr. Coriell,

The attached closure plan for Clifty Creek Station's Landfill Runoff Collection Pond (LRCP) was prepared by Stantec Consulting Services Inc. (Stantec) for the Indiana-Kentucky Electric Corporation (IKEC).

The initial closure and post-closure plans for the LRCP were posted on October 11, 2016 as part of the U.S. Environmental Protection Agency (EPA) final coal combustion residuals (CCR) rule demonstrations. The plans were conceptual and subject to the completion of all necessary environmental reviews. Though conceptual, they demonstrated compliance with the requirements set forth in 40 CFR 257.102(b) and 257.104(d).

IKEC, Stantec, and the Indiana Department of Environmental Management (IDEM) met in Indianapolis on December 9, 2019 to discuss the requirements for Indiana CCR surface impoundment closures. IKEC submitted the closure plan for the LRCP to IDEM on June 17, 2021. No comment has been received from IDEM on this proposed closure plan to date.

The plan was prepared in accordance with the accepted practice of engineering and accurate information at the date of its submittal to meet the requirements described in 40 CFR 257.102(b). Changes to the closure plan may be required. Revised plans will be posted at that time.

June 27, 2023
Mr. Gabriel Coriell
Page 2 of 2

Reference: **Closure Plan
Clifty Creek Station
Landfill Runoff Collection Pond
Madison, Jefferson County, Indiana**

Regards,

STANTEC CONSULTING SERVICES INC.



Matt Vaughan
Principal
*Licensed in KY, IN
Phone: (502) 212-5088
matt.vaughan@stantec.com

stantec.com

Attachment: Stantec Consulting Services Inc. (2021). Closure Plan. Clifty Creek Station. Landfill Runoff Collection Pond. Madison, Jefferson County, Indiana. Prepared for Indiana-Kentucky Electric Corporation. June 16.

ATTACHMENT

Closure Plan. Clifty Creek Station. Landfill Runoff Collection Pond.

Stantec Consulting Services Inc. (2021)



**OHIO VALLEY ELECTRIC CORPORATION
INDIANA-KENTUCKY ELECTRIC CORPORATION**

3932 U. S. Route 23
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WRITER'S DIRECT DIAL NO:
(740) 897-7768

June 17, 2021

Ms. Kate Garvey
Office of Land Quality
Indiana Department of Environmental Management
Solid Waste Permits Section
100 N. Senate Avenue
MC 65-45 IGCN 1101
Indianapolis, IN 46204-2205

Dear Ms. Garvey:

**Re: Indiana-Kentucky Electric Corporation
Clifty Creek Station Landfill Runoff Collection Pond Closure Plan**

In accordance with 329 IAC 10-3-1(9), the Indiana-Kentucky Electric Corporation (IKEC) is submitting for agency review the accompanying Closure Plan for Clifty Creek Station's Landfill Runoff Collection Pond (LRCP). IKEC is committed to being a good steward of the environment and to satisfying our environmental compliance obligations. We recognize the importance of maintaining a close partnership with IDEM in this endeavor, and appreciate the opportunity to submit this plan for agency review. Note that while IKEC desires to work closely with IDEM, IKEC is not waiving any of the positions it identified in its May 28, 2021 letter to Steven Thill or those identified in its June 1, 2021 Petition for Review of the Approval of the Partial Closure Plan for IKEC's WBSP.

If you have any questions or comments please contact me at (740) 897-7768.

Sincerely,

A handwritten signature in black ink that reads "Tim Fulk".

Tim Fulk
Engineer II

TLF:gsc

Attachments



Closure Plan

Clifty Creek Station

Landfill Runoff Collection Pond

Madison, Jefferson County,
Indiana

Prepared for:
Indiana-Kentucky Electric Corporation
3932 U.S. Route 23
Piketon, Ohio 45661

June 16, 2021

**Closure Plan
Landfill Runoff Collection Pond
Clifty Creek Station
Madison, Jefferson County, Indiana**

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Landfill Runoff Collection Pond
Clifty Creek Station
Madison, Jefferson County, Indiana**

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1. Objective

Indiana-Kentucky Electric Corporation (IKEC) is submitting this Closure Plan for the Clifty Creek Station's Landfill Runoff Collection Pond (LRCP) to the Indiana Department of Environmental Management (IDEM) Office of Water Quality (OWQ) with copies to the Office of Land Quality (OLQ), Waste Section. IKEC requests OWQ coordinate its review and comments with OLQ in a timely manner that facilitates adherence to the proposed schedule to close the LRCP at the Clifty Creek Station.

The LRCP is an inactive coal combustion residuals (CCR) surface impoundment. It currently receives stormwater runoff from more than 500 acres. Contact water from the adjacent landfill and stormwater run-on from the adjacent hillsides flow into the LRCP. Discharge from the LRCP is managed under the site's National Pollution Discharge Elimination System (NPDES) permit at the southwest corner of the LRCP. The applicable NPDES Permit No. is IN0001759. IKEC is preparing to cap and close the LRCP following the requirements in 329 Indiana Administrative Code (IAC) 10-9-1(9)(b) and (c) which incorporates portions of the United States Environmental Protection Agency (EPA) Final CCR Rule by reference.

The closure design includes two distinct phases of construction. Phase 1 involves the operational drawdown of the existing water surface elevation within the LRCP to a reduced footprint using the existing discharge structure and construction of a stormwater run-on diversion ditch around the perimeter of the LRCP to divert run-on from the surrounding hillsides directly to a new outfall to the Ohio River. These two steps in Phase 1 will remove water from the LRCP and promote drying of the CCR material in preparation for closure.

Phase 2 involves dewatering the CCR, grading the CCR to subgrade elevations, the installation of the final cover system, and modifications to the existing outfall structure.

In association with the closure of the LRCP, minor modifications will be necessary at the landfill to manage stormwater runoff and leachate flows. This will require the construction of two interim sediment basins and two leachate collection ponds, and appurtenances. Proposed modifications to the existing IDEM landfill permit will be submitted under a separate permit application. For the purposes of this Closure Plan, it is assumed these landfill improvements will be completed prior to commencing LRCP closure construction.

Appendix A is a list of acronyms and abbreviations.

2. Description of the CCR Unit

The Clifty Creek Station is located on the north bank of the Ohio River west of Madison, Indiana. It consists of six coal-fired electric generating units, each nominally rated at 217 megawatts. The station began producing electricity in 1955 to support the Department of Energy's (DOE's) Portsmouth Gaseous Diffusion Plant located near Piketon, Ohio. The LRCP is located southwest of the station, adjacent to the CCR landfill. Figure 1 shows the location of the LRCP and other key facilities at the site.

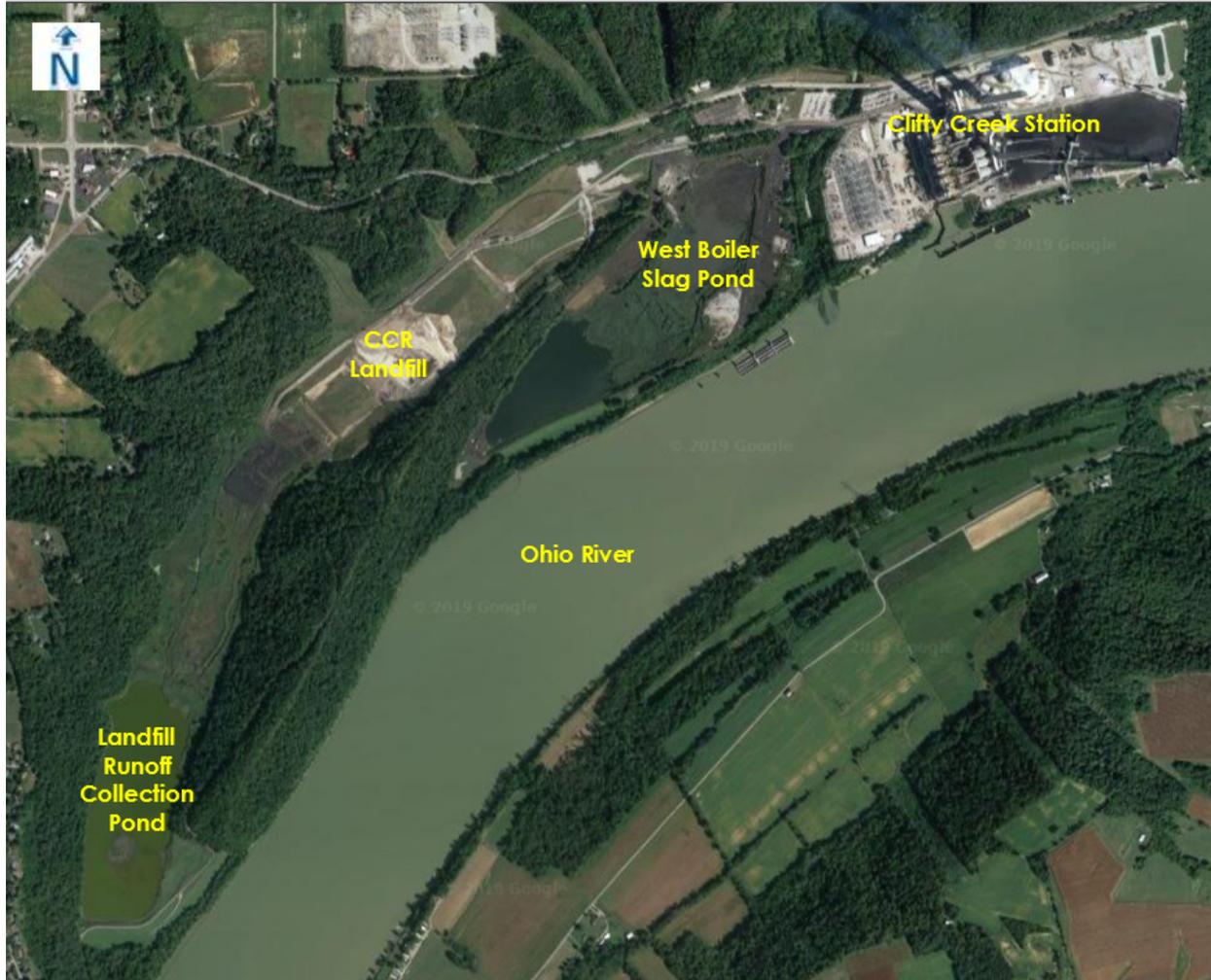


Figure 1- Aerial View of Clifty Creek Station

2.1. Impoundment Structure

The LRCP is surrounded by steep hillsides to the east and west, the CCR landfill to the north, and a constructed embankment to the south. The total closure area is approximately 57 acres, 38 of which is currently surface water. The current water surface elevation is approximately 486 feet. The crest of the south embankment is at approximate elevation 505 feet. FEMA (2015) Flood Insurance Study No. 18077CV000A

shows that the flood stages of the Ohio River at the LRCP are 463 feet and 468 feet for the 1 percent and 0.2 percent annual chance of flooding.

According to As-built drawings 16-3170-1, 16-3171-4, and 16-3171A-4, the crest of the dam is 26 feet wide. The upstream slopes are 2.5H:1V (horizontal slope : vertical slope). The downstream slopes are 2.5H:1V above elevation 478 feet and 3H:1V below this elevation. The exterior LRCP slopes are grass covered. As-built drawings are included in Appendix B.

2.2. Primary Spillway

Flows from the LRCP are currently permitted to be discharged through Outfall 001 to the Ohio River under NPDES Permit No. IN0001759 effective May 1, 2018, which is administered by IDEM. The LRCP's primary spillway consists of an intake structure in the pond connected by riser structures to a concrete pipe which runs under the LRCP dam and eventually to the Ohio River. The CCR pond intake structure consists of an inclined 6-foot x 3-foot reinforced concrete box culvert with intermittent riser box structures containing grated inlets located at every 11 feet in elevation. Currently, the two uppermost riser box structures are above the pond level and the lower riser structure is the active outlet for the pond. The inclined box is connected to a 400-foot long, 72-inch diameter concrete pipe that is located under the dam where outflow discharges into the Ohio River.

2.3. LRCP Location

The Clifty Creek Station is in Jefferson County, Madison Township, Indiana in Township 3N, Range 10E, Section 5. A legal description of the facility boundary is included in Appendix C. Located south of Indiana State Route 56 and northwest of a bedrock geologic feature called the Devil's Backbone, the CCR Landfill is being constructed in two phases from northeast to southwest, down the valley. Water from the landfill and stormwater run-on from the adjacent hillsides flows southwest to the LRCP. Discharge from the LRCP is managed under the site's NPDES permit at the southwest corner of the site.

In Appendix D, Figures 2 and 3 reflect a half-mile offset from the LRCP's waste boundary with regional water wells identified in the Indiana Department of Natural Resources (IDNR), Division of Water Well Record Database (IDNR, 2019). Figure 2 shows a plan view of the LRCP overlain on a November 2017 ESRI aerial. Figure 3 reflects the same data shown on a portion of the 7½-minute USGS topographic quadrangle map for Madison West (2019).

Four wells are shown within the half-mile offset. Available IDNR well information is provided in Appendix D. All four wells are owned by nearby Hanover College. Table 1 details the four wells within the half-mile offset from the LRCP.

Table 1 - Water Wells Within a Half-Mile Offset

Well Record Reference No.	Type	Depth (feet)	Completion Date	Location	Status
219384	Unconsolidated	68	06/30/1967	Madison–West, Jefferson County, IN	Inactive
219399	Unconsolidated	69	10/11/1948	Madison–West, Jefferson County, IN	Inactive
312919	Unknown	--	--	Madison–West, Jefferson County, IN	Inactive
312920	Unconsolidated	80	01/01/1940	Madison–West, Jefferson County, IN	Inactive

2.4. Available Geotechnical Data

Geotechnical data is available from six field explorations at the LRCP. A plan view of the borings and logs are provided in Appendix E. Appendix F contains a slope stability analysis for the LRCP closure.

American Gas & Electric Service Corp. (1953)

The 1956 as-built design drawings include geotechnical profiles within the LRCP dam. The drawings show the embankment dimensions and generalized original ground topography.

American Gas & Electric Service Corp. (1985)

An exploration program was carried out in 1984 to obtain geotechnical properties of the LRCP dam to evaluate the feasibility of raising it by 30 feet to elevation 535 feet. Thirty-seven borings and nine CPT soundings were advanced along the dam crest, downstream toe and pond of the LRCP facility. Results from the exploration indicate that the dam was constructed in compacted layers of silty and sandy clays. A 30 to 40-ft thick seam of gray clay was encountered underlying the main dam and the bedrock underlying foundation soils was described as Indiana Limestone with layers of calcareous shale.

Stantec (2016)

Stantec performed two geotechnical field explorations to support the safety factor demonstration under the CCR Rule. Four borings were advanced along the crest and the downstream toe of the LRCP Dam in 2009/2010 and one borehole was advanced in 2015 to confirm field conditions. Laboratory testing was performed to confirm field classifications (natural moisture content, hydrometer analyses, Atterberg limits), estimate shear strength (consolidated-undrained triaxial compression testing), and permeability. Results from the explorations indicate that the dam was constructed of lean and silty clay with sand. Silt with sand and silty sand soils were encountered in the foundation.

AGES (2016)

Applied Geology and Environmental Science (AGES), Inc. was contracted by IKEC to identify upgrades in the groundwater monitoring program of the LRCP necessary for compliance with the CCR Rule. In 2015, three monitoring wells were installed using a sonic drill rig. Excerpts from AGES (2018) are included in Appendix E. This includes a well summary table, a generalized geologic cross section, groundwater flow maps for four sampling periods, and sample/well construction logs.

Stantec (2021)

Stantec conducted a field exploration in 2021 to support design and construction of the LRCP closure. Six geotechnical borings were advanced to a depth ranging from 15 to 40 feet below existing grade. The logs described the material as boiler slag (coarse sand); lean and silty clay, silty sand, gravel with sand, or shale bedrock. Laboratory testing of samples collected during this exploration was ongoing at the time of this report. A plan view and boring logs for this exploration are included in Appendix E.

3. Regulatory Overview

3.1. Regulatory Framework for Design

The United States EPA Final CCR Rule defines the criteria for conducting the closure of CCR units under 40 CFR 257.102.

Per the IDEM CCR Fact Sheet, Indiana coal ash surface impoundments that are subject to an NPDES permit are not regulated under IDEM's solid waste program (IDEM 2021). Once the NPDES permit is terminated, the final disposal of solid waste in the surface impoundment is subject to the closure requirements under 329 Indiana Administrative Code (IAC) 10-9-1(9)(b) and (c), which incorporates portions of the CCR Rule by reference.

This submittal is an amendment to a written closure plan (40 CFR 257.102(b)(3)) describing closure in place (40 CFR 257.102(d)) for the LRCP.

Below is a general summary of how the LRCP will be closed. The permit-level drawings are included in Appendix G. Ditch sizing calculations associated with the permit-level design are included in Appendix H.

3.2. Description of Closure Plan - 257.102(b)(1)(i)

[A narrative description of how the CCR unit will be closed in accordance with this section.]

The intent is to consolidate within the LRCP where possible, grade stored CCR, and close the facility in place. The closure will consist of dewatering the unit through an NPDES-permitted outfall, grading, and construction of an engineered cap.

IDEM's OLQ has requested that the LRCP be closed in accordance with Type I restricted waste site (RWS) standards. Under 329 IAC 10-30-2, final cover must have:

- A maximum projected erosion rate of five tons per acre per year
- A final compacted cover of six inches of topsoil plus a minimum depth of compacted clay of 30 inches:
- Slopes not less than two percent nor greater than 33 percent.

Appendix I includes the final cover soil loss calculations. The final cover consists of a 30-inch cover material layer and six inches of earthen material capable of growing and sustaining native vegetative growth. A geosynthetic membrane liner and geocomposite drainage layer is included below the cover material. The capped surface will be graded to promote surface water runoff, and then seeded and mulched to promote growth of the vegetative cover.

3.3. Closure in Place - 257.102(b)(1)(iii)

[If closure of the CCR unit will be accomplished by leaving the CCR in place, a description of the final cover system, designed in accordance with paragraph (d) of this section, and the methods and procedures to be used to install the final cover.]

Prior to installing the final cover system, the CCR unit will be drained of free water and the material within the unit will be stabilized and graded to provide a stable and suitable subgrade upon which to construct the cap. All water will be managed in the remaining open portion of the pond and discharged through the existing NPDES outfall. The final closure system will consist of a flexible geomembrane liner (FML), that will have a permeability that is less than or equal to the permeability of the natural subsoils, and is no greater than 1×10^{-5} cm/sec. FML will be installed directly over the graded CCR material followed by a layer of geocomposite drainage media, 2.5-foot thick cover soil, and 6-inches of earthen material capable of growing and sustaining native vegetative growth. The capped surface will be graded to promote surface water runoff, and then seeded and mulched to promote growth of the vegetative cover.

Piezometers will be installed to monitor water levels within the closed footprint. Wellpoints and sump pumps may be added as needed to drawdown the near surface water to support construction of the proposed final cover system. All pumped water will be returned to the remaining open portion of the pond to be discharged through the existing NPDES outfall.

Stormwater drainage improvements will be implemented during the final closure activities with minor grading of existing channels and construction of new channels to improve drainage of the closed pond. The final cover slope will be a minimum of two percent (2%) and will convey surface water to an NPDES-permitted outfall. Permanent stormwater ditch slopes may vary and will be sized to adequately convey anticipated design storm events.

3.4. Closure Performance Standards - 257.102(d)(1)

3.4.1. Section 257.102(d)(1)(i),(ii),(iii)

[(i) Control, minimize or eliminate, to the maximum extent feasible, post-closure infiltration of liquids into the waste and releases of CCR, leachate, or contaminated run-off to the ground or surface waters or to the atmosphere; (ii) Preclude the probability of future impoundment of water, sediment, or slurry; (iii) Include measures that provide for major slope stability to prevent the sloughing or movement of the final cover system during the closure and post-closure care period]

Post-closure infiltration of liquids into the waste will be controlled through the design of the site grading plan, construction of an engineered cap system, and establishment of a stormwater management system in accordance with engineering practices. The intent of such a plan is to limit the infiltration of precipitation, cover, control, and prevent the releases of CCRs, and promote positive drainage. CCR materials will be placed and compacted in a manner to minimize settling and subsidence that could affect the integrity of the final cover system prior to cap placement.

Installation and quality control testing of the geosynthetics will be performed as specified by the manufacturer.

Stability analyses were performed as part of the EPA Final CCR Rule's design criteria demonstrations (Stantec, 2016). Stability analyses can be found in Appendix F. Additional analyses have been performed to support the proposed closure configuration and improvements to the LRCP dam.

3.4.2. Section 257.102(d)(1)(iv)

[Minimize the need for further maintenance of the CCR unit.]

The impoundment will be vegetated to prevent erosion. Maintenance of the final cover system will include regularly scheduled inspections to monitor post-closure conditions and preventative maintenance.

3.4.3. Section 257.102(d)(1)(v)

[Be completed in the shortest amount of time consistent with recognized and generally accepted good engineering practices.]

The impoundment will be closed in a time frame consistent with recognized and generally accepted good engineering practices. Refer to the schedule below for key milestone dates.

3.5. Draining and Stabilizing the Surface Impoundment

[The owner or operator of a CCR surface impoundment or any lateral expansion of a CCR surface impoundment must meet the requirements of paragraph (d)(2)(i) and (ii) of this section prior to installing the final cover system required under paragraph (d)(3) of this section.]

3.5.1. Section 257.102(d)(2)(i)

[Free liquids must be eliminated by removing liquid wastes or solidifying the remaining wastes and waste residue.]

Free liquid will be removed as part of the final closure of the CCR unit and discharged in a manner consistent with the facility's NPDES permit.

3.5.2. Section 257.102(d)(2)(ii)

[Remaining waste must be stabilized sufficient to support the final cover system.]

The remaining wastes that constitute the subgrade of the final cover system will be stabilized by removal of free liquids and providing bridging material as necessary.

3.6. Final Cover System - 257.102(d)(3)

[If a CCR unit is closed by leaving the CCR in place, the owner or operator must install a final cover system that is designed to minimize infiltration and erosion, and at a minimum, meets the requirements of paragraph (d)(3)(i) of this section, or the requirements of the alternative final cover system specified in paragraph (d)(3)(ii) of this section.

The final cover system must be designed and constructed to meet the criteria in paragraphs (d)(3)(i)(A) through (D) of this section. The design of the final cover system must be included in the written closure plan.]

The final closure system details are included above in Sections 3.2 and 3.3

3.7. Estimate of Maximum CCR Volume - 257.102(b)(1)(iv)

[An estimate of the maximum inventory of CCR ever on-site over the active life of the CCR unit.]

The LRCP is an inactive facility that no longer receives CCR material. The estimated maximum amount of CCR to ever be on-site for the LRCP is approximately 1,900 acre-feet.

3.8. Estimate of Largest Area of CCR Requiring Cover - 257.102(b)(1)(v)

[An estimate of the largest area of CCR unit ever requiring a final cover.]

The area of the proposed final cover system is approximately 57 acres.

3.9. Closure Schedule - 257.102(b)(1)(vi)

[A schedule for collecting all activities necessary to satisfy the closure criteria in the section, including an estimate of the year in which all closure activities for the CCR unit will be completed. The schedule should provide sufficient information to describe the steps that will be taken to close the CCR unit, including identification of major milestones such as coordinating with and obtaining necessary approvals and permits from other agencies, the dewatering and stabilization of the CCR surface impoundment closure, or installation of the final cover system, and the estimated timeframes to complete each step or phase of the CCR unit closure.]

LRCP closure activities will begin upon approval of this permit application. The intent is to complete construction of the cap .

USEPA (2020) has defined a closure schedule process for existing CCR surface impoundments that are considered “unlined” under the CCR Rule. Table 2 provides an approximate closure schedule to meet the required regulation.

Table 2 - Proposed Closure Schedule

Task	Completion Date
Phase 1 construction	End of 2023
Phase 2 construction	Within 5 years of cessation of flows or when technically feasible

4. General Considerations

General considerations for the LRCP closure are presented in the following sections. Subsequent to final closure, IKEC will address environmental concerns and permit obligations that are regulated by other IDEM divisions during the closure process. All demonstrations reflecting the LRCP's compliance with the EPA Final CCR Rule in terms of location restrictions, design criteria, operating criteria, and groundwater monitoring are available on IKEC's public website, www.ovec.com/CCRClifty.php.

The closure design includes two distinct phases of construction. Phase 1 involves the operational drawdown of the existing water surface elevation within the LRCP to a reduced footprint using the existing discharge structure, and construction of a stormwater run-on diversion ditch around the perimeter of the LRCP to divert run-on from the surrounding hillsides directly to a new outfall to the Ohio River. These two steps in Phase 1 will remove water from the LRCP and promote drying of the CCR material in preparation for closure.

Phase 2 involves dewatering the CCR, grading the CCR to subgrade elevations, the installation of the final cover system, and modifications to the existing outfall structure.

In association with the closure of the LRCP, minor modifications will be necessary at the landfill to manage stormwater runoff and leachate flows. This will require the construction of two sediment basins and two leachate collection ponds, and appurtenances. Proposed minor modifications to the existing IDEM landfill permit will be submitted under a separate permit application. For the purposes of this Closure Plan, it is assumed these landfill improvements will be completed prior to commencing LRCP closure construction.

5. Closure Plan Scope of Work

The closure design includes two distinct phases of construction. The following general tasks are anticipated as part of the closure process.

5.1. Phase 1 – Drawdown and Ditch Diversion

Specific activities associated with Phase 1 of the LRCP closure plan include:

- Operational drawdown of the existing water surface elevation within the LRCP to a reduced footprint using the existing discharge structure.
- Construction of a stormwater run-on diversion ditch around the perimeter of the LRCP to divert landfill runoff and run-on from the surrounding hillsides directly to a new outfall to the Ohio River.
- Notification of completion of Phase 1 closure to the IDEM OWQ and OLQ.
- Once Phase 1 construction is complete, the only inflows to the LRCP will be direct precipitation and stormwater run-on. All CCR waste streams will cease no later than 2023.

5.2. Phase 2 – Dewatering and Cover System

Specific activities associated with Phase 2 of the LRCP closure plan include:

- Continued dewatering of the CCR in the LRCP's reduced footprint.
- Re-grading the CCR to design subgrade elevations.
- Construct piezometers to monitor water levels.
- Construct the engineered cap system.
- Establish the access road and vegetation on the site.
- Notification of completion of Phase 2 final closure to the IDEM OWQ and OLQ.

5.3. Cap System

An engineered cap system will be constructed over the LRCP closure area. Appendix G provides the proposed permit drawings, including the cap area and details.

The engineered cap will consist of the following materials, listed in order of construction (from bottom to top):

- 40-mil geomembrane
- Geocomposite drainage layer
- 36 inches of cover soil, of which the top 6 inches are capable of supporting vegetation

The final cap system design will accommodate settling and subsidence so to preserve the cap system's integrity.

5.4. Stormwater Construction Permit

Since more than one acre will be disturbed during the pond closure activities, a Stormwater Notice of Intent (NOI) to discharge stormwater associated with construction activities will be submitted to IDEM OWQ. A Stormwater Pollution Prevention Plan (SWPPP) will be prepared and submitted as required by the NOI along with applicable permit fees. A Notice of Termination (NOT) to terminate the stormwater construction permit will be submitted upon completion of the pond closure.

5.5. Construction Quality Assurance

Construction quality assurance (CQA) plan activities will be finalized as part of the detailed design and prior to construction of each phase of the LRCP closure. Construction observations will be conducted and recorded to document the closure and CQA testing. CQA activities are included in the project Quality Management Plan (QMP). Sections of the QMP will include:

- A. Purpose and Scope
- B. Responsibility and Authority
- C. Quality Control Activities
- D. Quality Assurance Activities
- E. Material Testing
- F. Project Documentation

Appendix J includes a draft QMP. This is proposed as the basis for the final plan to maintain consistency on the site.

5.6. Closure Documentation

Upon completion of approved closure construction activities, a closure report will be prepared by an independent professional engineer registered in the State of Indiana to document the completed construction activities. The closure report will be submitted to IDEM OLQ and OWQ. The letter report will document the source of fill material, amount of fill material used, details regarding cap construction, and final cap elevations.

6. Post-Closure Plan

Post-closure care will be performed in accordance with the Post-closure Plan included as Appendix K.

Estimated Closure and Post-Closure costs are provided in Appendix L.

7. References

American Electric Power Service Corporation. (2016). *History of Construction*. CFR 257.73(c)(1). *West Boiler Slag Pond. Clifty Creek Plant. Madison, Indiana.* October. Prepared for Indiana-Kentucky Electric Corporation. GERS-16-142. Columbus, Ohio.

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APPENDIX A

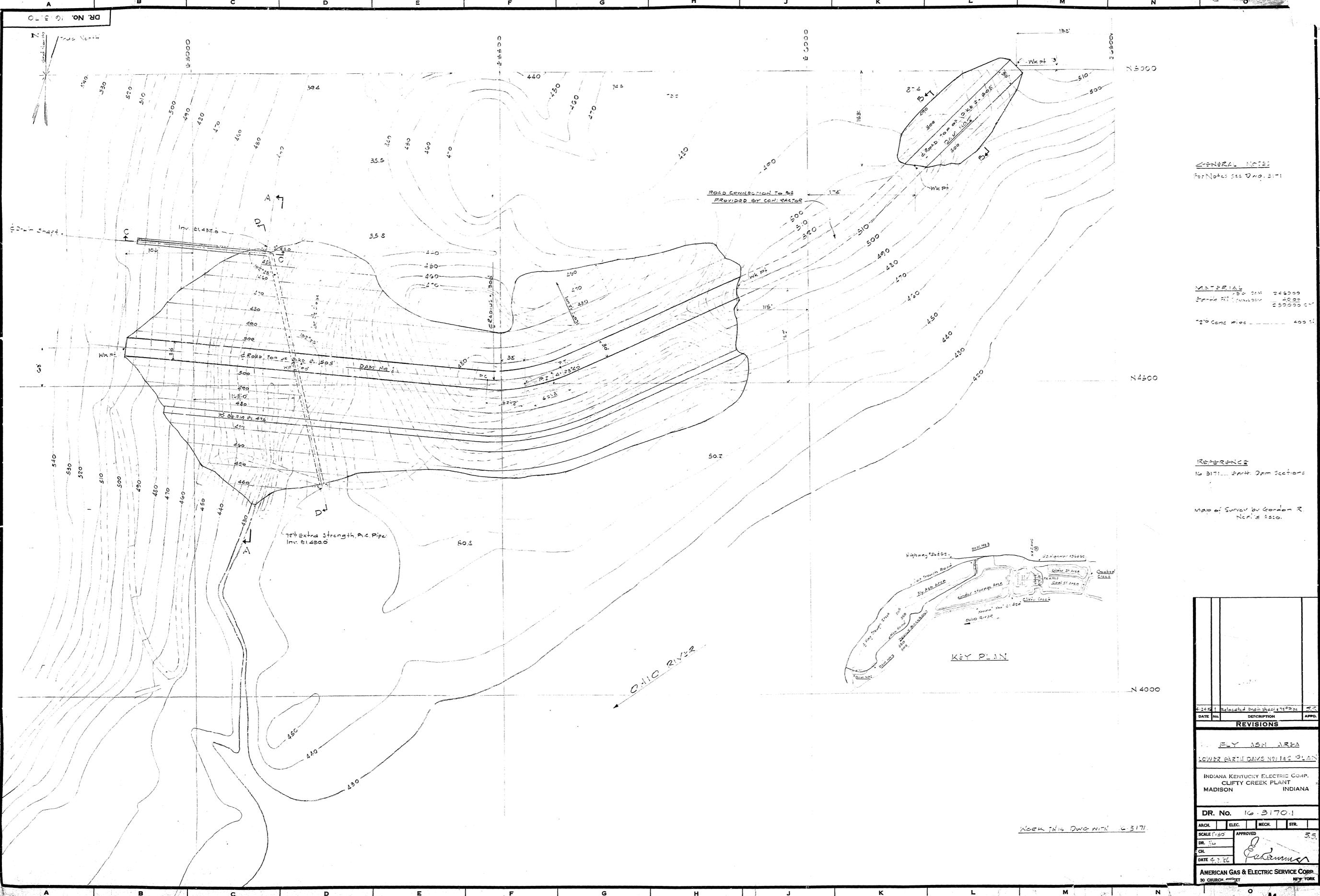
Acronyms and
Abbreviations

Acronyms and Abbreviations

AEPSC	American Electric Power Service Corporation
AGES	Applied Geology and Environmental Science, Inc.
CCR	Coal Combustion Residuals
CFR	Code of Federal Regulations
cm/sec	centimeters per second
CQA	Construction Quality Assurance
DOE	Department of Energy
ELG	Effluent Limitations, Guidelines
FML	flexible membrane liner
H:V	horizontal slope : vertical slope
IAC	Indiana Administrative Code
IDEM	Indiana Department of Environmental Management
IDNR	Indiana Department of Natural Resources
IKEC	Indiana-Kentucky Electric Corporation
LRCP	Landfill Runoff Collection Pond
mW	megawatts
No.	number
NOI	Notice of Intent
NOT	Notice of Termination
NPDES	National Pollutant Discharge Elimination System
OLQ	Office of Land Quality
OWQ	Office of Water Quality
QMP	Quality Management Plan
SWPPP	Stormwater Pollution Prevention Plan
UMA	uppermost aquifer
USEPA	United States Environmental Protection Agency
WBSP	West Boiler Slag Pond

APPENDIX B

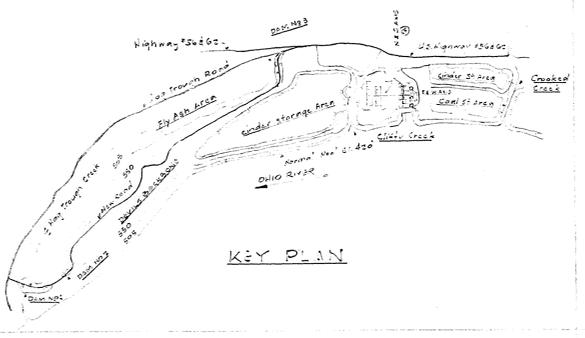
As-Built Design Drawings



GENERAL NOTES
 For Notes see Dwg. 3171

MATERIAL
 250' Dam 240000
 Earth Fill 100000
 72" Conc. Pipe 40000

REFERENCE
 16.3171 Earth Dam sections
 Map of Survey by Gordon R. Neri, # 1510.



KEY PLAN

DATE	NO.	DESCRIPTION	APPD.

REVISIONS

ELY ASH AREA
 LOWER EARTH DAM NO. 1 & 2 PLAN

INDIANA KENTUCKY ELECTRIC CO. P.
 CLIFTY CREEK PLANT
 MADISON INDIANA

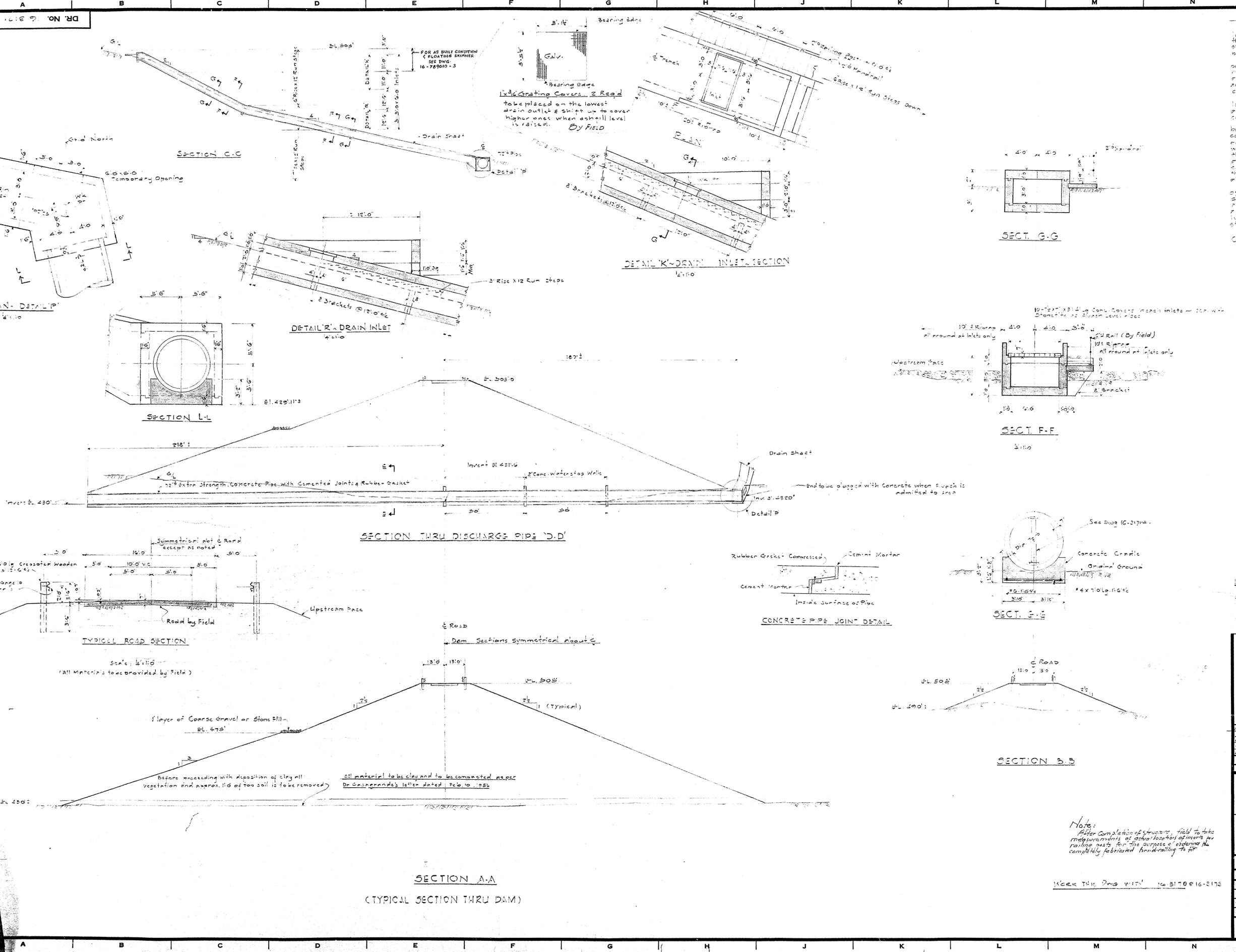
DR. No. 16.3170.1

ARCH.	ELEC.	MCH.	STR.
-------	-------	------	------

SCALE 1"=50'
 DR. No. 16.3170.1
 CH.
 DATE 4.1.36

APPROVED: *[Signature]* 35

Work this Dwg with 16.3171



GENERAL NOTES

Clay should be compacted in layers of 3 in. compacted thickness using a roller with a smooth surface exerting a foot pressure of at least 200 lbs. per sq. ft. This rolling should be done out at least 24 hours before road is to be opened to traffic. An indication of the water content of the clay should be made. Whenever this is not done, the air drive reduces the water content to a satisfactory degree. The surface of soil which could be crumpled such that no water can drain readily toward the outside slope. In addition, whenever a heavy rain thunders the surface should be rolled with a smooth roller to break the surface runoff to reduce the amount of water which will be absorbed by the clay.

A grass cover of any kind of vegetative stable surface should be provided on the slope immediately after construction and all erosion channels completely repaired until a satisfactory state of growth has developed. See further information in the Geographical letter of Feb. 10, 1935.

Reinforcing Drainchute DWG-3173
Reinforcing Schedule RS-1 3/4" x 4"

References & Notes not shown
See DWG 16-3170

Reinforcing of drain pipe
See DWG 16-3171A

DATE	NO.	DESCRIPTION	APP'D.
7/16/35	1	Added gratings at shown	S.B.
7/16/35	2	Minor revisions	J.S.
7/16/35	3	Stein curb omitted	J.S.
7/16/35	4	Revised wall thickness in Detail Section	J.S.
7/16/35	5	Revised drain grate 4 1/2" x 12"	J.S.

REVISIONS

FLY ASH AREA

LOWER EARTH DAMS SECTIONS & DETAILS

INDIANA KENTUCKY ELECTRIC CORP.
CLIFTY CREEK PLANT
MADISON INDIANA

DR. NO. 16-3171-B

ARCH.	ELEC.	MECH.	STR.
-------	-------	-------	------

SCALE: 1/4" = 1'-0"

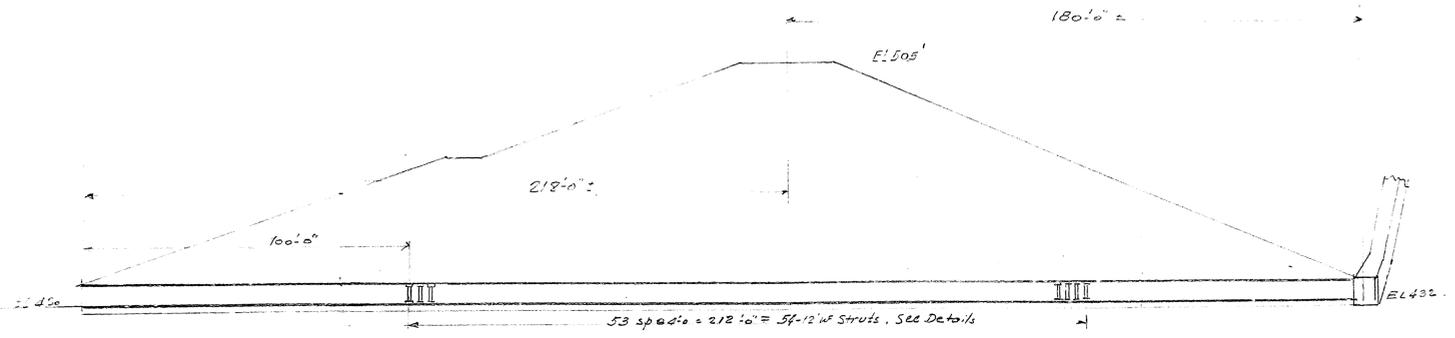
APPROVED: *[Signature]*

DATE: 2-25-36

AMERICAN GAS & ELECTRIC SERVICE CORP.
30 CHURCH STREET
NEW YORK

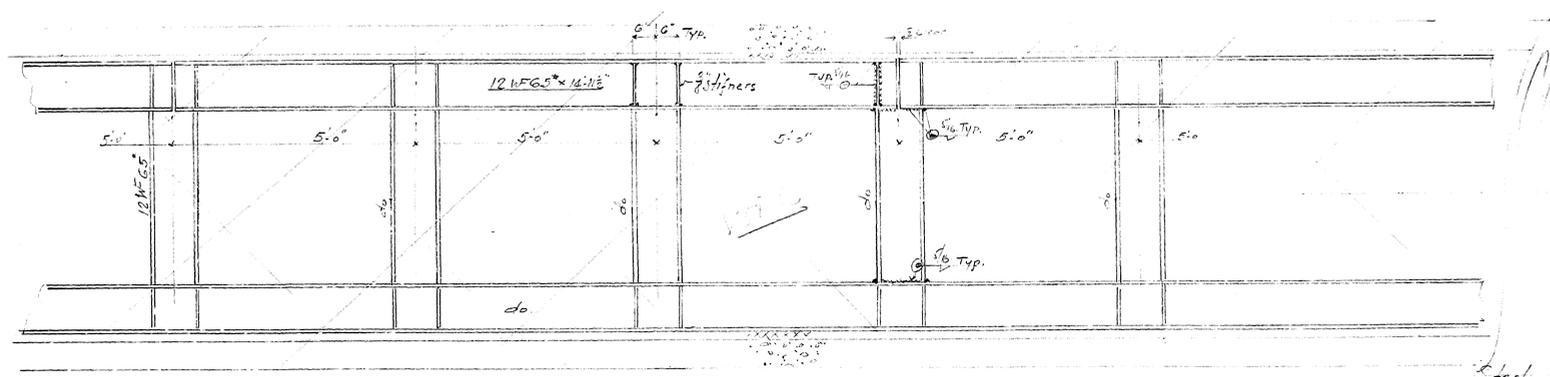
Note:
After completion of structure, field to the measurements of actual location of centers for railing posts for the purpose of explaining the completely fabricated hand-railing to fit.

KEEP THIS DWG WITH 16-3170 & 16-3173

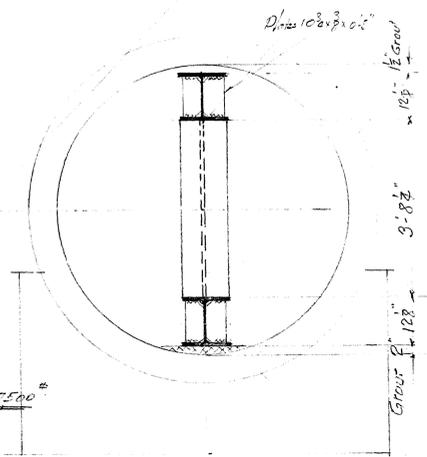


SECTION THRU DAM AT 6'-0" DRAIN
SCALE 1" = 40'-0"

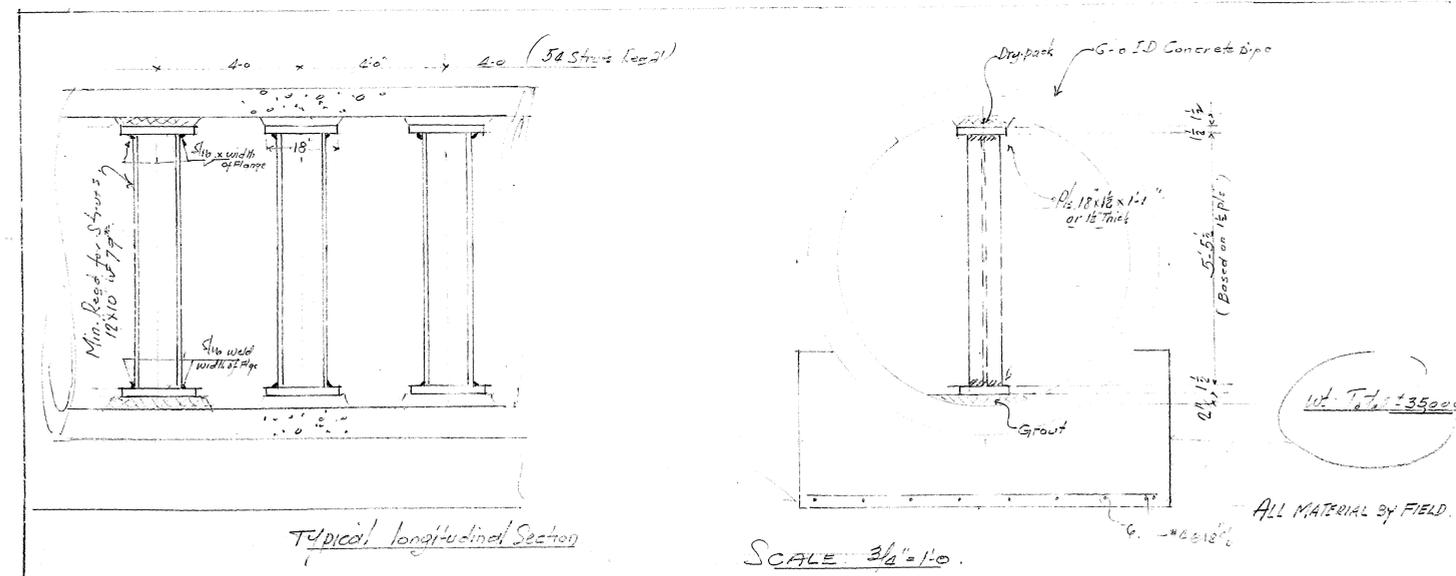
STEPS TO BE GIVEN ONE (1) COAT OF BITUMASTIC PAINT AND BE LEFT PERMANENTLY IN PLACE.



SCALE 2 1/2" = 1'-0"



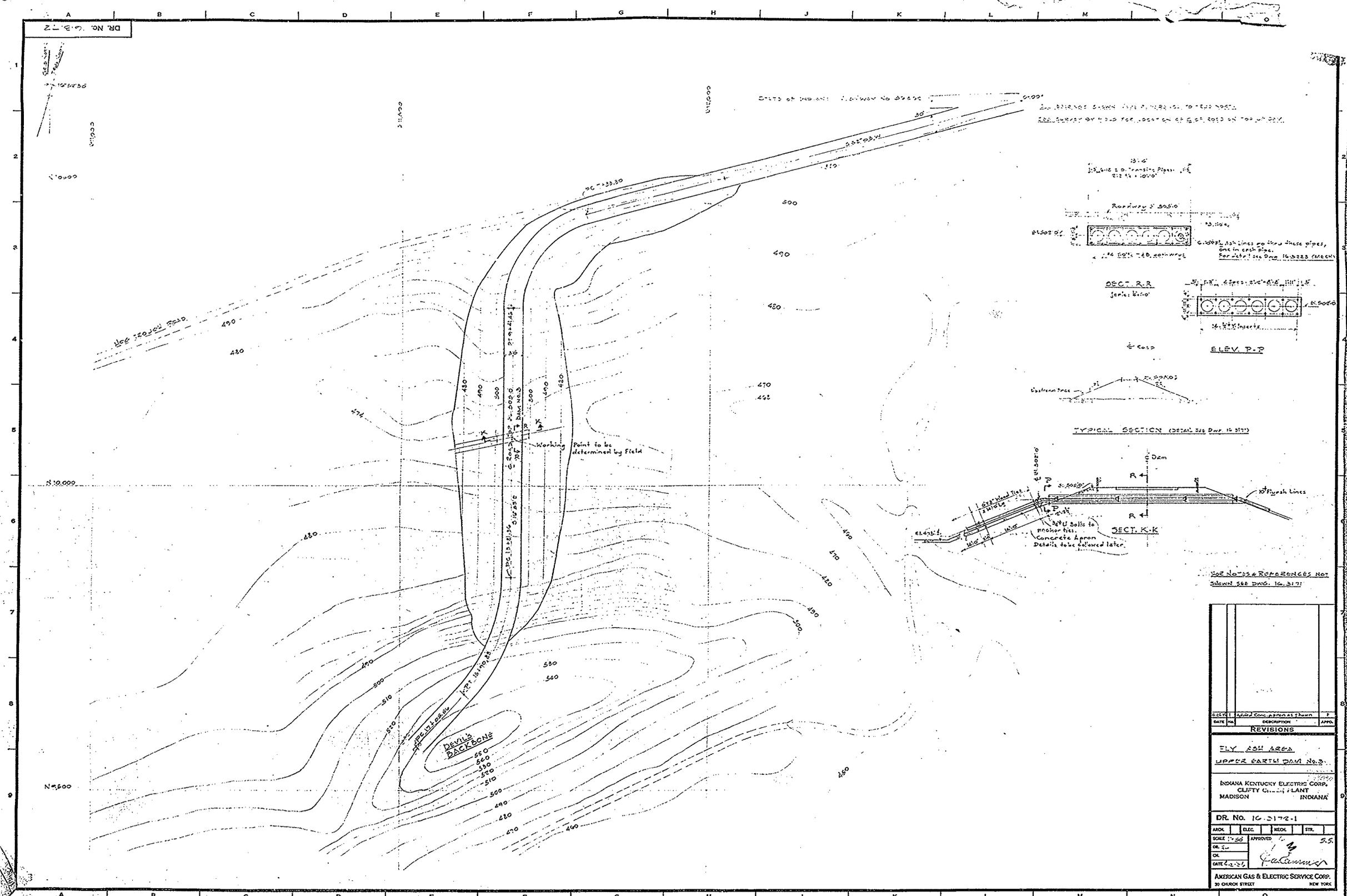
Scale detail of drain shaft and 6'-0" ID drain see DR. 16-3171



Typical longitudinal Section

SCALE 3/4" = 1'-0"

DATE	NO.	DESCRIPTION	APPROVED
REVISIONS			
FLY ASH AREA LOWER EARTH DAM 6'-0" DRAIN REINFORCING			
INDIANA KENTUCKY ELECTRIC CORP. CLIFFY CREEK PLANT MADISON INDIANA			
DR. No. 16-3171A.1			
ARCH.	ELEC.	MECH.	STR.
SCALE: Note APPROVED M			
CHK.			
DATE: 6/28/58			
AMERICAN GAS & ELECTRIC SERVICE CORP. 30 CHURCH STREET NEW YORK			



See Notes & References Not Shown See DWG. 10-2172-1

REV. NO.	DATE	DESCRIPTION	BY	APP'D.
REVISIONS				
FLY ASH AREA				
UPPER EARTH DAM No. 3				
INDIANA KENTUCKY ELECTRIC CORP. CLIFTY CREEK PLANT MADISON INDIANA				
DR. NO. 10-2172-1				
ARCH.	ELEC.	MECH.	STR.	
SCALE 1"=50'		APPROVED		5.5
DATE 4-2-51				
AMERICAN GAS & ELECTRIC SERVICE CORP. 30 DELORCH STREET NEW YORK				

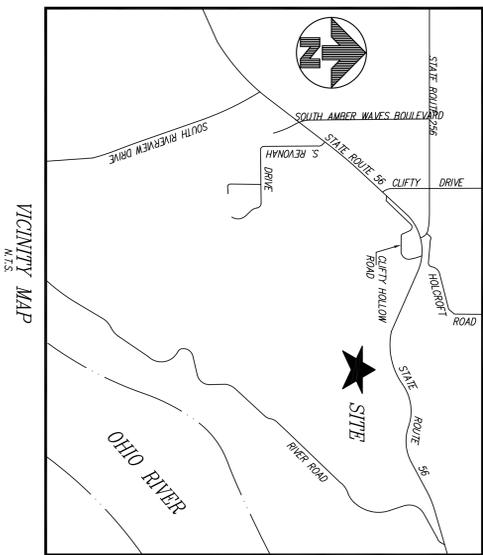
APPENDIX C

Boundary Survey

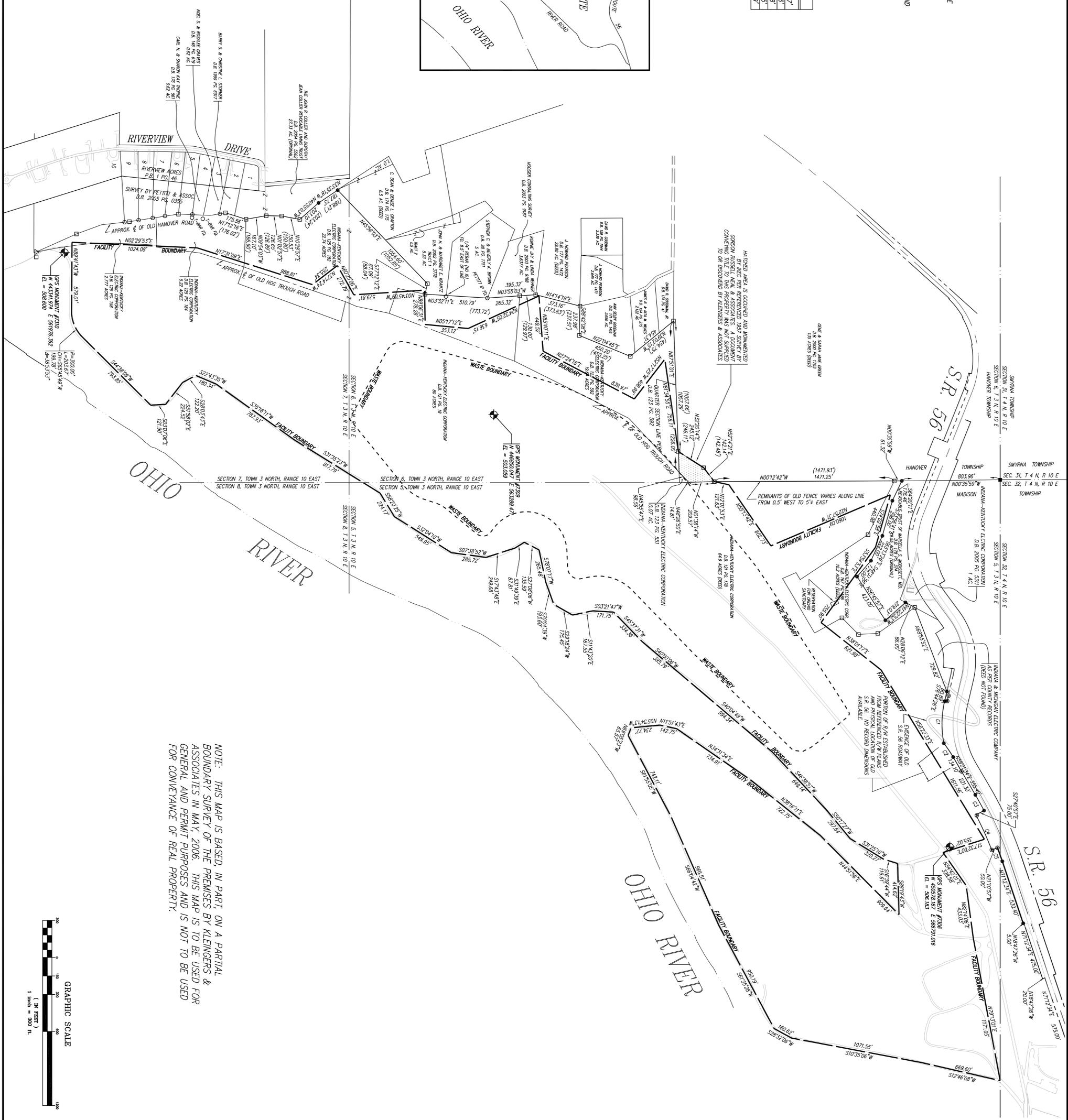


- LEGEND**
- 5/8" REBAR SET
 - 5/8" REBAR FOUND UNLESS NOTED OTHERWISE
 - CONCRETE P/W MARKER FOUND
 - RAILROAD SPIKE FOUND
 - CONCRETE MONUMENT WITH BRASS PLUG FOUND (SET PER 1997 NEAL SURVEY)
 - STONE FOUND

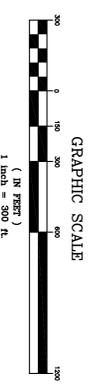
CURVE	LENGTH	RADIUS	DELTA	CHORD
C1	343.16'	806.20'	242°31'6"	588.9504'E
C2	133.55'	1034.46'	2°31'53"	N53°40'57"E
C3	146.10'	2824.79'	2°31'29"	N60°50'19"E
C4	512.52'	2754.79'	6°30'00"	N65°34'03"E
C5	117.09'	2804.79'	2°23'51"	N70°00'49"E



- GENERAL NOTES:**
- 1) PHYSICAL FEATURES, DRIERS, BUILDINGS, WATER COURSES, ETC. ARE BASED ON A COMBINATION OF FIELD LOCATIONS AND AERIAL PHOTOGRAPHIC MAPPING SUPPLIED BY THE CLIENT.
 - 2) SECTION LINES ARE SHOWN FOR REFERENCE ONLY. SURVEY.
 - 3) BEARINGS ARE BASED ON THE INDIANA STATE PLANE COORDINATE SYSTEM MAD 27, AS PER COORDINATES OF THE PLANT MONUMENTS SHOWN HEREON.
 - 4) INTERIOR PLANT PARCEL LINES NOT SHOWN IN FULL.
 - 5) REFERENCES USED INCLUDE INDIANA STATE HIGHWAY COMMISSION, F.A. PROJECT NO. 212 SEC. C PAVING PLANS FOR S.R. 56, DATED AUGUST 1930 AND ADDITIONAL IMPROVEMENT PLANS DATED 1937.
 - 6) ADDITIONAL REFERENCE DATA INCLUDES A REVISED BOUNDARY SURVEY - CLIFTY CREEK POWER PLANT SITE AS PERFORMED BY GORDON RUSSELL NEAL & ASSOCIATES DATED 11-1-1997. EASEMENTS OF RECORD NOT SHOWN.



NOTE: THIS MAP IS BASED, IN PART, ON A PARTIAL BOUNDARY SURVEY OF THE PREMISES BY KLEINGERS & ASSOCIATES IN MAY, 2006. THIS MAP IS TO BE USED FOR GENERAL AND PERMIT PURPOSES AND IS NOT TO BE USED FOR CONVEYANCE OF REAL PROPERTY.



FACILITY BOUNDARY / WASTE BOUNDARY MAP
 INDICATING A PORTION OF THE LANDS OF
INDIANA - KENTUCKY ELECTRIC CORPORATION
 SECTIONS 5, 6 & 7, TOWN 3 NORTH, RANGE 10 EAST
 SECTION 32, TOWN 4 NORTH, RANGE 10 EAST
 MADISON & HANOVER TOWNSHIPS JEFFERSON COUNTY, INDIANA

NO.	DATE	DESCRIPTION

KLEINGERS & ASSOCIATES
 ENGINEERS AND SURVEYORS
 6305 Centre Park Drive, West Chester, OH 45069
 (513) 779-7851 Fax: (513) 779-7852



SCALE: 1"=300'
 DATE: 11/22/06
 DRAWN: DLG
 DESIGNED: DLG
 CHECKED: DLG
 KREB: JMB
 JOB NO.: 060192VB
 IN-KY ELECTRIC CORP.
 Page 1 of 1



November 22, 2006

**LEGAL DESCRIPTION
INDIANA – KENTUCKY ELECTRIC CORPORATION
FACILITY BOUNDARY
357.74 ACRES**

Situated in Section 5, Town 3 North, Range 10 East, Madison Township and in Sections 6 and 7, Town 3 North, Range 10 East, Hanover Township, Jefferson County, Indiana and being part of the lands conveyed to Indiana-Kentucky Electric Corporation and being more particularly described as follows:

Commencing at a stone found at the northwest corner of said Section 5 and the northeast corner of said Section 6;

Thence along the westerly line of Section 5 and the easterly line of Section 6, also being the line between Madison and Hanover Townships, $S00^{\circ}35'59''E$ a distance of 803.96 feet;

Thence continuing along said section line, $S00^{\circ}12'42''E$ a distance of 1471.25 feet to a concrete monument found at the true Point of Beginning;

Thence along the lines or through the lands of the Indiana-Kentucky Electric Corporation the following sixty five (65) courses:

- 1) $N12^{\circ}01'53''E$ a distance of 121.63 feet;
- 2) $N55^{\circ}13'42''E$ a distance of 602.73 feet;
- 3) $N22^{\circ}57'31''W$ a distance of 1060.09 feet;
- 4) $S74^{\circ}02'58''E$ a distance of 440.98 feet;
- 5) $S53^{\circ}54'53''E$ a distance of 755.90 feet;
- 6) $N38^{\circ}01'17''E$ a distance of 621.98 feet;
- 7) $N58^{\circ}22'33''E$ a distance of 1611.56 feet;
- 8) $S17^{\circ}32'00''E$ a distance of 355.02 feet;
- 9) $N54^{\circ}42'01''E$ a distance of 328.58 feet;
- 10) $N82^{\circ}14'06''E$ a distance of 433.03 feet;
- 11) $N79^{\circ}13'01''E$ a distance of 1171.05 feet;
- 12) $S12^{\circ}46'08''W$ a distance of 669.60 feet;
- 13) $S10^{\circ}35'06''W$ a distance of 1071.55 feet;
- 14) $S28^{\circ}32'06''W$ a distance of 160.62 feet;
- 15) $S61^{\circ}35'28''W$ a distance of 950.19 feet;
- 16) $S66^{\circ}04'42''W$ a distance of 966.51 feet;
- 17) $S61^{\circ}55'05''W$ a distance of 742.11 feet;
- 18) $N69^{\circ}05'23''W$ a distance of 65.57 feet;
- 19) $N05^{\circ}34'13''W$ a distance of 234.77 feet;

- 20) N11°51'43"E a distance of 142.75 feet;
- 21) N34°31'34"E a distance of 734.91 feet;
- 22) N38°16'11"E a distance of 722.75 feet;
- 23) N44°51'56"E a distance of 909.64 feet;
- 24) S88°19'43"W a distance of 414.62 feet;
- 25) S16°39'44"W a distance of 119.61 feet;
- 26) S31°25'10"W a distance of 320.27 feet;
- 27) S50°17'27"W a distance of 297.64 feet;
- 28) S46°38'57"W a distance of 649.14 feet;
- 29) S40°04'49"W a distance of 994.34 feet;
- 30) S40°00'06"W a distance of 395.79 feet;
- 31) S45°37'31"W a distance of 334.36 feet;
- 32) S03°21'47"W a distance of 171.75 feet;
- 33) S11°43'20"E a distance of 167.55 feet;
- 34) S29°18'24"W a distance of 175.45 feet;
- 35) S70°04'39"W a distance of 193.60 feet;
- 36) S78°07'17"W a distance of 265.48 feet;
- 37) S27°08'06"W a distance of 135.59 feet;
- 38) S31°49'39"E a distance of 87.81 feet;
- 39) S17°43'48"E a distance of 249.68 feet;
- 40) S07°38'52"W a distance of 285.72 feet;
- 41) S32°04'10"W a distance of 549.95 feet;
- 42) S58°20'25"W a distance of 224.17 feet;
- 43) S31°35'23"W a distance of 817.79 feet;
- 44) S35°16'11"W a distance of 787.93 feet;
- 45) S22°43'35"W a distance of 180.34 feet;
- 46) S28°03'43"E a distance of 122.20 feet;
- 47) S51°58'02"E a distance of 224.52 feet;
- 48) S03°07'06"E a distance of 121.90 feet;
- 49) S44°38'09"W a distance of 793.85 feet;
- 50) Along a curve to the right, an arc distance of 203.67 feet, said curve having a central angle of 38°53'53", a radius of 300.00 feet, and a chord bearing S65°45'49"W for 199.78 feet;
- 51) N89°41'43"W a distance of 579.01 feet;
- 52) N02°29'53"E a distance of 1024.08 feet;
- 53) N17°31'09"E a distance of 988.81 feet;
- 54) N37°14'24"W a distance of 255.34 feet;
- 55) N60°25'06"E a distance of 272.79 feet;
- 56) N03°45'09"W a distance of 579.81 feet;
- 57) N89°06'31"E a distance of 278.28 feet;
- 58) N05°17'12"E a distance of 353.12 feet;
- 59) N24°32'05"W a distance of 636.15 feet;
- 60) N85°40'11"E a distance of 449.52 feet;
- 61) N27°24'18"E a distance of 839.97 feet;
- 62) N52°17'25"W a distance of 406.99 feet;
- 63) N81°34'55"E a distance of 756.11 feet;
- 64) N32°20'14"E a distance of 245.12 feet;

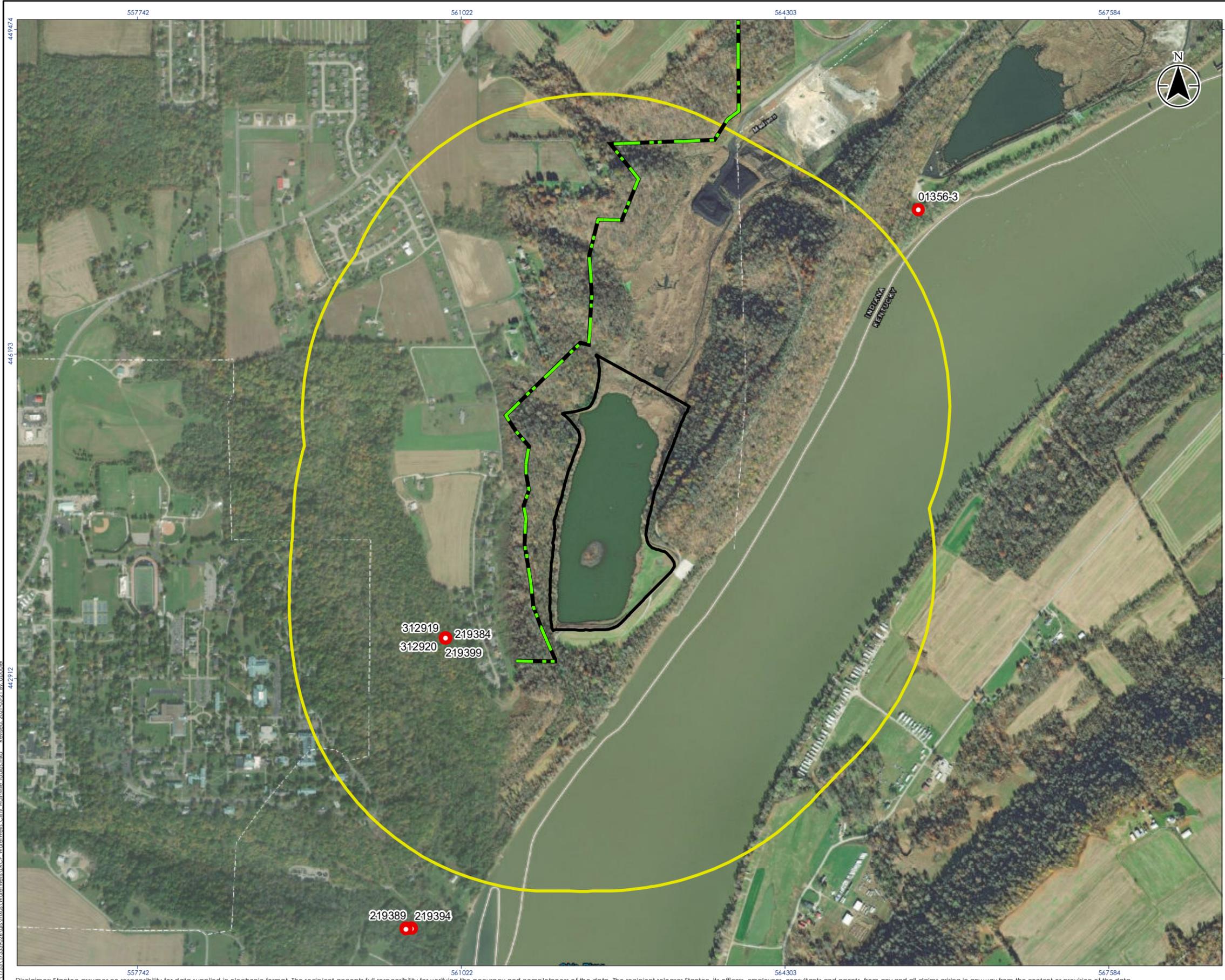
65) N52°14'21"E a distance of 142.14 feet to the Point of Beginning,
containing 357.74 acres, more or less.

Bearings are based on the Indiana State Plane Coordinate System, NAD '27 as per the
coordinates of the plant monuments provided by others.

This description was prepared by Kleingers & Associates, Inc., under the direction of
David L. Cox, Indiana Licensed Surveyor No. 890008 and is based on a partial survey of
the subject property performed in April and May, 2006. This description is to be used for
permit purposes only and is not for the conveyance of real property.

APPENDIX D

Location Figures



- Legend
- Private Wells
 - Property Line
 - LRPC Closure Boundary
 - Half-Mile Offset



- Notes
1. Coordinate System: Indiana State Plane East NAD83
 2. Base features - ESRI
 3. IDNR Water Wells June 2019
 4. Kentucky Water Wells - 2015
 5. USGS Quadrangle - Madison West
 6. IDNR estimated location by township, range, section, quarter section, and county



Project Location: Clifty Creek Station, Jefferson County, IN
 Prepared by AP on 04/14/2021
 Technical Review by EC on 04/14/2021
 Independent Review by PS on 04/14/2021

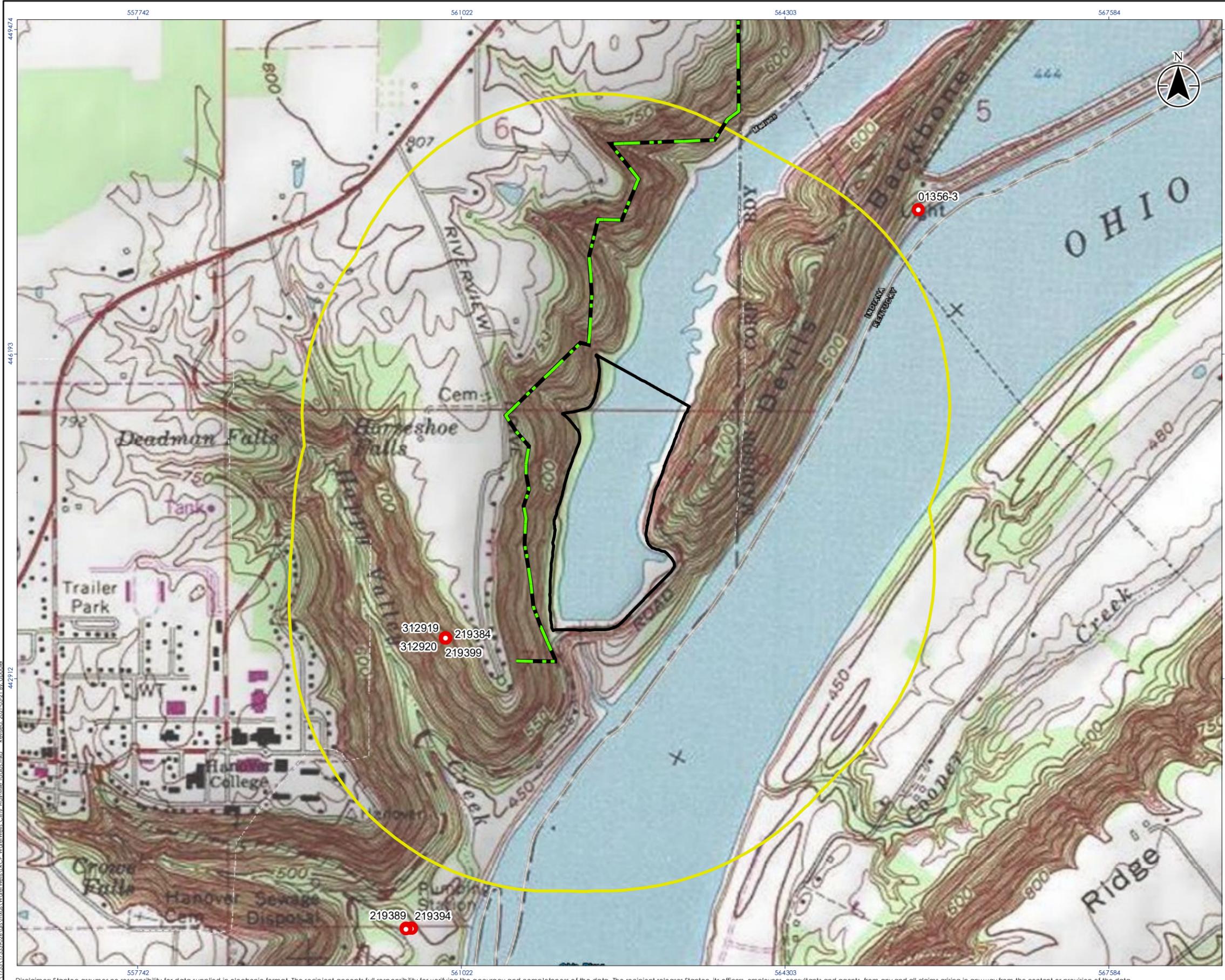
Client/Project: Indiana - Kentucky Electric Corporation
 Clifty Creek Station

Figure No. **2**

Title: **Water Well Half-Mile Offset
 Landfill Runoff Collection Pond Closure**

U:\1755\175539026\GIS\mxd\WaterWells\LRCP_WaterWells_Clifty_HalfMile_Offset.mxd - Revised: 2021_06_21 By: apocooler 442912

Disclaimer: Stantec assumes no responsibility for data supplied in electronic format. The recipient accepts full responsibility for verifying the accuracy and completeness of the data. The recipient releases Stantec, its officers, employees, consultants and agents, from any and all claims arising in any way from the content or provision of the data.



- Legend**
- Private Wells
 - Property Line
 - LRPC Closure Boundary
 - Half-Mile Offset



- Notes**
1. Coordinate System: Indiana State Plane East NAD83
 2. Base features - ESRI
 3. IDNR Water Wells June 2019
 4. Kentucky Water Wells - 2015
 5. USGS Quadrangle - Madison West
 6. IDNR estimated location by township, range, section, quarter section, and county



Project Location: Clifty Creek Station, Jefferson County, IN
 Prepared by AP on 04/14/2021
 Technical Review by EC on 04/14/2021
 Independent Review by PS on 04/14/2021

Client/Project: Indiana - Kentucky Electric Corporation
 Clifty Creek Station

Figure No. **3**
 Title: **Water Well Half-Mile Offset
 Landfill Runoff Collection Pond Closure**

Record of Water Well Indiana Department of Natural Resources

Reference Number	Driving Direction to Well	Date Completed
219384	2100W, 300N OF SE CORNER	6/30/1967

Owner-Contractor	Name	Address	Telephone
Owner	HANOVER COLLEGE	Not available	Not available
Driller	LAYNE NORTHERN	INDIANAPOLIS	Not available
Operator	LESTER HOYT	License Not available	

Construction Details			
Well	Use: Not available	Drilling Method: Not available	Pump Type: Not available
	Depth: 68.0	Pump Setting Depth: Not available	Water Quality: Not available
Casing	Length: Not available	Material: Not available	Diameter: 12.0
Screen	Length: 15.0	Material: Not available	Diameter: 12.0
	Slot Size: 50		

Well Capacity Test	Type of Test: Not available	Test Rate: 720.0 gpm	Bail Test Rate: Not available
	Drawdown: 13.0 ft.	Static Water Level: 30.0 ft.	Bailer Drawdown: Not available

Grouting Information	Material: Not available	Depth: From (not available) To (not available)
	Installation Method: Not available	Number of Bags Used: Not available

Well Abandonment	Sealing Material: Not available	Depth: From (not available) To (not available)
	Installation Method: Not available	Number of Bags Used: Not available

Administrative	County: JEFFERSON Range: 10E Topo Map: MADISON WEST, IN-KY Field Located By: Not available Courthouse Location By: Not available Location Accepted w/o Verification By: Not available Subdivision Name: Not available Ft W of EL: Not available Ft E of WL: Not available Ground Elevation: Not available Bedrock Elevation: Not available UTM Easting: Not available	Township: 3N Section: SE of the SE of the SW of Section 7 Grant: Not available Field Located On: Not available Courthouse Location On: Not available Location Accepted w/o Verification On: Not available Lot Number: Not available Ft N of SL: Not available Ft S of NL: Not available Depth of Bedrock: Not available Aquifer Elevation: Not available UTM Northing: Not available
-----------------------	--	---

Well Log

Top	Bottom	Formation
0.0	30.0	TOPSOIL & SOFT RED CLAY
30.0	33.0	GRITTY YEL CLAY
33.0	55.0	C. SAND AND GRAVEL W/ BOULDERS
55.0	60.0	M TO F SAND W/ BOULDERS
60.0	68.0	BLUE GREEN SHALE

Comments
CHEMICAL ANALYSIS

Record of Water Well

Indiana Department of Natural Resources

Reference Number	Driving Direction to Well	Date Completed
219399	SW 1/4 OF SE 1/4	10/11/1948
Owner-Contractor	Name	Address
Owner	HANOVER COLLEGE	HANOVER
Driller	A D COOK	Not available
Operator	Not available	License Not available
Telephone		
		Not available
		Not available
		Not available
Construction Details		
Well	Use: Not available	Drilling Method: Not available
	Depth: 69.0	Pump Setting Depth: Not available
		Pump Type: Not available
		Water Quality: Not available
Casing	Length: Not available	Material: Not available
Screen	Length: 16.0	Material: Not available
	Slot Size: Not available	Diameter: Not available
		Diameter: Not available
Well Capacity Test		
	Type of Test: Not available	Test Rate: 250.0 gpm
	Drawdown: Not available	Static Water Level: Not available
		Bail Test Rate: Not available
		Bailer Drawdown: Not available
Grouting Information		
	Material: Not available	Depth: From (not available) To (not available)
	Installation Method: Not available	Number of Bags Used: Not available
Well Abandonment		
	Sealing Material: Not available	Depth: From (not available) To (not available)
	Installation Method: Not available	Number of Bags Used: Not available
Administrative		
	County: JEFFERSON	Township: 3N
	Range: 10E	Section: SW of the SE of Section 7
	Topo Map: MADISON WEST, IN-KY	Grant: Not available
	Field Located By: Not available	Field Located On: Not available
	Courthouse Location By: Not available	Courthouse Location On: Not available
	Location Accepted w/o Verification By: Not available	Location Accepted w/o Verification On: Not available
	Subdivision Name: Not available	Lot Number: Not available
	Ft W of EL: Not available	Ft N of SL: Not available
	Ft E of WL: Not available	Ft S of NL: Not available
	Ground Elevation: Not available	Depth of Bedrock: Not available
	Bedrock Elevation: Not available	Aquifer Elevation: Not available
	UTM Easting: Not available	UTM Northing: Not available

Well Log

Top	Bottom	Formation
0.0	11.0	CLAY
11.0	33.0	LIME
33.0	57.0	S&G
57.0	60.5	FINE SAND
60.5	64.0	CLAY
64.0	69.0	CRS GRAV

Comments

None

Record of Water Well

Indiana Department of Natural Resources

Reference Number	Driving Direction to Well	Date Completed	
312919	Not available	Not available	
Owner-Contractor	Name	Address	Telephone
Owner	HANOVER COLLEGE	HANOVER	Not available
Driller	A.D. COOK	Not available	Not available
Operator	Not available	License Not available	
Construction Details			
Well	Use: Not available	Drilling Method: Not available	Pump Type: Not available
	Depth: Not available	Pump Setting Depth: Not available	Water Quality: Not available
Casing	Length: Not available	Material: Not available	Diameter: Not available
Screen	Length: Not available	Material: Not available	Diameter: Not available
	Slot Size: Not available		
Well Capacity Test	Type of Test: Not available	Test Rate: Not available	Bail Test Rate: Not available
	Drawdown: Not available	Static Water Level: Not available	Bailer Drawdown: Not available
Grouting Information	Material: Not available	Depth: From (not available) To (not available)	
	Installation Method: Not available	Number of Bags Used: Not available	
Well Abandonment	Sealing Material: Not available	Depth: From (not available) To (not available)	
	Installation Method: Not available	Number of Bags Used: Not available	
Administrative	County: JEFFERSON	Township: 3N	
	Range: 10E	Section: SW of the SE of Section 7	
	Topo Map: MADISON WEST, IN-KY	Grant: Not available	
	Field Located By: Not available	Field Located On: Not available	
	Courthouse Location By: Not available	Courthouse Location On: Not available	
	Location Accepted w/o Verification By: VCH	Location Accepted w/o Verification On: 6/1/1966	
	Subdivision Name: Not available	Lot Number: Not available	
	Ft W of EL: Not available	Ft N of SL: Not available	
	Ft E of WL: Not available	Ft S of NL: Not available	
	Ground Elevation: Not available	Depth of Bedrock: Not available	
	Bedrock Elevation: Not available	Aquifer Elevation: Not available	
	UTM Easting: Not available	UTM Northing: Not available	
Well Log Comments	ABANDONED		

Record of Water Well

Indiana Department of Natural Resources

Reference Number	Driving Direction to Well	Date Completed
312920	Not available	1/1/1940
Owner-Contractor	Name	Address
Owner	HANOVER COLLEGE	HANOVER
Driller	A.D. COOK	Not available
Operator	Not available	License Not available
Telephone		
		Not available
		Not available
Construction Details		
Well	Use: Not available	Drilling Method: Not available
	Depth: 80.0	Pump Setting Depth: Not available
		Pump Type: Not available
		Water Quality: Not available
Casing	Length: Not available	Material: Not available
Screen	Length: 20.0	Material: Not available
	Slot Size: Not available	Diameter: Not available
		Diameter: Not available
Well Capacity Test	Type of Test: Not available	Test Rate: Not available
	Drawdown: Not available	Static Water Level: Not available
		Bail Test Rate: Not available
		Bailer Drawdown: Not available
Grouting Information	Material: Not available	Depth: From (not available) To (not available)
	Installation Method: Not available	Number of Bags Used: Not available
Well Abandonment	Sealing Material: Not available	Depth: From (not available) To (not available)
	Installation Method: Not available	Number of Bags Used: Not available
Administrative	County: JEFFERSON	Township: 3N
	Range: 10E	Section: SW of the SE of Section 7
	Topo Map: MADISON WEST, IN-KY	Grant: Not available
	Field Located By: Not available	Field Located On: Not available
	Courthouse Location By: Not available	Courthouse Location On: Not available
	Location Accepted w/o Verification By: Not available	Location Accepted w/o Verification On: Not available
	Subdivision Name: Not available	Lot Number: Not available
	Ft W of EL: Not available	Ft N of SL: Not available
	Ft E of WL: Not available	Ft S of NL: Not available
	Ground Elevation: Not available	Depth of Bedrock: Not available
	Bedrock Elevation: Not available	Aquifer Elevation: Not available
	UTM Easting: Not available	UTM Northing: Not available
Well Log		
Comments		
SAND AND GRAVEL; S.W.L. 9/49-35' AND PUMPING LEVEL OF 44'; S.W.L. 1950-48' PUMPING LEVEL-40'; S.W.L. 8/50-24' PUMPING LEVEL 16'		

APPENDIX E

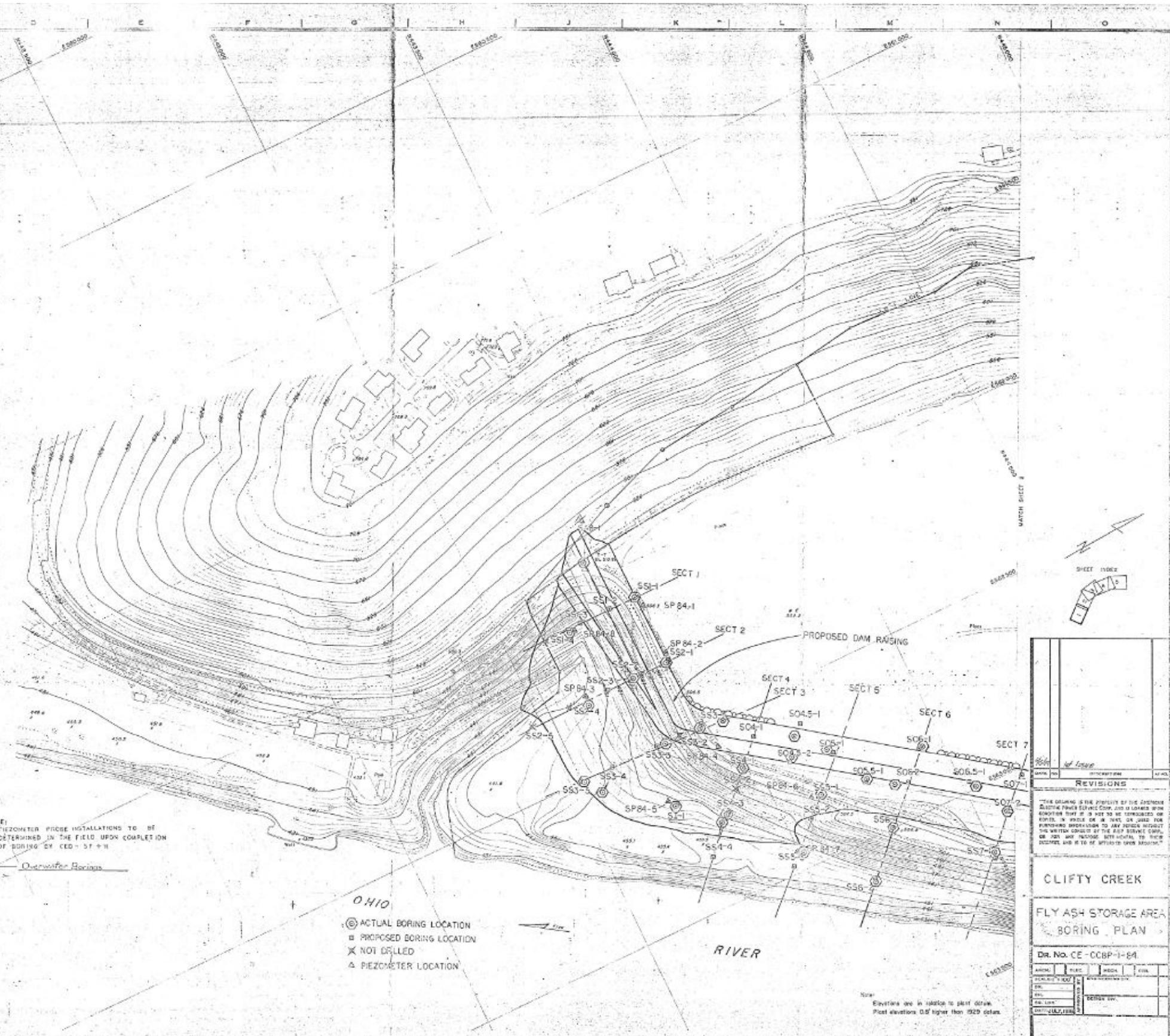
Geotechnical Data

AGESC (1956)

QTY	BORING NUMBER	COORDINATES	BORING TYPE	REMARKS
2	SS1-1	N 443346 E 562675	SPT	20' ROCK CORE
1	SAB-1	N 443350 E 562680	SPT	ROCK CORE TO ELEV 430' PRESSURE TESTS
1	SS1-1	N 443396 E 562705	SPT	20' ROCK CORE
1	SS1-2	N 443374 E 562692	CPT	NO ROCK CORE MAYBE AUGER BORING
2	SS1-3	N 443395 E 562705	CPT	NO ROCK CORE WATER AUGER BORING
4	SS1-4	N 443382 E 562706	SPT	20' ROCK CORE
1	SS2-1	N 443390 E 562705	SPT	20' ROCK CORE PIEZOMETER
1	SS2-2	N 443380 E 562706	CPT	NO ROCK CORE MAYBE AUGER BORING
2	SS2-3	N 443391 E 562706	CPT	20' ROCK CORE PIEZOMETER
3	SS2-4	N 443346 E 562715	SPT	NO ROCK CORE
4	SS2-5	N 443375 E 562710	CPT	20' ROCK CORE
1	SS3-1	N 443390 E 562707	SPT	ROCK CORE TO ELEV 420'
1	SS3-2	N 443388 E 562700	CPT	NO ROCK CORE MAYBE AUGER BORING
2	SS3-3	N 443392 E 562702	CPT	NO ROCK CORE
3	SS3-4	N 443372 E 562712	SPT	20' ROCK CORE
4	SS3-5	N 443380 E 562710	CPT	NO ROCK CORE
1	SS4-1	N 443462 E 562654	SPT	ROCK CORE TO ELEV 420' PIEZOMETER
1	SS4-2	N 443419 E 562700	CPT	NO ROCK CORE MAYBE AUGER BORING
2	SS4-3	N 443370 E 562760	CPT	NO ROCK CORE PIEZOMETER
3	SS4-4	N 443344 E 562773	SPT	20' ROCK CORE
1	SS5-1	N 443372 E 562826	SPT	ROCK CORE TO ELEV 430'
1	SS5-2	N 443370 E 562828	CPT	NO ROCK CORE
3	SS5-3	N 443468 E 562708	CPT	20' ROCK CORE
1	SS6-1	N 443395 E 562797	SPT	ROCK CORE TO ELEV 430' PRESS. TEST; 1" PIEZOMETER
2	SS6-2	N 443357 E 563115	SPT	20' ROCK CORE PIEZOMETER
1	SS7-1	N 443391 E 563136	SPT	ROCK CORE TO ELEV 430'
1	S04-1	N 443473 E 562515	CPT	ROCK CORE ELEV 430'
1	S04.5-1	N 443511 E 562541	SPT	10" ROCK CORE
1	S04.5-2	N 443502 E 562642	SPT	10" ROCK CORE
1	S05-1	N 443100 E 562717	CPT	ROCK CORE ELEV 430'
1	S05.5-1	N 443265 E 562842	SPT	10" ROCK CORE
1	S06-1	N 443147 E 562827	SPT	10" ROCK CORE
1	S06-2	N 443279 E 562830	CPT	ROCK CORE ELEV 430'
1	S06.5-1	N 444031 E 562293	SPT	10" ROCK CORE
1	S07-1	N 443180 E 563020	CPT	10" ROCK CORE
1	S07-2	N 444020 E 563014	SPT	ROCK CORE ELEV 430'
1	SAB-2	N 444374 E 563320	SPT	ROCK CORE TO ELEV 430' PRESSURE TESTS
2	SAB-3	N 444219 E 563174	SPT	20' ROCK CORE

NOTE:
PIEZOMETER PROBE INSTALLATIONS TO BE
DETERMINED IN THE FIELD UPON COMPLETION
OF BORING BY CED-57-9-8

Overwater Borings



- ACTUAL BORING LOCATION
- PROPOSED BORING LOCATION
- ✕ NOT DRILLED
- ▲ PIEZOMETER LOCATION

REVISIONS

NO.	DATE	DESCRIPTION

CLIFTY CREEK
FLY ASH STORAGE AREA
BORING PLAN

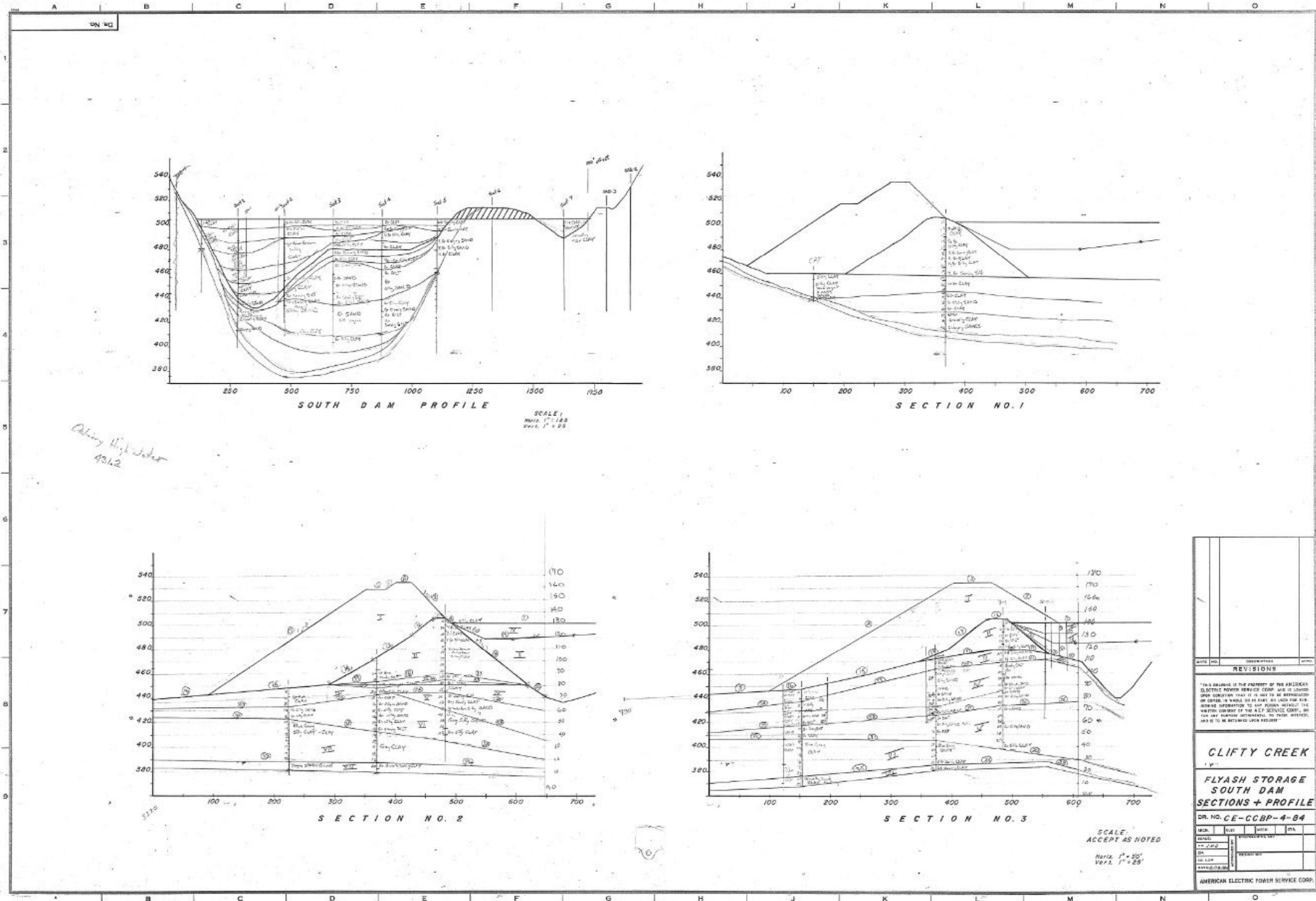
Dr. No. CE-CCBP-1-84

Scale: 1" = 100'

DATE: JULY, 1984

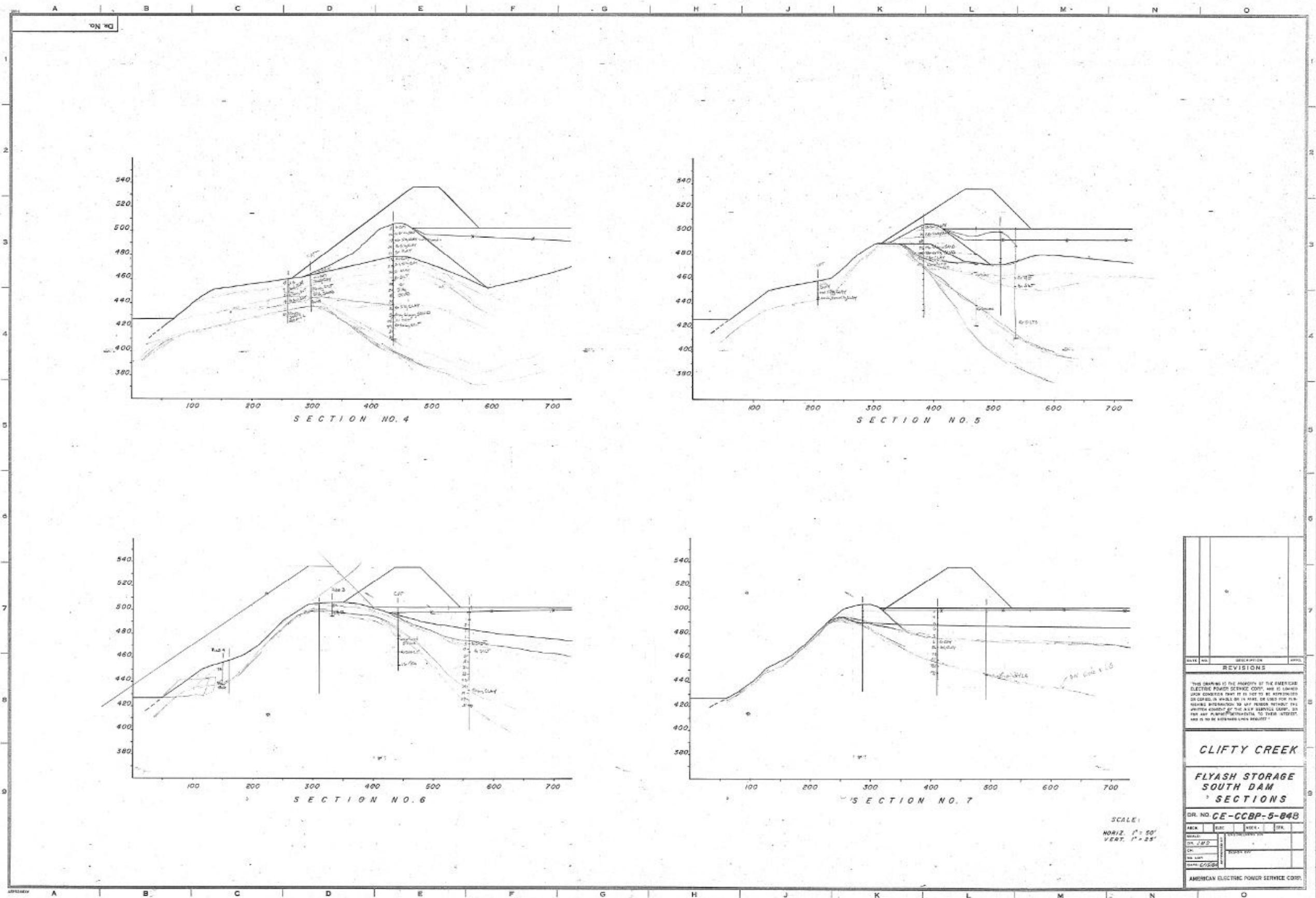
Sheet 1 of 5

Note:
Elevations are in relation to point datum.
Point elevations 0.8' higher than 1929 datum.



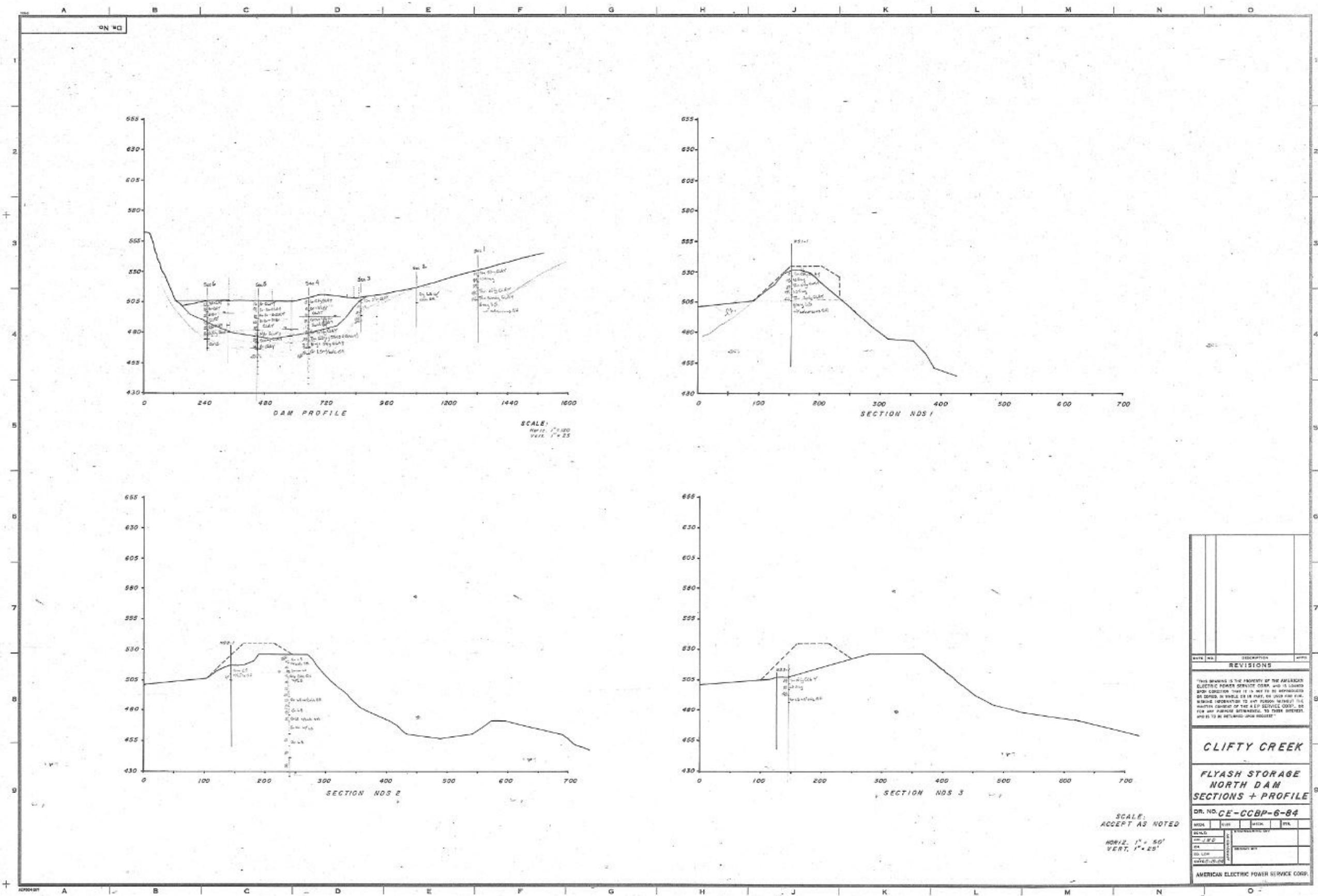
Along High Water
434.2

DATE	NO.	DESCRIPTION	BY
REVISIONS			
<small>THIS DRAWING IS THE PROPERTY OF THE AMERICAN ELECTRIC POWER SERVICE CORP. AND IS LOANED UNDER CONDITION THAT IT IS NOT TO BE REPRODUCED OR COPIED IN WHOLE OR IN PART, AND UNDER THE STRICT CONFIDENTIALITY OF THE A.E.P. SERVICE CORP. FOR ANY PURPOSES OTHER THAN TO WHICH INTENDED, AND IS TO BE RETURNED UPON REQUEST.</small>			
CLIFTY CREEK			
FLYASH STORAGE SOUTH DAM SECTIONS + PROFILE			
DR. NO. CE-CCBP-4-84			
SCALE:	ACCEPT AS NOTED		
Horiz. 1" = 50'	Vert. 1" = 25'		
AMERICAN ELECTRIC POWER SERVICE CORP.			



DATE	NO.	DESCRIPTION	BY
REVISIONS			
<small>THIS DRAWING IS THE PROPERTY OF THE AMERICAN ELECTRIC POWER SERVICE CORP. AND IS LOANED UNDER CONDITION THAT IT IS NOT TO BE REPRODUCED OR COPIED IN WHOLE OR IN PART, OR USED FOR PUBLISHING INFORMATION TO ANY PERSON WITHOUT THE WRITTEN CONSENT OF THE A.E.P. SERVICE CORP. OR ANY OTHER PARTY. PERMISSION TO THIS EFFECT, AND IS TO BE OBTAINED FROM THE A.E.P. SERVICE CORP.</small>			
CLIFTY CREEK			
FLYASH STORAGE SOUTH DAM SECTIONS			
DR. NO. CE-CCBP-5-848			
PREP.	DES.	CHK.	APP.
DR. J.M.P.			
CH.			
DATE			
SCALE			
AMERICAN ELECTRIC POWER SERVICE CORP.			

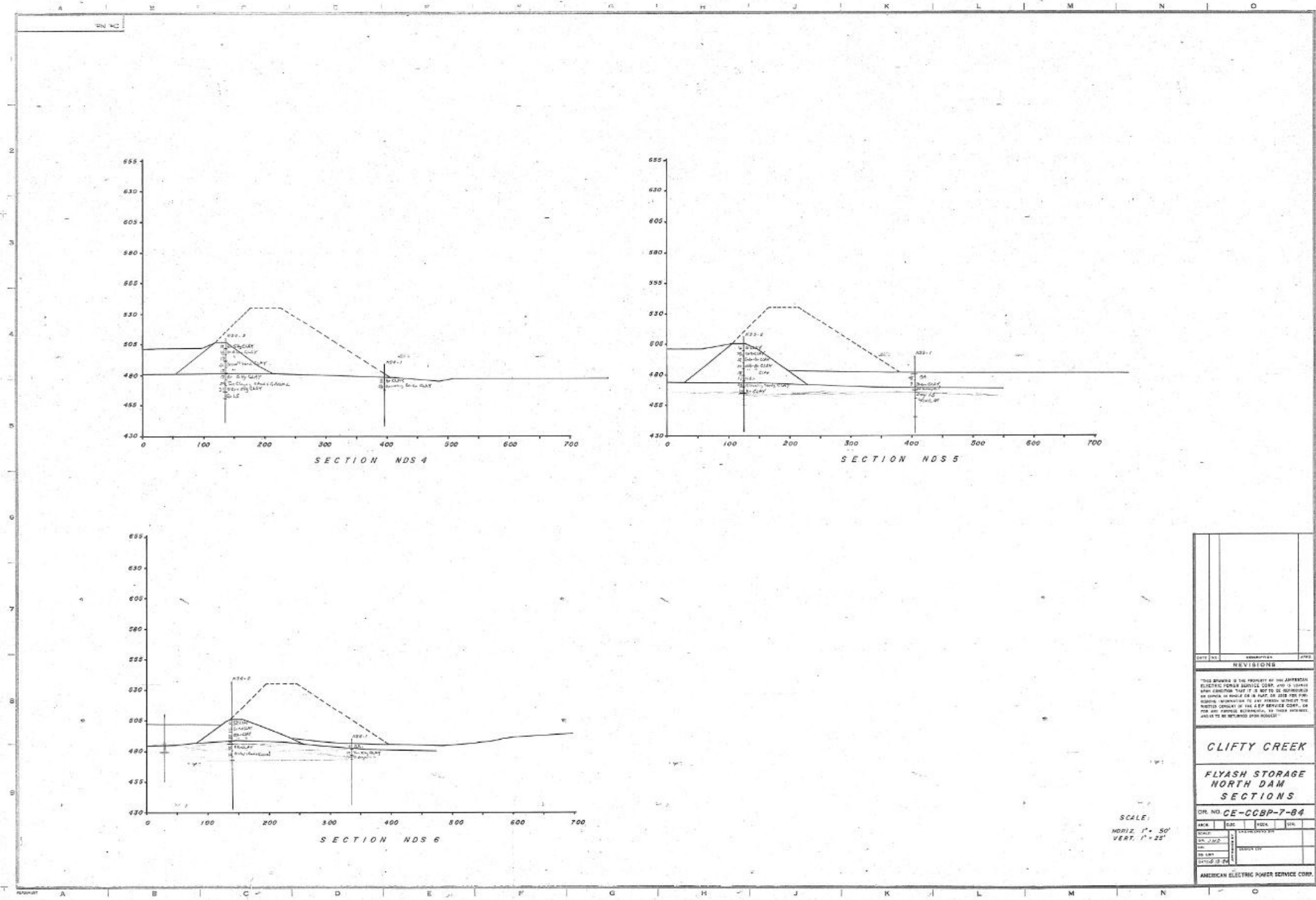
SCALE:
 HORIZ. 1" = 50'
 VERT. 1" = 25'



SCALE:
 HORIZ. 1" = 100'
 VERT. 1" = 25'

SCALE:
 ACCEPT AS NOTED
 HORIZ. 1" = 50'
 VERT. 1" = 25'

DATE	NO.	DESCRIPTION	BY
REVISIONS			
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CLIFTY CREEK			
FLYASH STORAGE NORTH DAM			
SECTIONS + PROFILE			
DR. NO. CE-CCBP-6-84			
DESIGNED BY	CHECKED BY	DATE	BY
APPROVED BY	DATE		
AMERICAN ELECTRIC POWER SERVICE CORP.			



DATE	BY	REVISIONS	APP'D.
REVISIONS			
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GLIFTY CREEK			
FLYASH STORAGE NORTH DAM SECTIONS			
DR. NO. CE-CCBP-7-84			
DATE	BY	CHKD.	APP'D.
02-1-84	J.M.P.		
AMERICAN ELECTRIC POWER SERVICE CORP.			

SCALE:
 HORIZ. 1" = 50'
 VERT. 1" = 25'

AGESC (1984)

**Indiana - Kentucky Electric Corporation
Clifty Creek Plant
Madison, Indiana**

Flyash Dam Raising Feasibility Report

**APPENDIX G
Boring Logs**

January 31, 1985

Prepared by

**Soils, Foundation and Hydro Section
Civil Engineering Division
American Electric Power Service Corporation**

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. NS4-1 Date _____ Sheet 3 of 4

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
							Gray calcareous shale
					18		Gray calcareous shale
					19		
					20		
		20.0 30.0	31.9	9.8			Gray calcareous shale
					21		Gray calcareous shale
					22		Gray calcareous shale
					23		
					24		

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

Job No. _____

Company _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

NSB

Boring No. _____ Date _____ Sheet 4 of 4

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION <small>Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.</small>
					25		<i>Calcareous shale Gray</i>
					26		
					27		<i>Gray calcareous shale</i>
					28		
					29		
					30		
							<i>Stopped boring 30.0</i>

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. NS4-2 DATE _____ SHEET 2 OF 3

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								20	
								1	
								2	
5	22.5	24.0	10	7	10	3"		3	Gravelly Sand - Clay - Br - moist 1" max size - limestone frags. GC
								4	
								5	
								6	
								7	
6	27.5	29.0	5	9	8	9"		8	Silty Clay - Br - moist - low to med plasticity - slight reaction to HCL
								9	CL
								30	
								1	
								2	
7	32.5	34.0	7	9	15	7"		3	Clayey sand + Gravel - TAN - saturated 3/4" max size - limestone frags. GC
								4	
								5	
								6	
								7	
8	37.5	39.0	6	11	10	3"		8	Silty Clay - Blue Green - moist - low to med plasticity - slight reaction to HCL
								9	CL
								40	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Boring No. N54-2 Date _____ Sheet 3 of 3

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
		40.0					Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
	"	44.8	22%	4.8	40.0 - 44.8		Gray Hard limestone w/ calcareous shale
					41		Gray calcareous shale
					42		
					43		Gray calcareous shale
					44		Gray calcareous shale
					45		Stopped boring 44.8

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

PANY IKEC
PROJECT CLIFTY CREEK NORTH FAD

BORING No. N55-1 DATE 8-20-84 SHEET 1 OF 5

TYPE OF BORING SPT CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 8-20-84 BORING COMPLETED 8-21-84

GROUND ELEVATION 479.40 REFERRED TO _____ DATUM

FIELD PARTY: Roush - Lambert

LOCATION OF BORING:	
WATER LEVEL	<u>7.5</u>
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING, NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								0	
								1	
								2	
								3	
<u>1</u>	<u>0</u>	<u>4.5</u>	<u>17</u>	<u>20</u>	<u>25</u>	<u>5"</u>		<u>3</u>	<u>Bottom Ash SATURATED</u>
								4	
								5	
								6	
								7	
								8	
	<u>8.0</u>	<u>9.5</u>	<u>5</u>	<u>5</u>	<u>4</u>	<u>6"</u>		<u>8</u>	<u>Clay - Blue Green + Be mottled - med to low plasticity - Moist</u>
								9	
								10	
								11	
								12	
								13	
<u>3</u>	<u>13.0</u>	<u>13.1</u>				<u>1"</u>		<u>3</u>	<u>lime stone gravel</u>
								4	
								5	
								6	
								7	
								8	
								9	
								20	

ENGINEER _____

ACF CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____
 Company _____
 Subject _____
 Location of Boring: _____
 Water Level _____
 Time _____
 Date _____

Boring No. N55-1 Date _____ Sheet 2 of 5
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____ Datum _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RDD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Deaths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		13.1					
		14.8	67%	1.7			lime stone lt. Ba. clay
					14		Gray calcareous shale lime stone
		14.8					Gray calcareous shale
		18.7	32%	3.9			lime stone
					15		Gray calcareous shale
					16		13.1-34.8 Gray Hard limestone w/ gray calcareous shale layers
					17		Gray calcareous shale
					18		Gray calcareous shale
					19		Gray calcareous shale
		18.7					
		24.8	25%	5.9			Gray Hard limestone w/ calcareous layers 25%
					20		

AEC CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring:	
Water Level	
Time	
Date	

Boring No. MS-1 Date _____ Sheet 3 of _____
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____
 _____ Date _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depth of water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					21		
					22		
					23		
					24		
		24.8 34.8	22 ^o / _h	10.0	25		Gray calcareous shale
					26		
					27		Gray calcareous shale

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

COMPANY TKEC
PROJECT CHIFTY CREEK FAD

BORING No. 1552 DATE 8-17-84 SHEET 1 OF 2

TYPE OF BORING SPT RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 8-17-84 BORING COMPLETED 8-17-84

GROUND ELEVATION 54.59 REFERRED TO _____ DATUM

FIELD PARTY: Roush - Lambert

LOCATION OF BORING:	
WATER LEVEL	<u>25.5</u>
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO								
									0	Moved boring, 4' west of stake
1	2.5	4.0	6	7	9	4"			2	Clay - Br. moist, low to med. plasticity - moderate reaction to HCL
									3	
									4	CL
									5	
									6	
2	7.5	9.0	4	10	50/2	7"			7	Clay - Gray Br. moist, low to med. plasticity, moderate reaction to HCL
									8	
									9	CL
									10	
									11	
3	12.5	14.0	6	6	6	5"			12	Clay - Gray Br. + Br. low to med. plasticity, moist, moderate reaction to HCL
									13	
									14	CL
									15	
									16	
									17	
4	17.5	19.0	3	8	14	2"			18	Clay - LT Br. Dr. Br. low to med. plasticity, strong reaction to HCL, moist to wet
									19	
									20	CL

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. NS-2 DATE _____ SHEET 2 OF 2

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								20	
								1	
								2	
5	22.5	24.0	4	6	12	2"		3	Clay - Lt. Br. Dr. Br. Low to med. plasticity, STRONG REACTION TO HCL. moist to wet.
								4	
								5	CL
								6	
								7	
6	27.5	29.0	5	6	5	3"		8	GRAVELLY SANDY CLAY - Lt. Br. SATURATED, STRONG REACTION TO HCL
								9	LIME STONE SAND + GRAVEL
								30	GC
								1	
								2	
7	32.5	34.0	6	9	11	5"		3	SAME AS Sample No. 6
								4	
								5	
								6	
								7	
8	37.5	39.0	4	7	7	1"		8	BR. Clay
								9	
								40	Auger Refusal 39.5 Stopped boring 39.5

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

OB No. _____

PANT JK EC

PROJECT CLIFTY CREEK NORTH FAD

LOCATION OF BORING: _____

WATER LEVEL 11.0

TIME _____

DATE _____

BORING NO. N55.51 DATE 8-21-84 SHEET 1 OF 2

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 8-21-84 BORING COMPLETED 8-21-84

GROUND ELEVATION _____ REFERRED TO _____

DATUM _____

FIELD PARTY: Roush + Lambert

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								0	BORING MOVED 40' EAST OF STAKE
								1	
1	2.5	4.0	7	8	5	3"		2	Mixture - Gray silty clay - Bottom 2 ft -
								3	limestone frag - moist - trace of org. mat.
								4	
								5	
								6	
2	7.5	9.0	2	3	6	12"		7	Clay - reddish br. - med. to low plasticity -
								8	moist. CL
								9	
								0	
								1	
								2	
3	12.5	13.2	4	5 1/2		6"		2	Silty clay - tan - moist to wet - low to
								3	med. plasticity - moderate reaction to HCL
								4	CL
								5	
								6	
								7	
								8	
								9	
								0	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. ^{NSJ.5-1} _____ Date _____ Sheet 2 of 3

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		14.9					
		19.5	35%	4.1	15		GRAY HARD lime STONE w/ CALcareous SHALE LAYERS
					16		
							Calcareous SHALE CORE lost?
					17		
					18		GRAY HARD limestone w/ CALcareous SHALE LAYERS
					19		
					20		stopped boring 19.5
					21		

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

COMPANY IKEC

PROJECT CLIFTY CREEK NORTH F.R.D.

BORING NO. MS6-1 DATE 8-21-84 SHEET 1 OF 5

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 8-21-84 BORING COMPLETED 8-21-84

GROUND ELEVATION 482.88 REFERRED TO _____ DATUM _____

FIELD PARTY: Roush-Eambert

LOCATION OF BORING:	
WATER LEVEL	<u>11.0</u>
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"					
							0	<u>Water Elev. Ground level 8-22-84</u> <u>Moved boring 7' west of stake</u>
							1	
							2	
							3	<u>Bottom ash moist</u>
<u>1</u>	<u>3.0</u>	<u>4.5</u>	<u>7</u>	<u>7</u>	<u>6</u>	<u>4"</u>	4	
							5	
							6	
							7	
							8	<u>Clay silt - tan. moist to wet - limestone</u> <u>frag. trace of org. mat.</u>
<u>2</u>	<u>8.0</u>	<u>9.5</u>	<u>2</u>	<u>9</u>	<u>8</u>	<u>6"</u>	9	
							10	<u>ML</u>
							1	
							2	
							3	<u>Gray shale + limestone frag.</u>
<u>3</u>	<u>13.0</u>	<u>13.5</u>	<u>15</u>	<u>59</u>	<u>2</u>	<u>2"</u>	4	
							5	
							6	
							7	
							8	
							9	
							0	

ENGINEER _____

AEC CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. NSL-1 Date _____ Sheet 2 of _____

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Datum

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RDD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
		13.5 15.2	20%	1.1			Rock Type, Color, Quality, Drilling observations, Depth of water lost, Observed fluctuations in water level, notes on drilling ease, etc.
							13.5- 35.2
							GRAY HARD LIMESTONE w/ CALCAREOUS SHALE LAYERS
					14		← CORE LOST ?
							GRAY SHALE CALCAREOUS
					15		
		15.2 25.2	25%	9.8			GRAY SHALE CALCAREOUS
					16		
							GRAY SHALE CALCAREOUS
					17		
							GRAY SHALE CALCAREOUS
					18		
							GRAY SHALE CALCAREOUS
					19		
					20		

NEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. NS67 Date _____ Sheet 3 of 5

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					21		
							> shale Gray calcareous
					22		> shale Gray calcareous
					23		shale Gray calcareous
							> shale " "
					24		
					25		
		25.2 35.2	58%	10.0			
					26		
					27		

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring:	
Water Level	
Time	
Date	

Boring No. NS6-1 Date _____ Sheet 4 of 5

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION <small>Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.</small>
					28		Shale Gray calcareous
							Shale " "
					29		shale " "
					30		shale " "
					31		
					32		
					33		
					34		

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

PANY _____

PROJECT _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

BORING No. N56-2 DATE _____ SHEET 2 OF 3

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"					
							20	
							1	
5	22.5	29.0	6	10	13	4"	2	Clay- Reddish Br. moist, low to med, plasticity w/ limestone frag.
							3	
							4	
							5	
							6	
							7	
6	27.5	29.0	14	8	8	12"	8	Clayey Sandy + Gravel - Br. SATURATED 3/4" max size, STRONG REACTION TO HCl
							9	(limestone GRAVELS)
							30	
							1	
							2	limestone Gravel
7	32.5	32.8	50			1/3	3	
							4	
							5	
							6	
							7	
							8	
							9	
							0	

ENGINEER _____

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Boring No. N56-2 Date _____ Sheet 3 of _____

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

Field Party: _____

Project _____
Location of Boring: _____

Water Level	_____
Time	_____
Date	_____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
		35.5 40.2	68%	4.6			35.5 - 40.2 Gray Hard Limestone w/ Gray calcareous shale layers
					36		Gray calcareous shale
					37		" " "
					38		" " "
					39		" " "
					40		" " "
					41		stopped boring 40.2

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

ANY _____

PROJECT _____

LOCATION OF BORING: _____

WATER LEVEL	
TIME	
DATE	

BORING No. NAB-5 DATE _____ SHEET 3 OF 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

_____ DATUM

FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"					SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							40	GRAY lime stone
	40.4	42.9			2.4		1	
							2	
	42.9	45.8			2.8		3	GRAY lime stone
							4	
							5	
	45.8	50.8			4.7		6	GRAY lime stone
							7	
							8	
							9	
							50	
	50.8	55.8			4.9		1	GRAY lime stone w/ CALcareous shale layers
							2	
							3	
							4	
							5	
	55.8	65.8			9.6		6	GRAY CALcareous shale w/ lime stone layers
							7	
							8	
							9	
							60	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

PANY _____

PROJECT _____

BORING No. NAB-5 DATE _____ SHEET 4 OF 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	_____
TIME	_____
DATE	_____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"					SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							60	
							1	
							2	
							3	
							4	
							5	
	65.8	75.8			9.8	RQP 62%		GRAY LIME STONE
							6	
							7	
							8	
							9	
							70	
							1	
							2	
							3	
							4	
							5	
	75.8	85.8			10.0	39%		GRAY LIME STONE
							6	
							7	
							8	
							9	
							80	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

BORING No. _____ DATE _____ SHEET 5 OF 5

COMPANY _____

TYPE OF BORING _____ RIG _____

PROJECT _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

LOCATION OF BORING: _____

BORING BEGUN _____ BORING COMPLETED _____

WATER LEVEL _____

GROUND ELEVATION _____ REFERRED TO _____

TIME _____

_____ DATUM

DATE _____

FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION <small>SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.</small>
	FROM	TO	BLOWS / 6"					
							80	
							1	
							2	
							3	
							4	
							5	
	85.8	95.8			100	R9D 84%	6	Gray Hard limestone
							7	
							8	
							9	
							10	
							1	
							2	
							3	
							4	
							5	
	95.8	100.5			50	R9D 83%	6	Gray Hard limestone
							7	
							8	
							9	
							100	

ENGINEER _____

BOWSER-MORNER, INC.

420 DAVIS AVENUE, P.O. BOX 51, DAYTON, OHIO 45401

ENGINEERING REPORT

REPORT TO: American Electric Power Service Corp. REPORT DATE: October 2, 1984
1 Riverside Plaza
P.O. Box 16631
Columbus, Ohio 43216 REPORT NO.: 28089-1084-425

Attention: Mr. Kevin C. Miller
Civil Engineering Division

REPORT ON: Submitted of Field Boring Logs, Indiana-Kentucky Electric Corporation, Clifty Creek Power Station Flyash Impoundment, Madison, Indiana

Bowser-Morner, Inc. was authorized to provide drilling services at the subject site by contract number 1134 dated August 24, 1984.

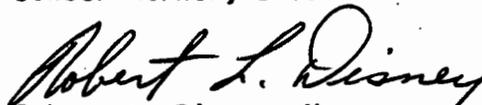
All soil samples, rock cores, and field boring logs were given to A.E.P. drill crews working at site. Results of the cone penetration tests performed by Woodward Clyde Consultants have not been received by Bowser-Morner, Inc. as of this date. This information is expected to be received by October 5, 1984, and will be forwarded immediately upon receipt.

All field boring logs have been rewritten to help clarify the hand written submitted field logs. All soil strata were visually classified by the driller and no attempt was made to change his logs, but only to make them more legible.

Please contact the writer if there are any questions.

Respectfully submitted,

Bowser-Morner, Inc.


Robert L. Disney, Manager
Soils Exploration Department

RLD/lag(#7)
2-Client
2-File

Coordinates, Elevations - Clifty Creek Ash Pond
Borings and Piezometers

<u>Boring No.</u>	<u>North</u>	<u>East</u>	<u>Grd. Elev.</u>
SS1-1	443,396.608	562,095.889	506.4'
SS1-2	443,309.415	561,945.600	511.2
SS1-3	443,185.435	562,105.878	457.5
SS2-1	443,396.944	562,298.268	505.2
SS2-3	443,291.332	562,299.155	473.5
SS2-4	443,146.177	562,315.152	444.0
SS3-1	443,400.792	562,507.540	505.7
SS3-3	443,292.943	562,502.207	475.0
SS3-4	443,072.526	562,552.619	453.1
SS3-5	443,040.442	562,501.381	450.3
SS4-1	443,462.922	562,663.196	504.4
SS4-4	443,344.316	562,773.019	456.5
SS5-1	443,629.159	562,826.808	504.0
SS6-1	443,769.260	562,997.733	506.0
SS6-2	443,657.692	563,115.269	452.4
SS7-1	443,998.052	563,186.481	506.0
SAB-1	443,311.870	561,942.766	511.2
SAB-3	444,219.168	563,174.539	510.3
SI-1	443,246.916	562,675.227	456.9
NS1-1	449,764.876	566,714.021	530.6
NS2-1	449,972.633	566,848.095	517.2
NS5-1	450,696.599	567,097.703	481.8
NS5-2	450,593.658	566,839.317	505.4
NS5.5-1	450,706.318	566,947.011	483.5
NS6-1	450,808.096	566,964.043	484.4
NS6-2	450,742.242	566,781.840	506.2
NS4-1	450,415.751	567,159.228	477.2
NS4-2	450,380.723	566,901.629	505.7
NAB-5	450,031.446	566,992.941	528.8
NS3-1	450,153.057	566,937.097	507.7

SS4-3
N 443365.4 E 562761.5 E1 458.0

SS5-3
443510.0 562,954.6 453.5

<u>Piezometer</u>	<u>North</u>	<u>East</u>	<u>Ground</u>	<u>Elevations</u> <u>Casing</u>	<u>Top Pipe</u>
SP84-1	443,404.716	562,121.849	505.30'	507.79'	507.70'
SP84-2	443,404.130	562,270.565	504.66	508.48	508.38
SP84-3	443,149.098	562,289.250	440.65	444.07	444.02
SP84-4	443,429.340	562,578.553	504.18	507.52	507.45
SP84-5	443,231.782	562,657.726	457.60	460.52	460.46
SP84-6	443,603.765	562,810.631	505.04	507.77	507.68
SP84-7	443,497.555	562,943.715	454.42	457.37	457.29
SP84-8	443,200.487	562,104.880	456.69	459.59	459.52
NP84-1	450,320.291	566,927.815	505.59	508.27	508.19
NP84-2	450,441.324	566,890.223	505.48	509.03	508.98
NP84-3	450,660.383	566,703.275	502.62	505.79	505.69
NP84-4	450,413.695	567,161.991	477.32	480.02	479.80
NP84-5	450,692.879	567,088.078	481.80	484.92	484.92

Coordinates - Clifty Creek Ash Pond
Borings - Underwater

<u>Boring No.</u>	<u>North</u>	<u>East</u>
S04-1	443,473.371	562,515.058
S04.5-1	443,633.008	562,641.125
S04.5-2	443,602.971	562,692.785
S05-1	443,700.203	562,717.288
S05.5-1	443,765.818	562,842.851
S06-1	443,947.949	562,827.761
S06.5-1	444,031.583	562,993.305
S06-2	443,829.158	562,890.528
S07-2	444,080.933	563,094.392

Water Elevation 10/2/84 500.16'

S07-1 not found in pond

OCT 29 1984

AMERICAN ELECTRIC POWER SERVICE CORPORATION



DATE: Oct. 26, 1984

SUBJECT: Boring Locations and 765KV Tower Elevations
Clifty Creek Plant fly ash dam area.

FROM: K. L. Hern

TO: Kevin Miller - Columbus

As per our phone conversation of Oct. 23, 1984 please find listed below the coordinates and ground elevations of borings SS-4-3 and SS-5-3 in the south end of the fly ash dam area.

<u>Boring No.</u>	<u>North</u>	<u>East</u>	<u>Elev.</u>
SS4-3	443,365.4	562,761.5	458.0' (Grd.)
SS5-3	443,510.0	562,954.6	453.5' "

Boring S07-1 was not located due to the float being missing.

We also ran elevations on the 765KV tower legs that you requested, please find elevations listed below. Direction is noted as facing the Jefferson sub-station.

<u>Leg</u>	<u>Ground Elev.</u>
L/Rear	535.06
R/Rear	534.86
L/Front	511.25
R/Front	511.61

K. L. Hern, jr.
K. L. Hern

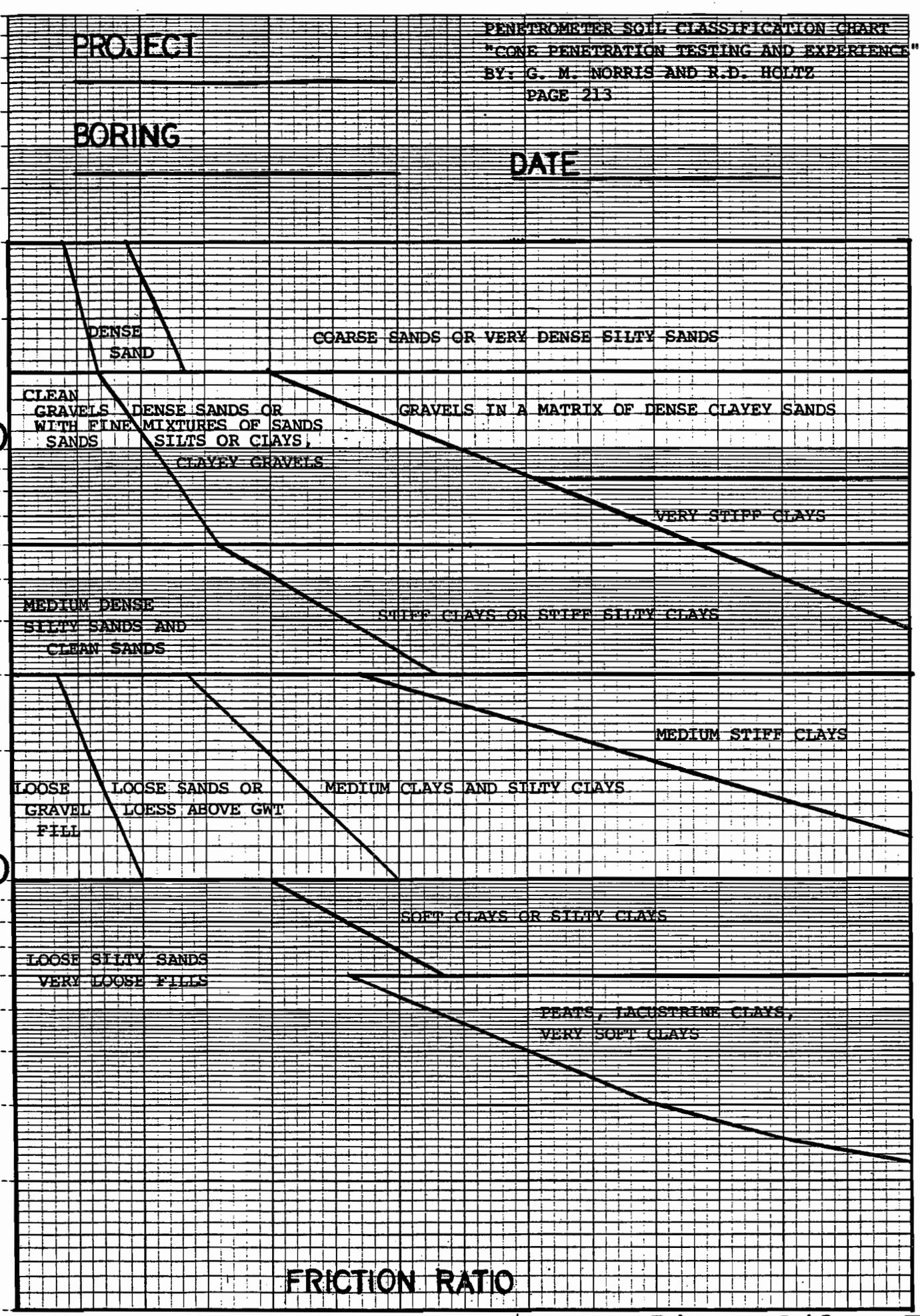
/jp
xc: 001.011.000.003:984.300
068.447.290.001:984.300

PROJECT _____

BORING _____

DATE _____

10
9
8
7
6
5
4
3
2
100
9
8
7
6
5
4
3
2
10
9
8
7
6
5
4
3
2
1



AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY IKEC
PROJECT CLIFTY CREEK Fly ASH DAM

BORING No. SS1-1 DATE 6-27-84 SHEET 1 of 9

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 6-27-84 BORING COMPLETED 6-27-84

GROUND ELEVATION 505.52 REFERRED TO _____ DATUM _____

LOCATION OF BORING:	
WATER LEVEL	<u>62.5</u>
TIME	
DATE	

FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								0	moved boring 40 FT SOUTH OF STAKE
								1	
1	2.5	4.0	7	7	10	6"		2	CLAY - multi-colored BRs, low to med. plasticity, moist
								3	CL
								4	
								5	
								6	
2	7.5	9.0	3	5	7	7"		7	Same as Sample No. 1
								8	
								9	
								10	
								1	
								2	Same as Sample No. 1
3	12.5	14.0	3	7	10	8"		3	
								4	
								5	
								6	
								7	
4	17.5	19.0	7	7	8	9"		8	silty clay - Gray + Br. mottled, low to med plasticity, STRONG REACTION TO HCL, moist
								9	
								2.0	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SS1-1 DATE _____ SHEET 2 OF 9

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO	6	8	9	11"				
									20	
									1	
									2	
5	22.5	24.0	6	8	9	11"			3	Silty clay - GRAY + BR. mottled, low plasticity, STRONG REACTION TO HCL, moist CL-MI
									4	
									5	
									6	
									7	
6	27.5	29.0	5	6	9	9"			8	Sandy clay - yellowish BR. low plasticity, moist, SLIGHT REACTION TO HCL CL
									9	
									30	
									1	
									2	
7	32.5	34.0	5	9	12	9"			3	Clay - Reddish BR. low to med. plasticity, moist
									4	
									5	
									6	
									7	
8	32.5	39.0	8	12	11	16"			8	Silty clay - yellowish BR. low plasticity, moist, moderate REACTION TO HCL
									9	
									40	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

BORING No. SS1-1 DATE _____ SHEET 3 OF 9

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

FIELD PARTY: _____ DATUM _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO							SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								4 0	<i>Piezometer</i>
								1	
								2	
9	42.5	44.0	5	12	18	14"		3	<i>Sandy silt - yellowish Br. slight plasticity, moist</i> <i>ML</i>
								4	
								5	
								6	<i>40.0 to 50.0 Reddish Br. saturated silt (auger return)</i>
								7	
10	42.5	49.0	11	13	11	15"		8	<i>Same as Sample No. 9</i>
								9	
								5 0	
								1	
								2	
11	52.5	54.0	7	10	13	15"		3	<i>clay - Gray + Br. mottled, low to med. plasticity, moist</i> <i>CL</i>
								4	
								5	
								6	
								7	
12	52.5	59.0	13	16	19	16"		8	<i>Same as Sample No. 11</i>
								9	
								6 0	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

ANY _____

PROJECT _____

BORING No. SSI-1 DATE _____ SHEET 4 of 9

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

DATUM

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO								
									60	
									1	
13	62.5	64.0	9	11	16	16"			2	Clay - Reddish Br. med. plasticity, moist
									3	CL
									4	
									5	65.2 WATER
									6	
14	67.5	69.5	2	4	4	7"			7	Clayey sand - Br. SATURATED, Poorly Graded, 100% Fine grain
									8	SC
									9	
									70	
									1	
15	72.5	74.0	4	8	12	16"			2	Clay - Gray - moist - med. plasticity
									3	CL
									4	
									5	
									6	
16	77.5	79.0	9	10	14	16"			7	Gravelly clay - Blue, moist, med. to Hi. plasticity, Gravel poorly graded limestone frag.
									8	CL
									9	
									80	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

BORING No. SSI-1 DATE _____ SHEET 5 OF 9

COMPANY _____

TYPE OF BORING _____ RIG _____

PROJECT _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

LOCATION OF BORING:	
WATER LEVEL	_____
TIME	_____
DATE	_____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								8 0	
								1	
								2	
17	82.5	84.0	10	13	17	14"		3	GRAVELLY clay - olive green, moist Med to Hi plasticity, gravel poorly Graded limestone frag
								4	CL-GC
								5	
								6	
								7	
18	87.5	89.0	21	26	20	15"		8	Clayey sands - yellowish BR. poorly grade, moist
								9	SC
								9 0	90.5 Augering Hard
								1	
								2	
								3	
								4	
								5	
								6	
								7	
								8	
								9	
								0	

ENGINEER _____

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Boring No. SSI-1 Date _____ Sheet 6 of 6

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

Project _____
Location of Boring: _____

Water Level	_____
Time	_____
Date	_____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RDD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
							93.0-94.0
	"	93.0-95.0	0	2.0	94		WEATHERED LIMESTONE w/ CALcareous Be shale on joints
					95		
		95.0-105.0	34%	9.3	96		Grey hard limestone
					97		
					98		98.2 Lost water
					99		
					100		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

Boring No. SSI-1 Date _____ Sheet 7 of 9

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Datum

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RDD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					101		
					102		
					103		
					104		
					105		CORE LOSS ?
	105-115	51%	10.0		106		GRAY HARD lime STONE w/ LAMINATE LAYERS OF CALCAREOUS SHALE
					107		

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY TKEC
PROJECT Cherty Creek Fly Ash Dam

BORING No. SS1-2 DATE 7-10-84 SHEET 1 of 2

TYPE OF BORING SPT RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 7-10-84 BORING COMPLETED 7-10-84

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: Roush - Lambert

LOCATION OF BORING:	
WATER LEVEL	<u>DRY</u>
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								0	
								1	
								2	
								3	
1	3.0	4.5	7	10	12	5"		3	Clay. Reddish Br. moist, low to med. plasticity.
								4	
								5	CL
								6	
								7	
								8	
2	8.0	8.8	7	5 ⁹ / ₁₃		5"		8	Same as Sample No. 1
								9	
								10	
								11	
								12	
								13	
3	13.0	13.6	17	5 ⁹ / ₁₁		4"		13	Gravelly Clay. Reddish Br. yellowish Br moist. Gravel consist of lime stone frag. 1/2" max size
								14	
								15	
								16	
								17	
								18	
								19	
4	18.0	19.5	8	12	15	4"		18	Same as Sample No. 3
								19	
								20	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY I.KEC
PROJECT CHFTY CREEK F.A.D.

BORING No. SAB-1 DATE 9-12-84 SHEET 1 OF _____

TYPE OF BORING SPT RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 9-12-84 BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"					SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							0	
							1	
							2	
							3	
1	3.0	4.5	4	5	12	3"		Gravelly Sandy clay - Br. moist 1" max size gravel. consist of lime stone frags.
							4	
							5	
							6	
							7	
							8	
2	8.0	9.5	4	9	11	8"		Silt/ clay - Reddish Br. moist - low to med plasticity CL
							9	
							10	
							11	
							12	
							13	
							14	
							15	
3	13.0	14.5	4	7	15	11"		Clay - yellowish Br. moist - med. to low plasticity CL
							16	
							17	
							18	
							19	
4	18.0	19.5	14	20	26	10"		Gravelly Sand clay - yellowish Br. moist - gravel. consist of lime stone frag
							20	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SAB-1 DATE _____ SHEET 2 OF _____

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"					SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							20	
							1	
							2	
							3	
5	23.0	23.0	50%		0		4	
							5	
							6	
							7	
							8	
							9	
							0	
							1	
							2	
							3	
							4	
							5	
							6	
							7	
							8	
							9	
							0	

ENGINEER _____

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

SAG-1

Boring No. _____ Date _____ Sheet 3 of _____
Type of Boring _____ Rig _____
Casing used _____ Size _____ Drilling mud used _____
Boring begun _____ Boring completed _____
Ground Elevation _____ referred to _____ Datum _____
Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION <small>Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.</small>
		23.0					
		24.9	0	1.7			CORE LOST? 23.0-24.9 Gray Hard limestone w/ calcareous shale layers 15 MAX length of core
					24		
		24.9	10%	6.8			
		32.7			25		24.9-32.7 CORE LOST? Gray calcareous shale w/ limestone layers
					26		
					27		
					28		
					29		
					30		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

SAB-1

Boring No. _____ Date _____ Sheet 4 of _____
Type of Boring _____ Rig _____
Casing used _____ Size _____ Drilling mud used _____
Boring begun _____ Boring completed _____
Ground Elevation _____ referred to _____
Date _____
Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RDD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
					31		
					32		
		32.7 34.9	58%	2.0	33		lime stone Gray Hard GRAY CALcareous shale
					34		GRAY CALcareous shale
		34.9 44.9	39%	9.8	35		34.9-44.9 GRAY Hard lime stone w/ CALcareous shale layers
					36		GRAY CALcareous shale
					37		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring:	
Water Level	
Time	
Date	

SAB-1

Boring No. _____ Date _____ Sheet 5 of _____

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					38		<i>Gray calcareous shale</i>
					39		
					40		
					41		
					42		
					43		
					44		

Dutch Cone Test

Company IKEC

Probe No. SSI-3 Date 8-15-84

Job CLIFTY CREEK FAD

Drillers _____

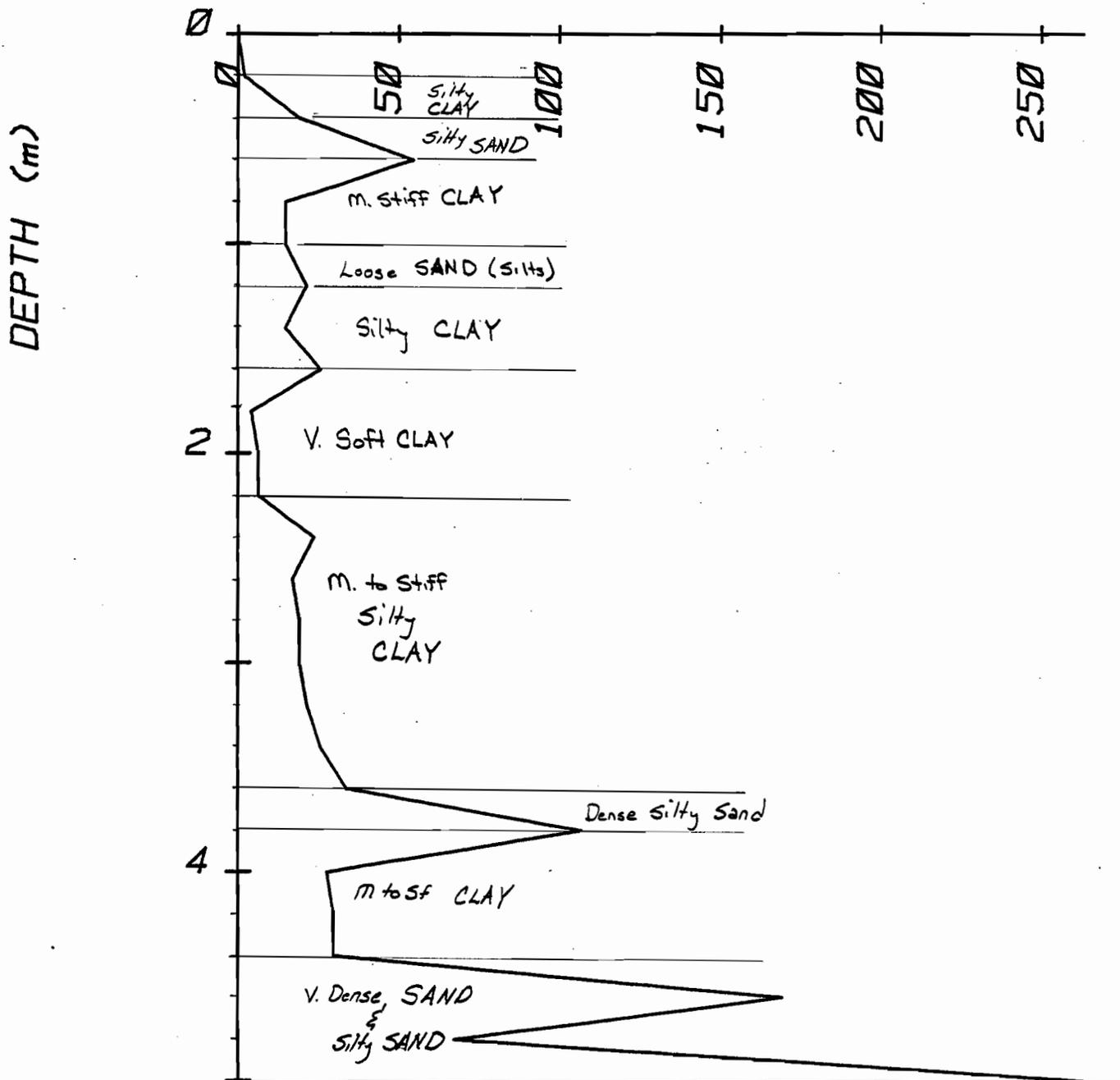
Recorder Lambert

Page 1 of 2 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS								
		.2	C		—			
			C+S		—			
		.4	C		1.6	19	0.56	0.029
			C+S		2.4			
		.6	C		5.0	54	0.42	0.008
			C+S		5.6			
		.8	C		1.2	15	1.25	0.085
			C+S		3.0			
		1.0	C		1.2	15	0.84	0.056
			C+S		2.4			
		.2	C		1.8	21	0.28	0.013
			C+S		2.2			
		.4	C		1.2	15	0.42	0.028
			C+S		1.8			
		.6	C		2.2	25	0.70	0.028
			C+S		3.2			
		.8	C		1.2	4	1.39	0.347
			C+S		2.2			
		2.0	C		.4	6	0.56	0.093
			C+S		1.2			
		.2	C		.4	6	0.70	0.117
			C+S		1.4			
		.4	C		2.0	23	0.70	0.030
			C+S		3.0			
		.6	C		1.4	17	0.97	0.057
			C+S		2.8			
		.8	C		1.6	19	0.70	0.037
			C+S		2.6			
		3.0	C		1.6	19	0.70	0.037
			C+S		2.6			
		.2	C		1.8	21	1.39	0.066
			C+S		3.8			
		.4	C		2.2	25	2.78	0.111
			C+S		6.2			
		.6	C		3.0	33	1.39	0.042
			C+S		5.0			
		.8	C	10.0		106	2.09	0.020
			C+S	13.0				
		4.0	C		2.4	27	6.68	0.247
			C+S	12.0				
		.2	C		2.6	29	2.23	0.077
			C+S		5.8			
		.4	C		2.6	29	1.25	0.043
			C+S		4.4			
		.6	C	16.0		169	3.34	0.020
			C+S	11.0				
		.8	C		6.2	67	1.25	0.019
			C+S		8.0			
		5	C	25.0		263	3.48	0.013
			C+S					

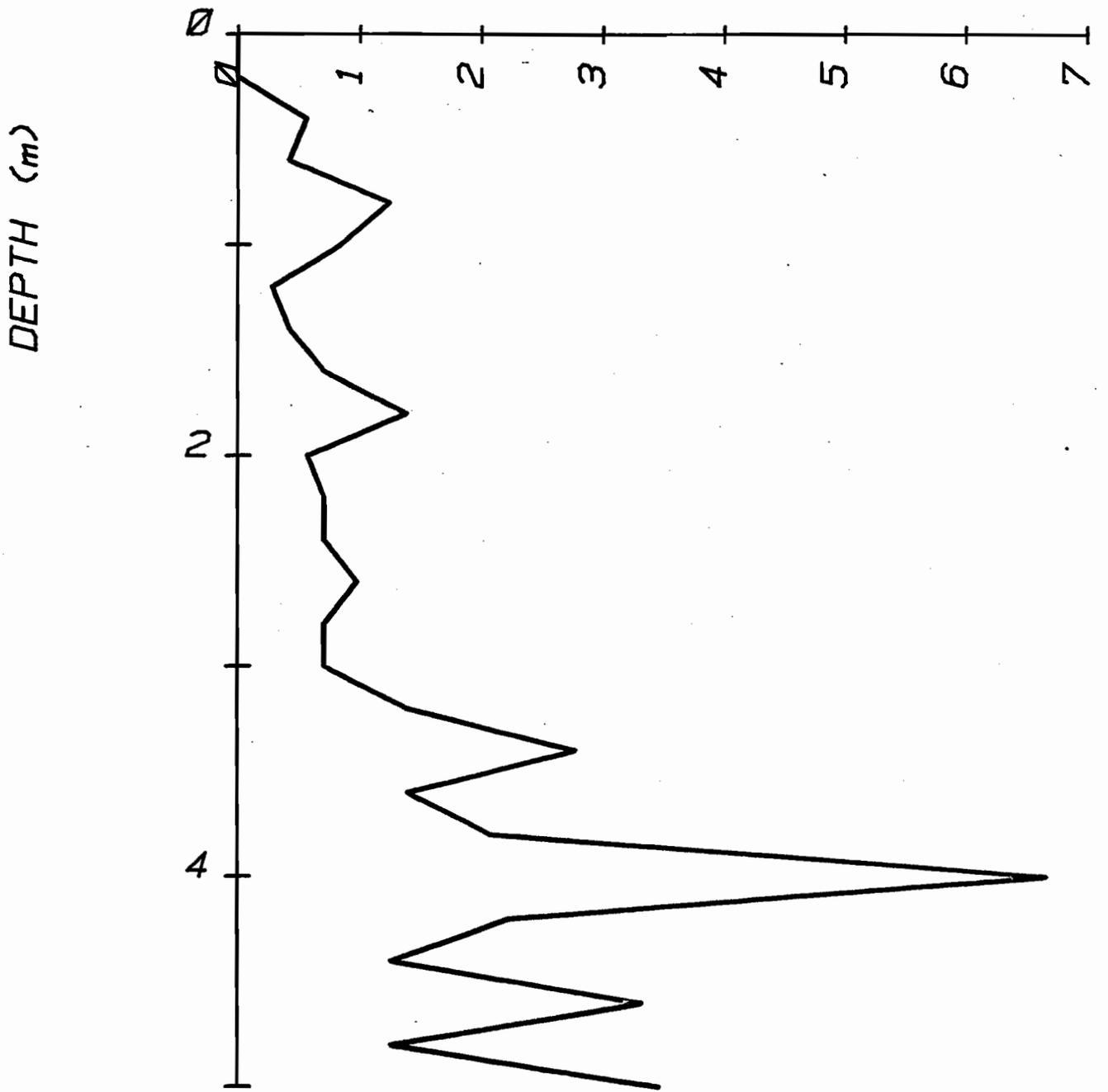
25' SOUTH OF STAKE

CONE RESISTANCE (TSF)



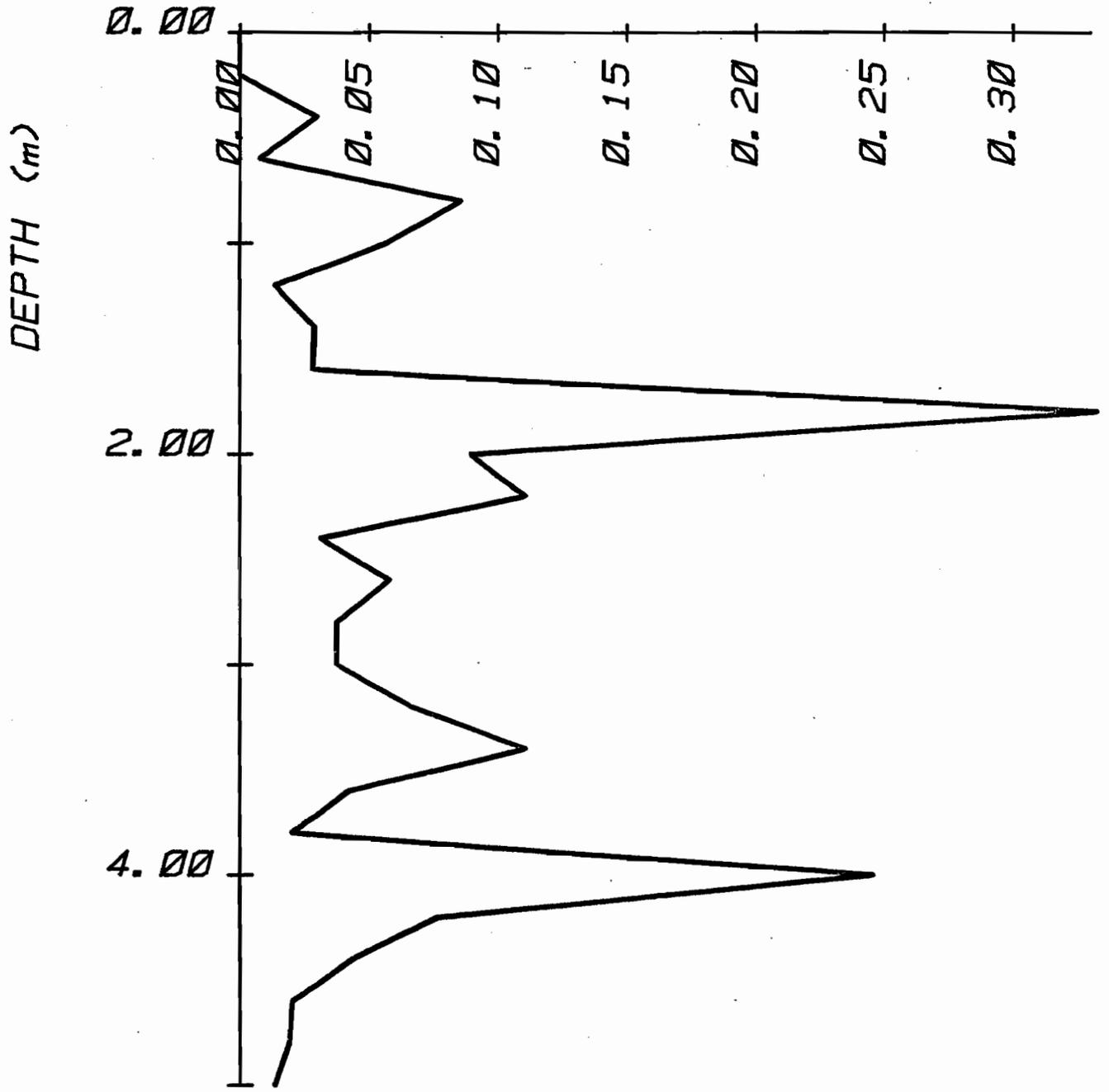
Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS1-3

SLEEVE RESISTANCE (TSF)



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS1-3

FRICITION RATIO



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS1-3

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

COMPANY IKEC
PROJECT CHITY CREEK Fly Ash Dam

BORING NO. SS2-1 DATE 6-28-84 SHEET 1 OF 5

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 6-28-84 BORING COMPLETED _____

GROUND ELEVATION 504.45 REFERRED TO _____ DATUM _____

FIELD PARTY: Roush - Bumgarner

LOCATION OF BORING:	
WATER LEVEL	<u>28.5</u>
TIME	
DATE	<u>7-10-84</u>

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								0	Moved boring 5 FT SOUTH
								1	
								2	
1	3.0	4.5	3	5	5	8"		3	Silty Clay - Gray Br. low to med. plasticity, moist
								4	
								5	CL
								6	
								7	
2	8.0	9.5	3	5	7	6"		8	Clay - yellowish Br. + Gray Br. low to med plasticity, moist, slight reaction to HCL
								9	CL
								10	
								11	
								12	
3	13.0	14.5	4	5	8	8"		3	Clay - Gray low to med plasticity / moist, slight reaction to HCL
								4	CL
								5	
								6	
								7	
								8	
4	18.0	19.5	6	13	13	12"		8	Silty Clay - yellowish Br. low plasticity, moist
								9	
								20	CL-MC

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

BORING No. SS2-1 DATE _____ SHEET 3 OF 5

COMPANY _____

TYPE OF BORING _____ RIG _____

PROJECT _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

_____ DATUM

FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								40	
								1	
								2	
								3	
9	43.0	44.5	7	6	14	12"			Silt/clay - yellowish Br. low to med. plasticity, slight reaction to HCL, moist
								5	CL
								6	
								7	
								8	
10	48.0	49.5	6	8	21	16"			Sandy clay - Br. moist, low plasticity, slight reaction to HCL
								9	CL
								50	
								1	
								2	
								3	
11	53.0	54.5	10	14	19	15"			Clay - Br + Gray - low to med plasticity, moist
								5	CL
								6	
								7	
								8	
12	58.0	59.5	10	16	19	16"			Clay - Gray - med to low plasticity moist
								9	
								60	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SS2-1 DATE _____ SHEET 4 of 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

FIELD PARTY: _____ DATUM _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION <small>SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES, BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.</small>
	FROM	TO	BLOWS / 6"						
								60	
								1	
								2	
								3	
13	63.0	64.5	8	12	14	15"		4	Sandy silt - Br. slight plasticity moist ML
								5	
								6	
								7	
								8	
14	68	69.5	4	6	7	14"		9	Sandy clay - Gray, low plasticity moist, trace of organic CL
								70	
								1	
								2	
								3	
15	73.0	74.5	4	5	7	14"		4	Silt/Sand - yellowish Br, 100% fine grain, saturated SM
								5	
								6	
								7	
								8	
	78.0	79.5	15	23	20	16"		9	Silty sand - Gray, saturated, 100% fine grain, strong reaction to HCL SM
								80	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

BORING No. 552-1 DATE _____ SHEET 5 OF 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"							
									80	
									1	
									2	
									3	
17	83.0	84.5	10	13	15	16"			4	SILT SAND - GRAY, 100% FINE GRAIN, SATURATED, STRONG REACTION TO HCL
									5	SM
									6	
									7	
									8	
18	88.0	89.5	7	10	12	16"			9	Same as Sample No 17
									10	
									1	
									2	
									3	
19	93.0	94.5	10	14	15	16"			4	CL Silty Clay. Gray Low to med. plasticity, moist to wet, strong reaction to HCL
									5	CL
									6	
									7	
									8	Stopped boring 94.5
									9	
									0	

ENGINEER _____

Dutch Cone Test

Company TKEC

Probe No. 552-3 Date 8-15-84

Job CLIFTY CREEK PAD

Drillers Roush + Lambert

Order Lambert

Page 1 of 6 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
				0.0	-0.2			
			C					
		.2	C+S					
			C					
		.4	C+S					
			C		7.4			
		.6	C+S		9.0	79	1.11	0.014
			C		5.4			
		.8	C+S		8.8	58	2.37	0.041
			C		4.8			
		1.0	C+S		8.8	52	2.78	0.054
			C		6.8			
		1.2	C+S		9.2	73	1.67	0.023
			C		4.4			
		.4	C+S		7.4	48	2.09	0.044
			C		4.4			
		.6	C+S		7.2	48	1.95	0.041
			C		3.6			
		.8	C+S		5.6	40	1.39	0.035
			C		3.2			
		2.0	C+S		5.8	35	1.81	0.052
			C		2.0			
		.2	C+S		5.0	23	2.09	0.091
			C		5.0			
		.4	C+S		6.2	54	0.84	0.016
			C		4.2			
		.6	C+S		7.6	46	2.37	0.052
			C		4.8			
		.8	C+S		8.4	52	2.51	0.048
			C		4.0			
		3.0	C+S		7.0	44	2.09	0.048
			C		5.2			
		.2	C+S		8.2	56	2.09	0.037
			C		6.4			
		.4	C+S		9.0	69	1.81	0.026
			C		6.4			
		.6	C+S		9.4	69	2.09	0.030
			C		3.4			
		.8	C+S		7.0	38	2.51	0.066
			C		4.2			
		4.0	C+S		7.6	46	2.37	0.052
			C					
		.2	C+S					
			C					
		.4	C+S					
			C					
		.6	C+S					
			C					
		.8	C+S					
			C					

20' SOUTH OF 552-2

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SS2-3 DATE _____ SHEET 3 of 6

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO	BLOWS / 6"						
								3 0	
								1	
5	32.5	34.0	7	9	9	13"		2	Clay - Gray, moist - low to med. plasticity.
								3	
								4	cl
								5	
								6	
6	37.5	39.0	7	7	9	16"		7	clayey sand - Gray - moist - poorly graded. 100% FINE GRAIN
								8	
								9	sc
								4 0	
								1	
7	42.5	44.0	5	7	8	16"		2	silty clay - Gray - low plasticity - moist.
								3	
								4	cl
								5	
								6	
8	47.5	49.0	4	6	6	16"		7	silty sand - Gray - moist to wet - 100% FINE GRAIN. FINE GRAIN lens w/ org. MAT.
								8	
								9	sm
								5 0	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

PANY _____

PROJECT _____

BORING No. SS2-3 DATE _____ SHEET 4 OF 6

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE No.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								5 0	
								1	
								2	
9	52.0	54.0	6	9	8	16"		3	Silty Clay - moist - Gray - Low plasticity - w/ org. MAT. (Lignite?)
								4	
								5	
								6	
								7	
10	57.5	59.0	3	6	7	15"		8	Sandy Silt - Gray - moist to wet - 100% v-fine grain
								9	ML
								6 0	
								1	
								2	
11	62.5	64.0	5	6	8	15"		3	Clay - Gray - moist - low to med. plasticity
								4	CL
								5	
								6	
								7	
12	67.5	69.0	5	9	10	16"		8	Same as Sample No 11
								9	
								7 0	

ENGINEER

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

PANY _____

PROJECT _____

BORING No. SS2-3 DATE _____ SHEET 5 of 6

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								70	
								1	
								2	
13	72.5	74.0	5	8	9	16"		3	Clay - GRAY - moist - low to med. plasticity
								4	cl
								5	
								6	
								7	
14	77.5	79.0	4	7	8	16"		8	Same as Sample No. 13
								9	
								80	
								1	
								2	
15	82.5	84.0	6	8	8	16"		3	Same as Sample NO 13
								4	
								5	
								6	
								7	
16	87.5	89.0	8	12	8	8"		8	Gravelly sandy clay - GRAY SATURATED (Gravelly sand lime stone) 1" max size
								9	
								90	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB NO. _____

BORING No. SS23 DATE _____ SHEET 6 of 6

COMPANY _____

TYPE OF BORING _____ RIG _____

PROJECT _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

_____ DATUM

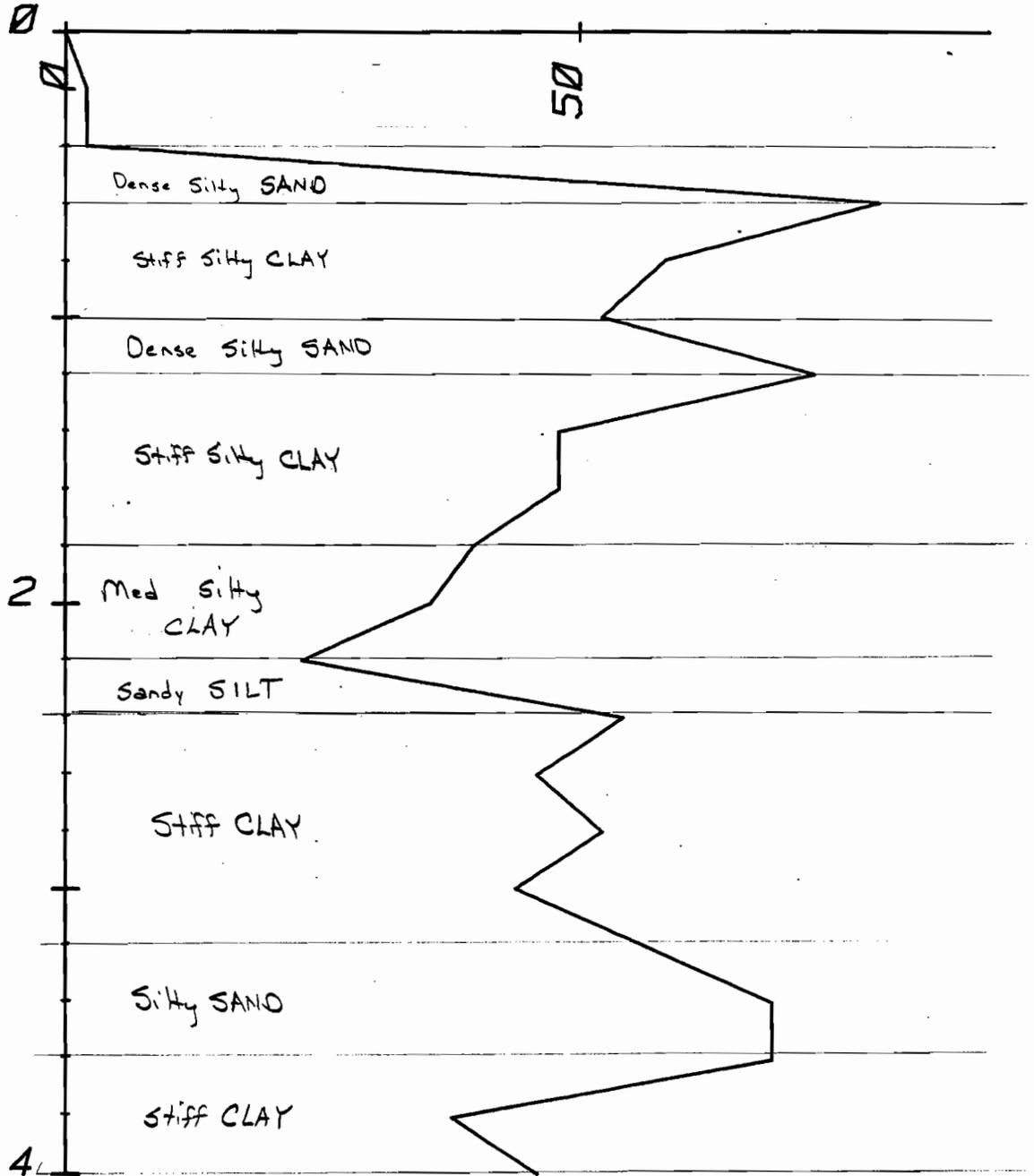
FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"					SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							90	
							1	
							2	
17	92.5	93.6	9	19	59 1	6'		Gravelly Sandy Clay - Gray SATURATED. 1" max size
							3	
							4	(Grave + sand - lime stone)
							5	
							6	
							7	Stopped boring 94.9
							8	
							9	
							0	
							1	
							2	
							3	
							4	
							5	
							6	
							7	
							8	
							9	
							0	

ENGINEER

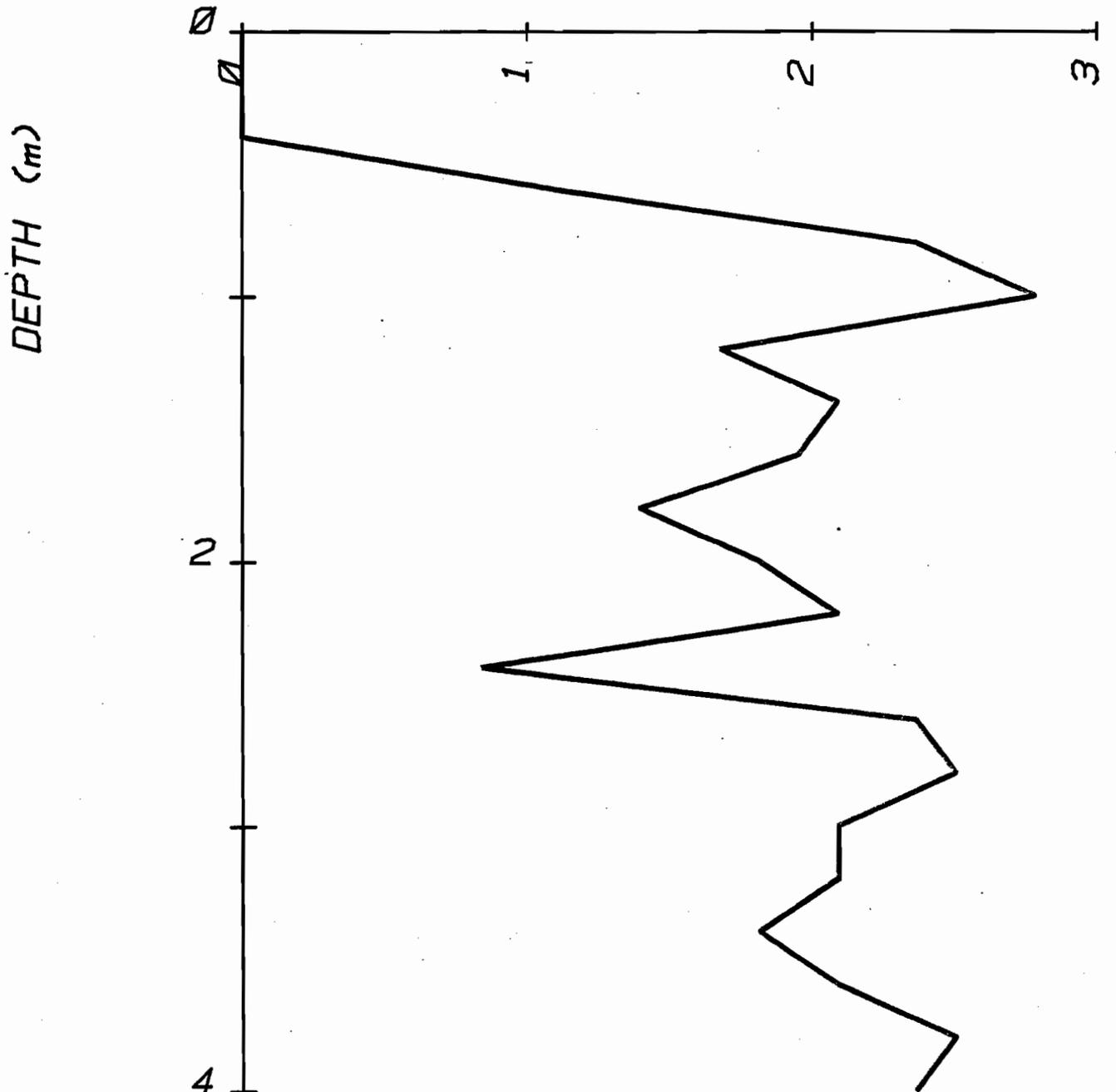
CONE RESISTANCE (TSF)

DEPTH (m)



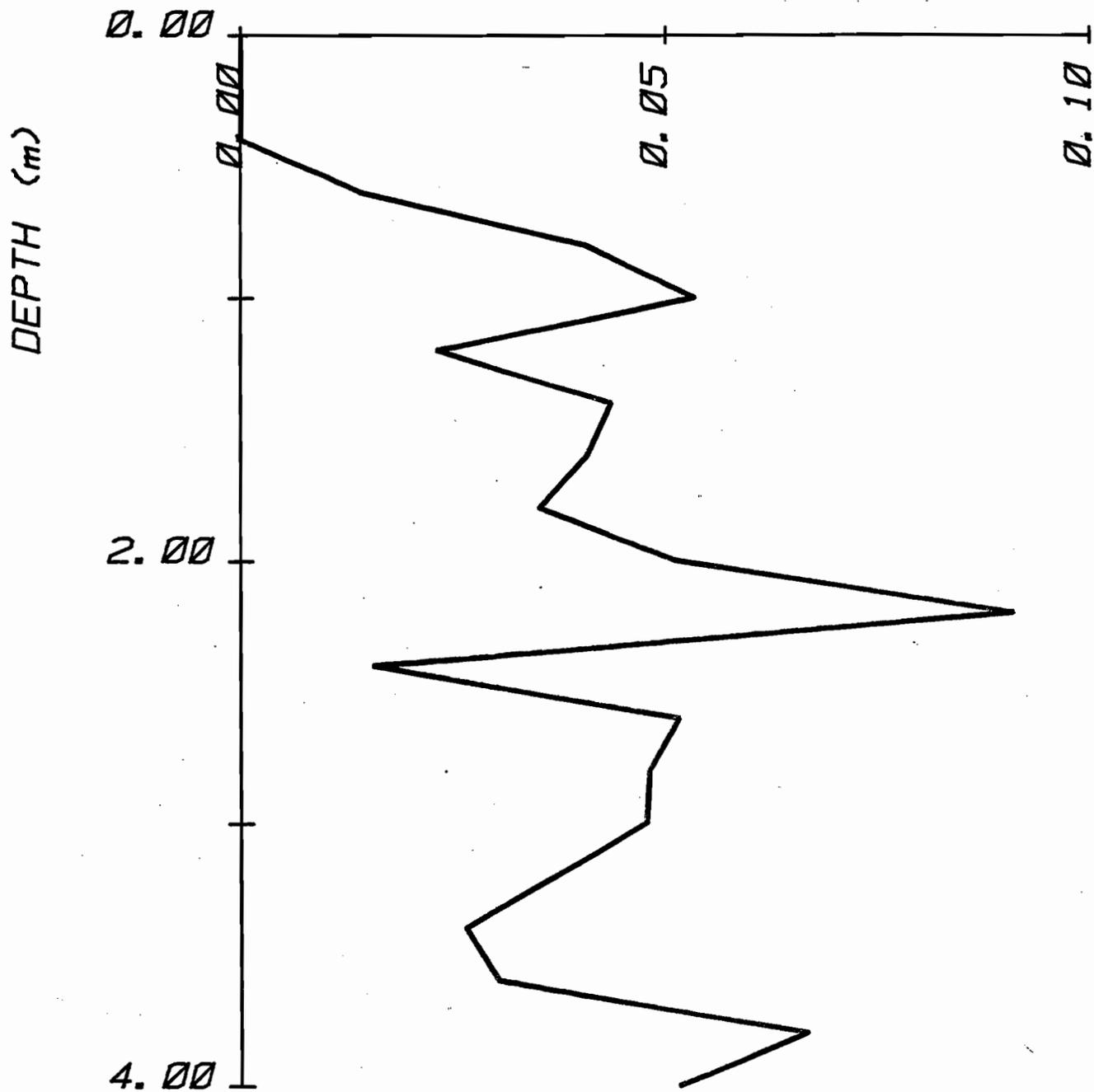
Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS2-3

SLEEVE RESISTANCE (TSF)



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS2-3

FRICITION RATIO



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS2-3

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY IKEC
PROJECT CLIFTY CREEK FAD

BORING No. 552-4 DATE 8-1-84 SHEET 1 OF 4

TYPE OF BORING SPT RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 8-1-84 BORING COMPLETED 8-1-84

GROUND ELEVATION 439.80 REFERRED TO _____

FIELD PARTY: Roush + Lambert

LOCATION OF BORING:	
WATER LEVEL	<u>54'</u>
TIME	
DATE	<u>8-1-84</u>

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								0	<u>7.4 water ELEV - 8-2-84</u>
								1	
								2	
<u>1</u>	<u>2.5</u>	<u>4.0</u>	<u>3</u>	<u>6</u>	<u>9</u>	<u>4"</u>		3	<u>clay - multi-colored Br. moist low to med. plasticity</u>
								4	<u>CL</u>
								5	
								6	
								7	
<u>2</u>	<u>7.5</u>	<u>9.0</u>	<u>5</u>	<u>7</u>	<u>5</u>	<u>2"</u>		8	<u>same as sample No. 1</u>
								9	
								10	
								11	
								12	
								13	
								14	
								15	
								16	
								17	
<u>3</u>	<u>12.5</u>	<u>14.0</u>	<u>2</u>	<u>3</u>	<u>5</u>	<u>14"</u>		18	<u>same as sample No. 1</u>
								19	
								20	
								21	
								22	
								23	
								24	
								25	
								26	
								27	
								28	
								29	
								30	
								31	
								32	
								33	
								34	
								35	
								36	
								37	
								38	
								39	
								40	
								41	
								42	
								43	
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								48	
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								61	
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								67	
								68	
								69	
								70	
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								73	
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								75	
								76	
								77	
								78	
								79	
								80	
								81	
								82	
								83	
								84	
								85	
								86	
								87	
								88	
								89	
								90	
								91	
								92	
								93	
								94	
								95	
								96	
								97	
								98	
								99	
								100	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

PANY _____

PROJECT _____

BORING NO. SS2-4 DATE _____ SHEET 2 of 4

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								20	
								1	
5	22.5	24.0	2	3	3	16"		2	Silty Clay - Gray, wet, low to med. plasticity, w/ black (pyrite?)
								3	
								4	CL
								5	
								6	
6	27.5	29.0	4	8	9	16"		7	Clay. Blue Green, moist, med to low plasticity
								8	
								9	CL
								30	
								1	
7	32.5	39.0	4	8	9	16"		2	Same as Sample No 6 Trace of V-Fine sand
								3	
								4	
								5	
								6	
								7	
8	37.5	39.0	4	7	8	16"		8	Same as Sample No. 6
								9	
								40	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SSZ-4 DATE _____ SHEET 3 of 4

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								40	
								1	
								2	
9	42.5	44.0	3	4	7	16"		3	Clay - Blue-Gray. low to med plasticity moist to wet - TRACE OF V-FINE SAND CL
								4	
								5	
								6	
								7	
10	47.5	49.0	5	7	10	16"		8	Same as Sample No. 9
								9	
								50	
								1	
								2	
11	52.5	54.0	5	8	11	16"		3	Same as Sample No. 9
								4	
								5	Water 54'
								6	
								7	
12	52.5	58.0	8	11	11	12"		8	silty Clay. Blue Gray. SATURATED Low to med plasticity - TRACE OF limy stone FRAG END OF Spoon CL
								9	
								60	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING



JOB No. _____

COMPANY IKEC
PROJECT CLIFTY CREEK FLY ASH DAM

LOCATION OF BORING:	
WATER LEVEL	<u>58.1</u>
TIME	
DATE	

BORING No. SS3-1 DATE 6-26-84 SHEET 1 OF 5
 TYPE OF BORING SpT RIG B-61
 CASING USED _____ SIZE _____ DRILLING MUD USED _____
 BORING BEGUN 6-26-84 BORING COMPLETED 6-26-84
 GROUND ELEVATION 459.16 REFERRED TO _____ DATUM Beaus
 FIELD PARTY: Roush-Lambert

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO								SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
									0	Moved boring 5' SE of stake
									1	
1	2.5	4.0	3	4	7	10"			2	Clay - BR, moist, low to med plasticity
									3	CL
									4	
									5	
									6	
2	7.5	9.0	5	6	7	10"			7	Silty clay - yellowish BR, low plasticity, moist, slight reaction to HCL
									8	
									9	CL-MI
									10	similar to SS1-1 #6
									11	
									12	Clay - BR, moist, low to med plasticity
3	12.5	14.0	4	10	12	8"			13	CL
									14	
									15	
									16	
									17	
4	17.5	19.0	3	7	10	12"			18	Silt - BR, slight plasticity, moist
									19	ML
									20	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

PANY _____

PROJECT _____

BORING No. SS3-1 DATE _____ SHEET 2 OF _____

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO								
									20	
									1	
5	22.5	24.0	7	9	13	14"			2	
									3	Silty Clay - Gray + Br. mottled, low to med. plasticity, moist, STRONG REACTION TO HCL
									4	CL
									5	Original ground
									6	
6	27.5	29.0	15	13	14	12"			7	
									8	Clayey Sand - Reddish Br. poorly graded, 100% Fine Grain, moist
									9	SC
									30	
									1	
									2	
7	32.5	34.0	3	4	6	8"			3	Silty Clay - Br. moist, low to med. plasticity
									4	CL
									5	
									6	
									7	
8	37.5	39.0	5	7	8	12"			8	Sandy silt - Br. slight plasticity moist
									9	ML
									40	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SS3-1 DATE _____ SHEET 3 OF _____

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								40	
								1	
								2	
9	42.5	44.0	7	9	13	15"		3	Sandy silt - Br. moist, slight plasticity, STRONG REACTION TO HCL
								4	ML
								5	
								6	
								7	 Not found in 4-1
10	42.5	49.0	11	12	12	13"		8	Sand - Lt. Br. moist, Poorly Graded, 100% Fine Grain, STRONG REACTION TO HCL
								9	SP
								50	
								1	
								2	
11	52.5	54.0	11	13	20	15"		3	clayey sand - Br. moist, STRONG REACTION TO HCL 100% FINE GRAIN
								4	SC
								5	
								6	
								7	top - silty clay - Br. moist, low plasticity, slight REACTION TO HCL
12	57.5	59.0	5	15	2	16"		8	SR.1 WATER
								9	
								60	Bottom - Sand - Br. poorly Graded, STRONG REACTION TO HCL
									SP

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SS3-1 DATE _____ SHEET 4 of 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO							SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								6 0	
								1	
								2	TOP
13	62.5	64.0	6	8	12	16"		3	Sandy silt - Br. wet, slight plasticity - STRONG REACTION TO HCL. Bottom ML
								4	SAND. Br. poorly graded, 100% FINE GRAIN, STRONG REACTION TO HCL SP
								5	
								6	
								7	
14	67.5	69.0	6	12	13	16"		8	Silty sand - Br moderate reaction to HCL, wet, poorly graded
								9	
								7 0	SM
								1	
								2	
15	72.5	74.0	6	7	7	16"		3	SAND - GRAY. SATURATED, POORLY GRADED, 100% FINE GRAIN, STRONG REACTION TO HCL
								4	
								5	SP
								6	
								7	
16	77.5	79.0	13	20	17	16"		8	Same as sample no. 15
								9	
								8 0	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB NO. _____

BORING No. SS3-1 DATE _____ SHEET 5 OF 5

COMPANY _____

TYPE OF BORING _____ RIG _____

PROJECT _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"	BLOWS / 6"				SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							80	
							1	
							2	
17	82.5	84.0	10	14	4"		3	Sand - GRAY - poorly graded, 100% FINE GRAIN, STRONG REACTION TO HCL. SATURATED
							4	SP
							5	
							6	
							7	
18	87.5	89.0	11	14	7"		8	Silty sand - GRAY, poorly graded moist, STRONG REACTION TO HCL
							9	SM
							90	
							1	
							2	
19	92.5	94.0	11	12	5"		3	Same as sample no. 18
							4	
							5	
							6	
							7	
20	97.5	99.0	13	13	8"		8	Silty clay - GRAY, moist, slight reaction to HCL, slight reaction to HCL, w/ FINE GRAIN SAND LENS
							9	CL
							100	Stopped boring 99.0 WATER 59.6

ENGINEER

Dutch Cone Test

Company IKEC

Probe No. 553-3 Date 8-7-84

Job CLIFTY CREEK FAD

Drillers Macknight - Lambert

Order Lambert

Page 1 of 6 Elevation ^{Elv. 6708'} Higher - 468.09

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	- 0.2			
		.2	C		—			
			C+S		—			
		.4	C		7.8	84	0	0
			C+S		—			
		.6	C	14.0	—	148	0	0
			C+S		—			
		.8	C		9.4	100	5.29	0.053
			C+S	17.0				
		1.0	C		7.0	75	6.96	0.093
			C+S	17.0				
		.2	C		4.2	46	3.20	0.070
			C+S		8.8			
		.4	C		2.8	31	3.20	0.103
			C+S		7.4			
		.6	C		3.4	38	1.39	0.037
			C+S		5.4			
		.8	C		3.2	35	1.81	0.052
			C+S		5.8			
		2.0	C		3.6	40	1.53	0.038
			C+S		5.8			
		.2	C		4.0	44	0.84	0.019
			C+S		5.2			
		.4	C		2.2	25	1.11	0.044
			C+S		3.8			
		.6	C		2.6	29	0.56	0.019
			C+S		3.4			
		.8	C		2.2	25	0.70	0.028
			C+S		3.2			
		3.0	C		1.8	21	0.70	0.033
			C+S		2.8			
		.2	C		2.0	23	0.70	0.030
			C+S		3.0			
		.4	C		2.4	27	0.84	0.030
			C+S		3.6			
		.6	C		3.0	33	0.42	0.013
			C+S		3.6			
		.8	C		1.6	19	0.56	0.031
			C+S		2.4			
		4.0	C		3.0	33	0.28	0.008
			C+S		3.4			
		.2	C		2.8	31	0.84	0.027
			C+S		4.0			
		.4	C		2.6	29	0.70	0.024
			C+S		3.6			
		.6	C		3.2	40	0.28	0.007
			C+S		4.0			
		.8	C		2.0	23	0.56	0.024
			C+S		2.8			
		5	C		2.0	23	0.42	0.018
					2.1			

Dutch Cone Test

Company _____

Probe No. 553-3 Date _____

Job _____

Drillers _____

Order _____

Page 2 of 6 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	-0.2			
		.2	C		1.6	19	0.28	0.015
			C+S		2.0			
		.4	C		3.2	35	1.95	0.056
			C+S		6.0			
		.6	C		7.6	81	0.42	0.005
			C+S		8.2			
		.8	C	20.0		211	0	0
			C+S	20.0				
		6.0	C	12.0		127	4.18	0.033
			C+S	18.0				
		.2	C		9.2	98	4.73	0.048
			C+S	16.0				
		.4	C		8.0	86	1.11	0.013
			C+S		9.6			
		.6	C		9.0	96	0.56	0.008
			C+S		9.8			
		.8	C		9.2	98 106	2.64	0.027
			C+S	13.0				
		7.0	C	10.0		106 96	2.78	0.026
			C+S	14.0				
		.2	C		9.0	96 88	2.09	0.022
			C+S	12.0				
		.4	C		8.2	88 92	1.95	0.022
			C+S	11.0				
		.6	C		8.6	92 59	3.06	0.033
			C+S	13.0				
		.8	C	15.0		159 200	4.18	0.026
			C+S	21.0				
		.0	C	19.0		200 106	2.09	0.010
			C+S	22.0				
		.2	C	10.0		106 88	4.18	0.039
			C+S	16.0				
		.4	C		8.2	88 106	2.64	0.030
			C+S	12.0				
		.6	C	10.0		106	3.48	0.033
			C+S	15.0				
		.8	C	12.0		127	3.48	0.027
			C+S	17.0				
		.0	C	11.0		118	2.78	0.024
			C+S	15.0				
		.2	C		9.2	98	4.04	0.041
			C+S	15.0				
		.4	C					
			C+S					
		.6	C					
			C+S					
		.8	C					
			C+S					
			C					

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. 553-3 DATE _____ SHEET 3 of 6

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	36.0
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								30	
								1	
								2	Ditch came to 31.1 Average Return V-FINE GRAIN SAND.
								3	
1	32.5	34.0	15	16	16	6"		4	SAND - BR. POORLY GRADED, SATURATED, STRONG REACTION TO HCL SP
								5	
								6	
								7	
2	37.5	39.0	6	11	12	13"		8	SILTY SAND - BR. SATURATED, SLIGHT REACTION TO HCL SM
								9	
								10	
								1	
								2	Same as Sample No 2
3	42.5	44.0	11	20	17	16"		3	
								4	
								5	
								6	
								7	
4	47.5	49.0	15	27	28	12		8	SILTY SAND - RUST BR. POORLY GRADED, WET TO SATURATED, STRONG REACTION TO HCL
								9	
								50	SP - SM

ENGINEER _____

Dutch Cone Test

Company _____

Probe No. 553-3 Date _____

Job _____

Drillers _____

Recorder _____

Page 4A of 6 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS								
			C					
		.2	C+S					
			C					
		.4	C+S					
			C					
		.6	C+S					
			C					
		.8	C+S					
			C					
		16.0	C+S					
			C					
		.2	C+S					
			C		7.8	84	2.92	0.035
		.4	C+S	12.0				
			C		6.8	73	2.92	0.040
		.6	C+S	14.0				
			C		4.2	46	2.37	0.052
		.8	C+S		7.6			
			C		7.0	75	1.39	0.018
		.0	C+S		9.0			
			C		8.4	90	2.51	0.028
		.2	C+S	12.0				
			C		6.6	71	3.06	0.043
		.4	C+S	11.0				
			C		2.2	25	3.34	0.134
		.6	C+S		7.0			
			C		7.2	77	0.70	0.009
		.8	C+S		8.2			
			C		4.4	48	2.51	0.052
		.0	C+S		8.0			
			C		4.6	50	2.64	0.053
		.2	C+S		8.4			
			C					
		.4	C+S					
			C					
		.6	C+S					
			C					
		.8	C+S					
			C					
		.0	C+S					
			C					
		.2	C+S					
			C					
		.4	C+S					
			C					
		.6	C+S					
			C					
		.8	C+S					
			C					

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. 593-3 DATE _____ SHEET 4 OF 6

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO								
									50	
									1	
									2	
5	52.5	54.0	7	12	11	14"			3	Silt. Gray. SATURATED, STRONG REACTION TO HCL ML
									4	
									5	DC.
									6	
									7	DUTCH CONE
									8	
									9	
									60	
									1	
									2	
7	62.5	64.0	4	5	7	10"			3	Same AS sample NO 5
									4	
									5	
									6	
									7	
8	67.5	69.0	5	8	9	16			8	Same AS sample NO 5
									9	
									70	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

BORING No. SS3-3 DATE _____ SHEET 5 OF 6

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO							
								70	
								1	
								2	
9	72.5	74.0	8	13	15	12"		3	Clay - Blue-Gray - low plasticity, moist to wet CL
								4	
								5	
								6	
								7	
10	77.5	79.0	7	10	13	16"		8	Same as Sample No. 9
								9	
								80	
								1	
								2	
11	82.5	84.0	6	10	13	16"		3	Same as Sample No. 9
								4	
								5	
								6	
								7	
12	87.5	89.0	6	9	13	16"		8	Sandy Clay - Gray Br. low to med plasticity, wet CL
								9	
								90	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SS3-3 DATE _____ SHEET 6 of 6

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

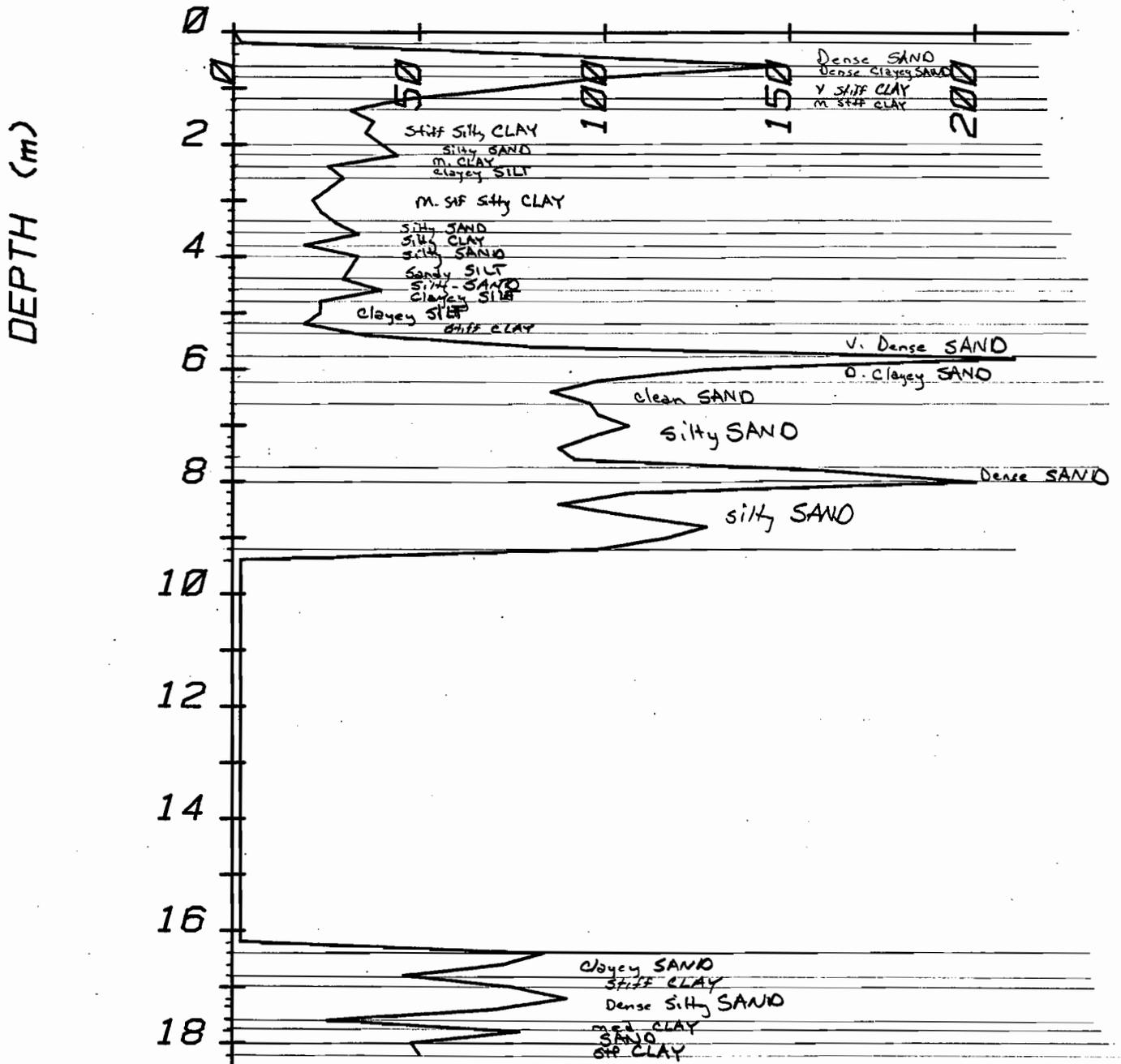
LOCATION OF BORING:

WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								90	
								1	
								2	
13	92.5	94.0	10	11	5	6"		3	Gravelly clay - poorly graded GRAY BR. WET, Broken limestone Fract.
								4	GC
								5	
								6	OVER DRIVE Sample Spoon - Spoon Refusal 95.0
								7	
								8	Stopped boring 95.0
								9	
								0	
								1	
								2	
								3	
								4	
								5	
								6	
								7	
								8	
								9	
								0	

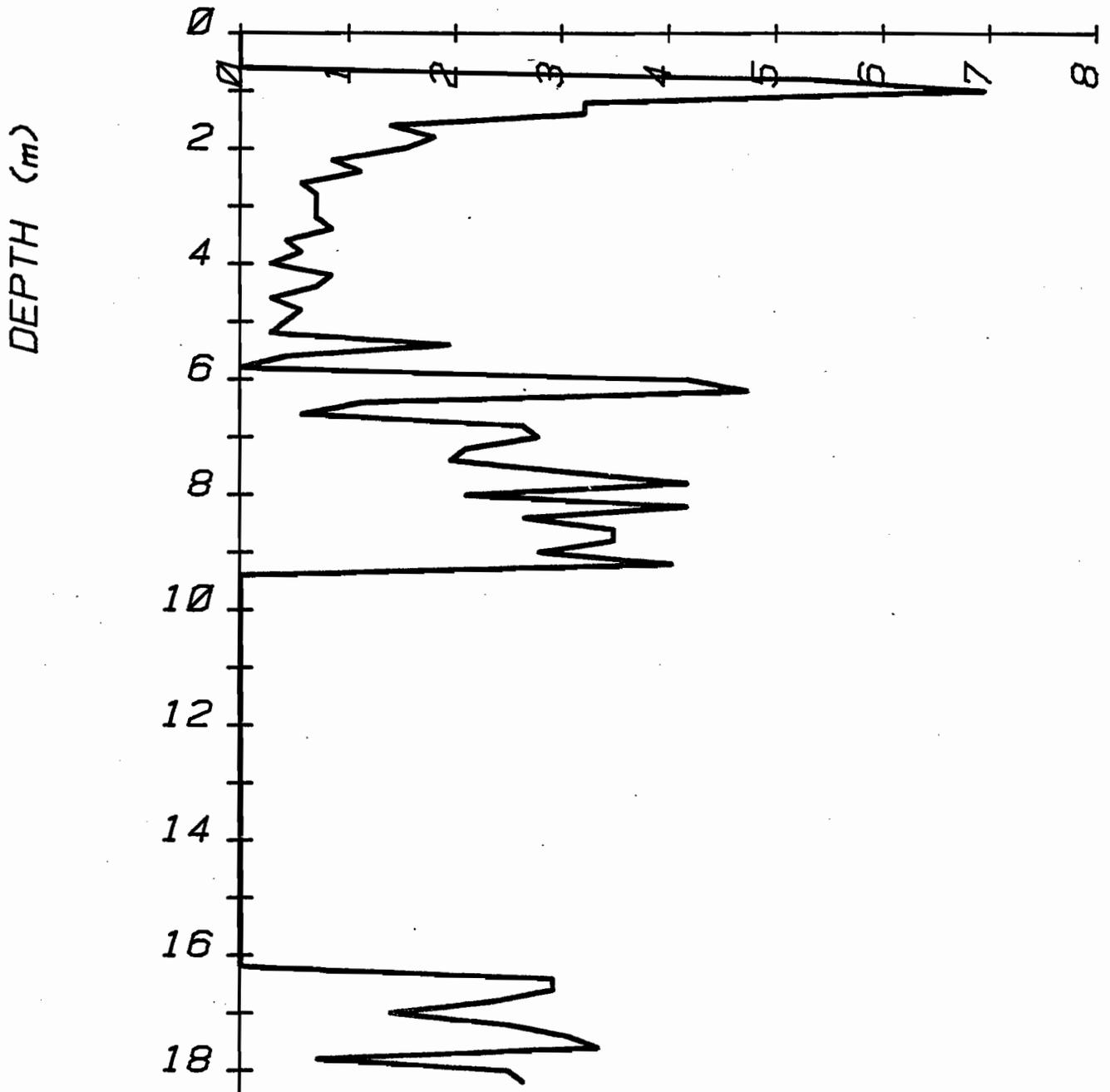
ENGINEER

CONE RESISTANCE (TSF)



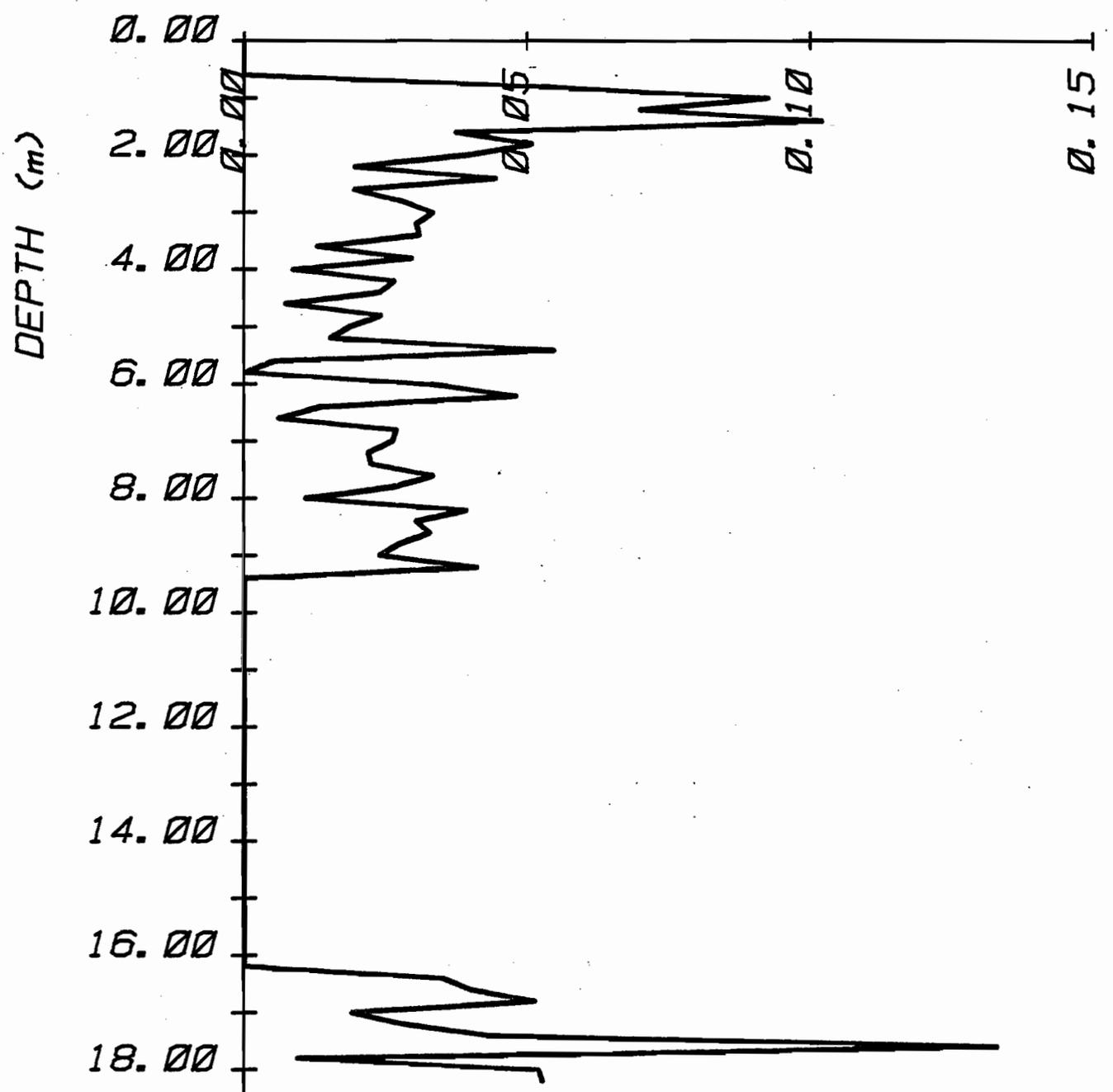
Clifty Creek Flyash Dam
 Dam Raising Study
 PROBE SS3-3

SLEEVE RESISTANCE (TSF)



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS3-3

FRICTION RATIO



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS3-3

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB NO. _____

COMPANY _____

PROJECT _____

BORING No. SS3-4 DATE _____ SHEET 4 of 7

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLWS / 6"	BLWS / 6"				SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							60	
							1	
							2	
							3	
13	63.0	64.5	6	9	10	16"		Clay - Blue Gray - moist, low to med. plasticity
							4	
							5	CL
							6	
							7	
							8	
14	68.0	69.5	7	9	12	16"		SAME AS Sample No. 13
							9	
							70	
							1	
							2	
							3	
15	73.0	74.5	15	9	11	12"		GRAVELLY SAND CLAY - Blue Gray SATURATED - Limestone Frag.
							4	
							5	CL
							6	
							7	
							8	
16	77.0	79.5	8	16	18	13"		SAME AS Sample No. 15 (yellowish Br)
							9	
							80	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company EKEC

Project CLIFTY CREEK FAD

Location of Boring: _____

Water Level	
Time	
Date	

Boring No. SS3-4 Date _____ Sheet 5 of _____

Type of Boring Coec Rig R-61

Casing used _____ Size _____ Drilling mud used _____

Boring begun 8-14-84 Boring completed 8-14-84

Ground Elevation _____ referred to _____

Field Party: Roush - Lambert

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations. Depth is water lost. Observed fluctuations in water level, notes on drilling ease, etc.
		80.9 85.3	45%	3.9	81		80.9 - 85.3 Gray HARD limestone w/ GRAY CALAREOUS SHALE LAYERS
					82		
					83		
					84		
					85		
		85.3 87.5	19%	2.2			
					86		Gray Calcareous shale
							Gray Calcareous shale
					87		

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

553-4

Boring No. _____ Date _____ Sheet 6 of 7

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Date _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
		87.5 95.3	64%	7.8			Rock Type, Color, Quality, Drilling observations, Depth, water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					88		Gray calcareous shale
					89		" " "
					90		" " "
					91		
					92		
					93		Gray calcareous shale
					94		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No: _____

Company _____

Project _____

Location of Boring:	
Water Level	
Time	
Date	

Boring No. SS3-4 Date _____ Sheet 7 of 7
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____
 Date _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					95		
		95.3 100.3	49%	5.0			
							Gamy calcareous shale
					96		" " "
					97		" " "
					98		" " "
					99		
					100		
							stopped boring 100.3

Dutch Cone Test

Company TKEC
 Job CLIFTY CREEK FAD
 Order Lambert

Probe No. SS3-S Date 8-6-84
 Drillers MacKwight - Lambert
 Page 1 of 5 Elevation 449.34

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	-0.2			
		.2	C		1			
			C+S		2			
		.4	C		2.2			
			C+S		3			
		.6	C		2.2			
			C+S		3.2			
		.8	C		1.8			
			C+S		2.2			
		1.0	C		1.0			
			C+S		2.8			
		.2	C		.6			
			C+S		1.8			
		.4	C		.6			
			C+S		1.6			
		.6	C		.6			
			C+S		1.6			
		.8	C		1.8			
			C+S		2.4			
		2.0	C		1.0			
			C+S		1.8			
		.2	C		1.6			
			C+S		2.4			
		.4	C		1.4			
			C+S		2.8			
		.6	C		1.4			
			C+S		2.6			
		.8	C		1.0			
			C+S		2.0			
		3.0	C		1.2			
			C+S		1.8			
		.2	C		1.6			
			C+S		2.0			
		.4	C		1.2			
			C+S		2.0			
		.6	C		1.2			
			C+S		2.0			
		.8	C		1.0			
			C+S		1.8			
		4.0	C		1.2			
			C+S		2.0			
		.2	C		1.4			
			C+S		2.2			
		.4	C		1.4			
			C+S		2.2			
		.6	C		1.2			
			C+S		2.0			
		.8	C		1.2			
			C+S		2.0			
			C		1.2			

Boring 8' NORTH OF STAKE

Dutch Cone Test

Company _____

Probe No. SS3-5 Date _____

Job _____

Drillers _____

Order _____

Page 2 of 5 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	-0.2			
		.2	C		1.2			
		.2	C+S		2.0			
		.4	C		1.2			
		.4	C+S		2.0			
		.6	C		1.0			
		.6	C+S		1.8			
		.8	C		1.2			
		.8	C+S		1.8			
		6.0	C		.8			
		6.0	C+S		1.8			
		.2	C		0.8			
		.2	C+S		1.4			
		.4	C		.2			
		.4	C+S		1.6			
		.6	C		.2			
		.6	C+S		1.4			
		.8	C		0.2			
		.8	C+S		2.0			
		7.0	C		0.2			
		7.0	C+S		1.0			
		.2	C		0.6			
		.2	C+S		1.8			
		.4	C		0.2			
		.4	C+S		0.6			
		.6	C		0.2			
		.6	C+S		1.6			
		.8	C		Void			Gravel Layer?
		.8	C+S		Void			
		8.0	C		2.2			
		8.0	C+S		3.6			
		.2	C		Void			
		.2	C+S		3.8			
		.4	C		4.4			
		.4	C+S	15.0				
		.6	C		2.8			
		.6	C+S		5.6			
		.8	C		5.6			
		.8	C+S		7.0			
		9.0	C		5.6			
		9.0	C+S		7.2			
		.2	C		5.4			
		.2	C+S		8.0			
		.4	C		6.0			
		.4	C+S		7.4			
		.6	C		5.2			
		.6	C+S		6.6			
		.8	C		5.6			
		.8	C+S		4.2			
		10.0	C		4.0			
		10.0	C+S		4.1			

Dutch Cone Test

Company _____

Probe No. SS3-5 Date _____

Job _____

Drillers _____

Order _____

Page 3 of 5 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	-0.2			
			C		4.4			
		.2	C+S		7.4			
			C		8.0			
		.4	C+S		8.4			
			C		7.6			
		.6	C+S		8.4			
			C		8.0			
		.8	C+S		8.6			
			C		8.2			
		11.0	C+S		9.0			
			C		8.4			
		.2	C+S		5.4			
			C		1.8			
		.4	C+S		3.6			
			C		5.4			
		.6	C+S		6.4			
			C		2.4			
		.8	C+S		3.8			
			C		0.8			
		12.0	C+S		3.2			
			C		0.2			
		.2	C+S		2.2			
			C		1.0			
		.4	C+S		1.6			
			C		0.2			
		.6	C+S		1.6			
			C		0.2			
		.8	C+S		2.0			
			C		1.0			
		13.0	C+S		1.8			
			C		0.2			
		.2	C+S		2.0			
			C		0.2			
		.4	C+S		2.0			
			C		0.2			
		.6	C+S		2.0			
			C		0.2			
		.8	C+S		1.8			
			C		0.2			
		14.0	C+S		2.0			
			C		1.0			
		.2	C+S		2.2			
			C		0.2			
		.4	C+S		2.2			
			C		0.2			
		.6	C+S		2.2			
			C		0.2			
		.8	C+S		2.0			
			C		0.2			
		15.0	C+S		2.0			

Dutch Cone Test

Company _____

Probe No. 553-5 Date _____

Job _____

Drillers _____

Order _____

Page 4 of 5 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	-0.2			
		.2	C		0.2			
			C+S		2.0			
		.4	C		0.2			
			C+S		1.8			
		.6	C		0.2			
			C+S		2.2			
		.8	C		0.2			
			C+S		2.4			
		16.0	C		0.2			
			C+S		2.0			
		.2	C		0.2			
			C+S		2.4			
		.4	C		1.2			
			C+S		2.2			
		.6	C		1.2			
			C+S		2.4			
		.8	C		0.2			
			C+S		2.2			
		17.0	C		0.2			
			C+S		2.0			
		.2	C		1.2			
			C+S		2.2			
		.4	C		0.2			
			C+S		2.0			
		.6	C		0.2			
			C+S		2.4			
		.8	C		0.2			
			C+S		2.6			
		18.0	C		1.6			
			C+S		2.6			
		.2	C		2.0			
			C+S		3.6			
		.4	C		3.6			
			C+S		5.2			
		.6	C		3.6			
			C+S		6.2			
		.8	C		2.4			
			C+S		5.4			
		19.0	C		0.2			
			C+S		4.2			
		.2	C		2.4			
			C+S		4.4			
		.4	C		3.0			
			C+S		4.4			
		.6	C		2.6			
			C+S		5.2			
		.8	C		2.6			
			C+S		5.0			
		20.0	C		2.2			
			C+S		1.2			

Dutch Cone Test

Company _____

Probe No. 553-5 Date _____

Order _____

Drillers _____

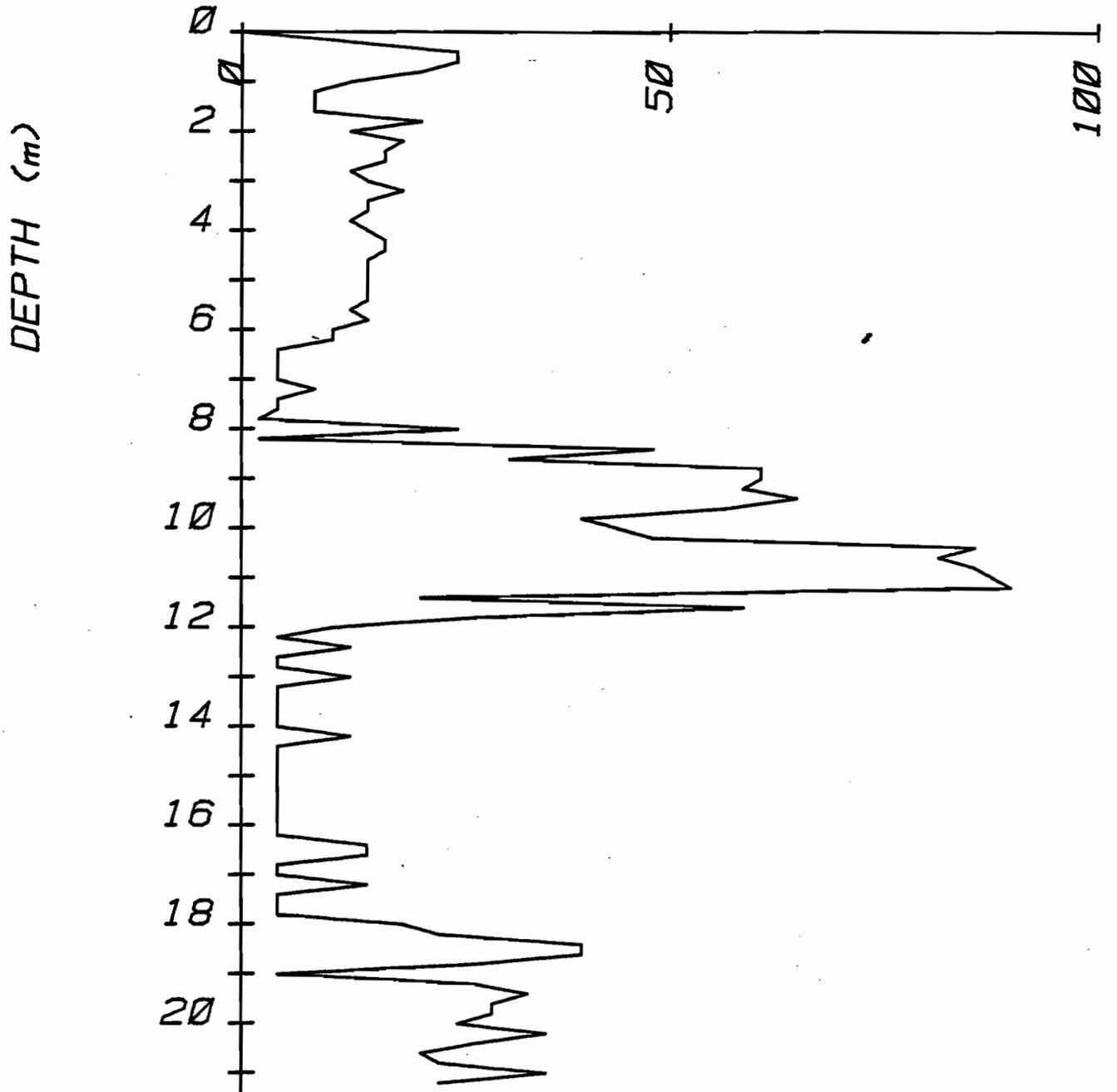
Order _____

Page 5 of 5 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST.	SLEEVE RESIST.	FRICTION RATIO
				HIGH	LOW			
GAGE ZERO READINGS				0.0	- 0.2	(1000N)	(1000N)	(%)
		.2	C		3.2			
		.2	C+S		5.0			
		.4	C		2.4			
		.4	C+S		4.4			
		.6	C		1.8			
		.6	C+S		3.6			
		.8	C		2.0			
		.8	C+S		2.8			
		21.0	C		3.2			
		21.0	C+S		4.6			
		.2	C		2.0			
		.2	C+S		4.4			
		.4	C					
		.4	C+S					
		.6	C					
		.6	C+S					
		.8	C					
		.8	C+S					
		.0	C					
		.0	C+S					
		.2	C					
		.2	C+S					
		.4	C					
		.4	C+S					
		.6	C					
		.6	C+S					
		.8	C					
		.8	C+S					
		.0	C					
		.0	C+S					
		.2	C					
		.2	C+S					
		.4	C					
		.4	C+S					
		.6	C					
		.6	C+S					
		.8	C					
		.8	C+S					
		.0	C					
		.0	C+S					
		.2	C					
		.2	C+S					
		.4	C					
		.4	C+S					
		.6	C					
		.6	C+S					
		.8	C					
		.8	C+S					
		.0	C					

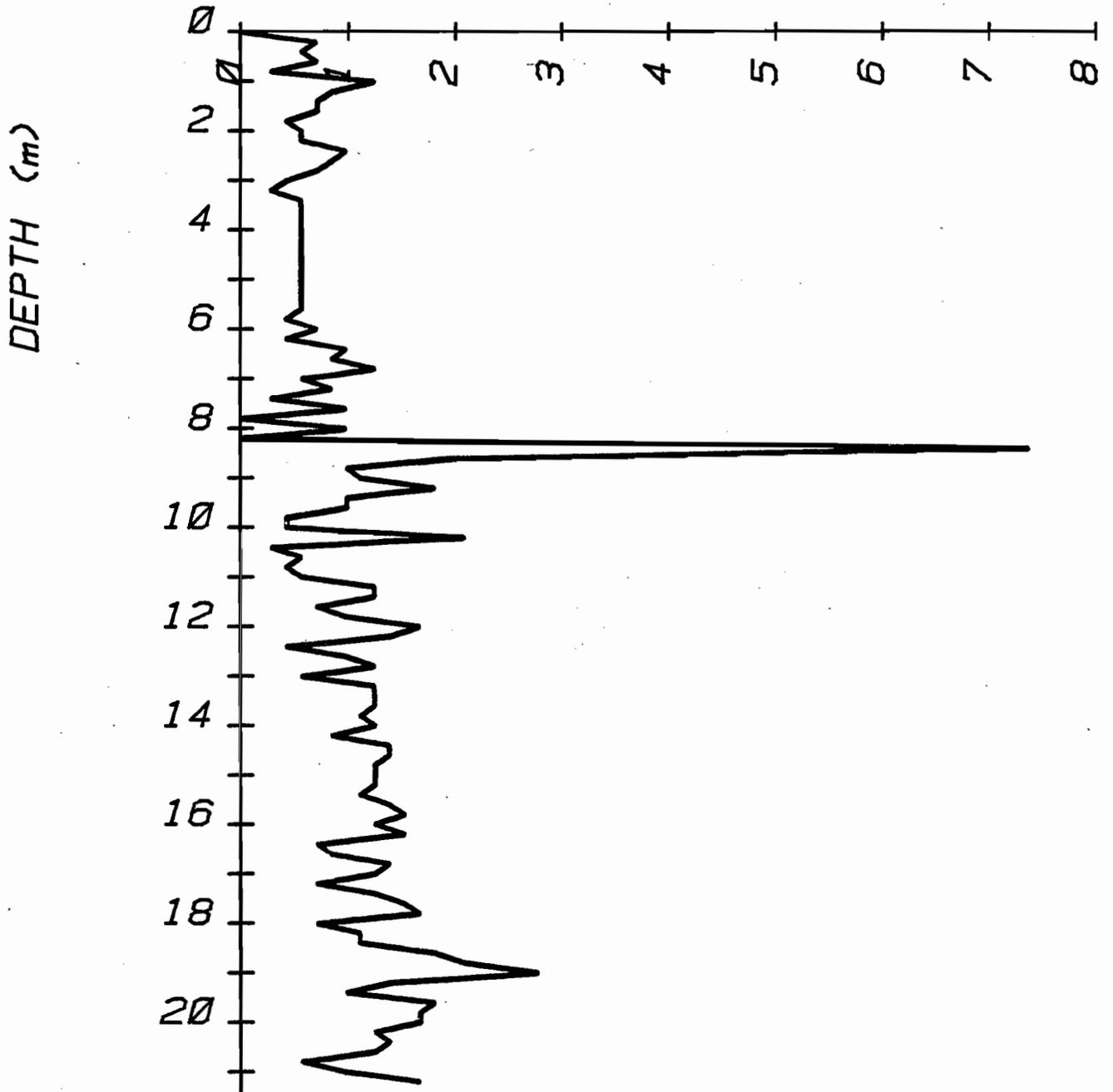
Stopped boring, 21.2 M

CONE RESISTANCE (TSF)



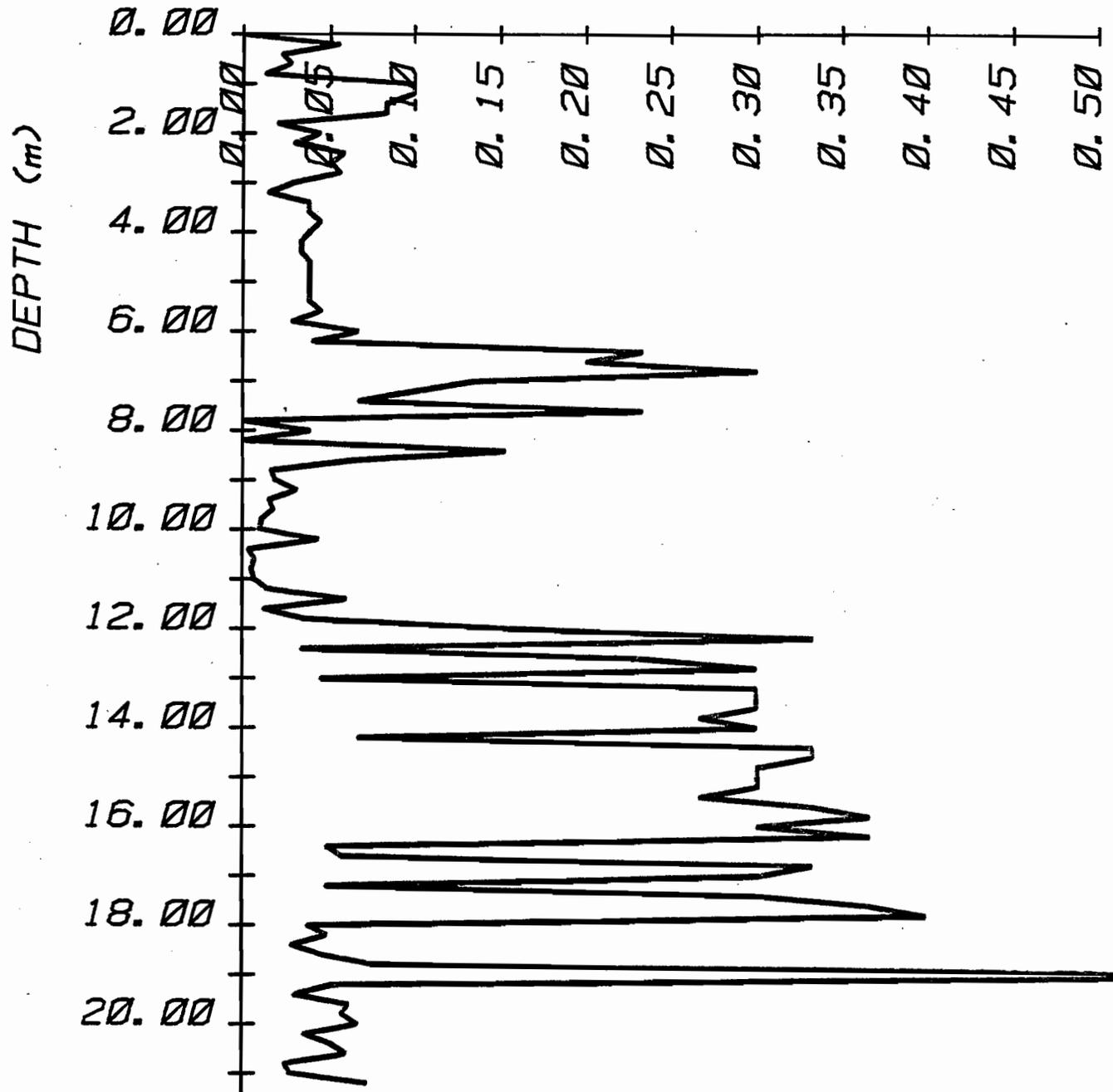
Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS3-5

SLEEVE RESISTANCE (TSF)



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS3-5

FRICITION RATIO



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS3-5

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

COMPANY _____

PROJECT _____

BORING No. SZ-1 DATE _____ SHEET 2 of 8

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								20	
								1	
								2	
								3	
5	23.0	24.5	5	6	3	16"		4	Clayey silt - Br. moist to wet w/ Fine Grain Sand lens
								5	ML
								6	
								7	
								8	
6	28.0	29.5	10	12	11	16"		9	Clayey sand - Br. moist to wet poorly graded, moderate reaction to HCl
								30	Water 29.4 River Elev. SC
								1	
								2	
								3	
7	33.0	34.5	10	11	13	16"		4	Same as Sample No 6
								5	
								6	
								7	
								8	
8	38.0	39.5	8	11	11	16"		9	Silty sand - Gray saturated, poorly graded, strong reaction to HCl
								40	SM

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

BORING No. SE-1 DATE _____ SHEET 3 OF 8

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO								
									40	
									1	
									2	
									3	
9	43.0	44.5	6	8	10	15"			4	Sandy silt - Gray. SATURATED, slight REACTION TO HCL
									5	ML
									6	
									7	
									8	SILT - GRAY. SATURATED, slight REACTION TO HCL
10	48.0	49.5	8	10	18	14"			9	ML
									50	
									1	
									2	
									3	
11	53.0	54.5	10	11	11	16"			4	Same as sample No. 9
									5	
									6	
									7	
									8	Same as sample No. 9
12	58.0	59.5	7	7	5	13"			9	
									60	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. SI-1 DATE _____ SHEET 4 of 8

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

_____ DATUM

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								6 0	
								1	
								2	
								3	
13	68.0	64.5	3	4	5	5"		4	Silt - Gray - SATURATED, slight Reaction to HCL
								5	ML
								6	
								7	
								8	
14	68.0	69.5	5	7	7	16"		9	clayey silt - Gray SATURATED, slight plasticity, slight Reaction to HCL
								7 0	ML
								1	
								2	
								3	
15	73.0	74.5	8	13	15	16"		4	Same AS Sample NO 14
								5	
								6	
								7	
								8	
	78.0	78.0	59 1/2			0		9	
								8 0	

ENGINEER _____

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

Boring No. SI-1 Date 8-9-84 Sheet 5 of 8

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Datum

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		78.0					WATER ELEV - 8-9-84 16.0
		83.5	64%	4.8	Core lost ?		
					79		78.0 - 99.9 - Gray Hard limestone w/ layers of calcareous clay shale
							Shale Gray
							Shale Gray
					80		
							shale GRAY
					81		
							Shale Gray
					82		
		83.5			83		Shale GRAY
		84.9	47%	1.4			shale GRAY
					84		Shale GRAY
		84.9					Shale GRAY
		99.9	56%	10.0	85		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

Boring No. 551 Date _____ Sheet 6 of 9

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RGD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION <small>Rock Type, Color, Quality, Drilling observations, Dept. water lost, Observed fluctuations in water level, notes on drilling ease, etc.</small>
							Shale Gray
					86		Shale Gray
							Shale Gray
					87		Shale Gray
							Shale GRAY Lens
					88		Shale GRAY Lens
							Shale GRAY
					89		Shale GRAY
							Shale GRAY
					90		Shale GRAY
							Shale GRAY
					91		Shale GRAY
							Shale GRAY
					92		Shale GRAY

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Boring No. SI-1 Date _____ Sheet 7 of 8

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum

Field Party: _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
					93		Shale GRAY
					94		Shale GRAY
	94.9 99.9		36% 5.0				Shale CLAY
					95		Shale GRAY
							Shale GRAY
					96		Shale GRAY
							Shale GRAY
					97		Shale GRAY
							Shale GRAY
					98		Shale GRAY
							Shale GRAY
					99		Shale GRAY

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

BORING No. SS4-1 DATE 7-11-84 SHEET 1 of 5

TYPE OF BORING SPT RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 7-11-84 BORING COMPLETED 7-11-84

GROUND ELEVATION 506.0 REFERRED TO _____ DATUM _____

FIELD PARTY: Roush-Lambert

PANY IKEC
PROJECT Cherty Creek Fly Ash Dam

LOCATION OF BORING:	
WATER LEVEL	<u>57.8</u>
TIME	<u>7-11-84</u>
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								0	
								1	
1	2.5	4.0	2	2	3	5"		2	
								3	Clay - Br, moist, low to med. plasticity w/ trace of gravel CL
								4	
								5	
								6	
2	7.5	9.0	5	8	15	8"		7	
								8	Clayey silt - Gray Br. moist, low plasticity ML
								9	
								10	
								11	Original Ground?
								12	
3	12.5	14.0	5	6	7	6"		13	
								14	Silty clay - Br. Br. low to med. plasticity moist, moderate reaction to HCL Parts Organics CL
								15	
								16	
								17	
4	17.5	19.0	8	12	12	12"		18	
								19	Silty Clay - Br. low plasticity moist, strong reaction to HCL CL
								20	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____
COMPANY _____
PROJECT _____

BORING No. SS4-1 DATE _____ SHEET 2 OF 5
TYPE OF BORING _____ RIG _____
CASING USED _____ SIZE _____ DRILLING MUD USED _____
BORING BEGUN _____ BORING COMPLETED _____
GROUND ELEVATION _____ REFERRED TO _____ DATE _____
FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"	BLOWS / 6"				SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES, BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							20	
							1	
							2	
5	22.5	24.0	7	7	10	13"		Clay - Br. moist, low to med plasticity
							3	cl
							4	
							5	
							6	
							7	
6	27.5	29.0	6	7	12	15"		Same as Sample No. 5
							8	
							9	
							30	
							1	
							2	
7	32.5	34.0	5	8	12	16"		Silty Clay - Br + Gray mottled, low plasticity, moist
							3	cl
							4	
							5	
							6	
							7	
8	37.5	39.0	6	8	8	15"		Clay - Br. low to med plasticity moist
							8	
							9	
							40	

ENGINEER

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

BORING No. SS4-1 DATE _____ SHEET 3 OF 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING: _____

WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								4 0	
								1	
9	42.5	44.0	6	8	9	12"		2	SILT. BR. slight plasticity moist
								3	ML
								4	
								5	
								6	
								7	
10	42.5	49.0	7	10	11	15"		8	Silty sand. BR. poorly graded, 100% FINE GRAIN, SLIGHT REACTION TO HCL.
								9	SM
								5 0	Piezometer
								1	
								2	
11	52.5	54.0	8	12	11	16"		3	Same as Sample No. 10 (wet)
								4	
								5	
								6	
								7	
12	52.5	59.0	7	9	9	16"		8	Same as Sample No 10
								9	
								6 0	

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

BORING No. SS4-1 DATE _____ SHEET 4 of 5

COMPANY _____

TYPE OF BORING _____ RIG _____

PROJECT _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM

FIELD PARTY: _____

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO								
								6 0		
								1		
								2		
13	62.5	64.0	8	10	14	16"		3		Silty Sand - multi-colored BR. wet 100% Fine Grain, moderate Reaction to HCL
								4		SM
								5		
								6		
								7		
14	67.5	69.0	8	12	14	16"		8		Silty Clay - Gray - low to med. plasticity - slight Reaction to HCL moist
								9		CL
								7 0		
								1		
								2		
15	72.5	74.0	2	2	3	15"		3		Clayey Sand - Gray - Saturated, Partly Graded, 100% Fine Grain, slight Reaction to HCL
								4		SC
								5		
								6		
								7		
16	77.5	79.0	8	9	13	16"		8		Silt - Gray Saturated, slight Reaction to HCL
								9		ML
								8 0		

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____
COMPANY _____
PROJECT _____

BORING No. 554-1 DATE _____ SHEET 5 OF 5
TYPE OF BORING _____ RIG _____
CASING USED _____ SIZE _____ DRILLING MUD USED _____
BORING BEGUN _____ BORING COMPLETED _____
GROUND ELEVATION _____ REFERRED TO _____ DATUM _____
FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								0	
								1	
								2	
17	82.5	84.0	10	14	15	13"		3	Sandy Silt - Gray, wet slight Reaction to HCL, 100% FINE GRAIN TRACE OF ORG. MAT.
								4	
								5	
								6	
								7	
18	87.5	88.0	9	10	20	14"		8	Same as Sample No. 17 w/ a FINE GRAIN SAND LAYER.
								9	
								0	
								1	
								2	
19	92.5	94.0	11	15	15	12"		3	Same as Sample No. 17 w/ a FINE GRAIN SAND LAYER
								4	
								5	
								6	Stopped boring, 94.0
								7	
								8	
								9	
								0	

Dutch Cone Test

Company IK EC

Probe No. SS4-3 Date 8-7-84

Job CLIFTY CREEK FAD

Drillers MAC KNIGHT - LAMBERT

Recorder LAMBERT

Page 1 of 2 Elevation 459.84

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	-0.2			
		.2	C		—			
		.2	C+S		—			
		.4	C		7.0			
		.4	C+S		—			
		.6	C		7.8			
		.6	C+S		9.2			
		.8	C		6.8			
		.8	C+S		9.4			
		1.0	C		5.8			
		1.0	C+S		9.0			
		.2	C		4.2			
		.2	C+S		8.2			
		.4	C		3.2			
		.4	C+S		6.0			
		.6	C		5.0			
		.6	C+S		6.2			
		.8	C		3.2			
		.8	C+S		5.2			
		2.0	C		3.6			
		2.0	C+S		4.8			
		.2	C		3.2			
		.2	C+S		5.0			
		.4	C		2.8			
		.4	C+S		4.4			
		.6	C		1.8			
		.6	C+S		3.4			
		.8	C		1.6			
		.8	C+S		2.6			
		3.0	C		1.6			
		3.0	C+S		2.4			
		.2	C		1.4			
		.2	C+S		1.8			
		.4	C		1.2			
		.4	C+S		1.6			
		.6	C		1.4			
		.6	C+S		2.2			
		.8	C		2.4			
		.8	C+S		3.2			
		4.0	C		3.2			
		4.0	C+S		4.4			
		.2	C		3.8			
		.2	C+S		5.0			
		.4	C		4.4			
		.4	C+S		6.6			
		.6	C		6.0			
		.6	C+S		7.4			
		.8	C		7.4			
		.8	C+S		7.6			
			C		9.4			
					9.8			

Dutch Cone Test

Company _____

Probe No. 554-3 Date _____

Job _____

Drillers _____

Recorder _____

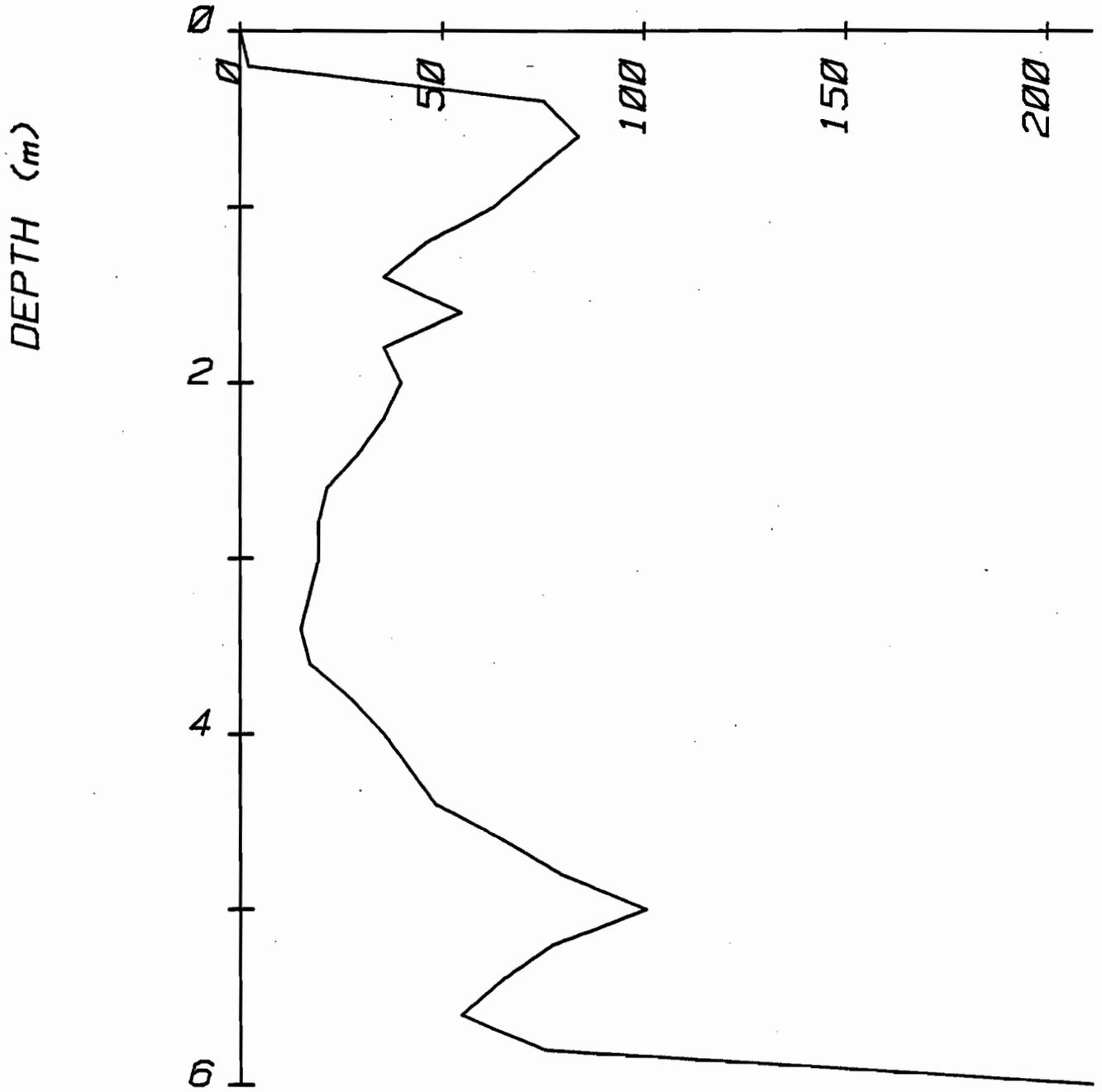
Page 2 of 2 Elevation _____

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	-0.2			
		.2	C		7.2			
			C+S		9.2			
		.4	C		6.0			
			C+S		9.8			
		.6	C		5.0			
			C+S		8.2			
		.8	C		7.0			
			C+S		8.8			
		6.0	C	20.0				
			C+S	40.0				
		.2	C					
			C+S					
		.4	C					
			C+S					
		.6	C					
			C+S					
		.8	C					
			C+S					
		.0	C					
			C+S					
		.2	C					
			C+S					
		.4	C					
			C+S					
		.6	C					
			C+S					
		.8	C					
			C+S					
		.0	C					
			C+S					
		.2	C					
			C+S					
		.4	C					
			C+S					
		.6	C					
			C+S					
		.8	C					
			C+S					
		.0	C					
			C+S					
		.2	C					
			C+S					
		.4	C					
			C+S					
		.6	C					
			C+S					
		.8	C					
			C+S					
		.0	C					
			C+S					

Augered From 19.68 To 23.5

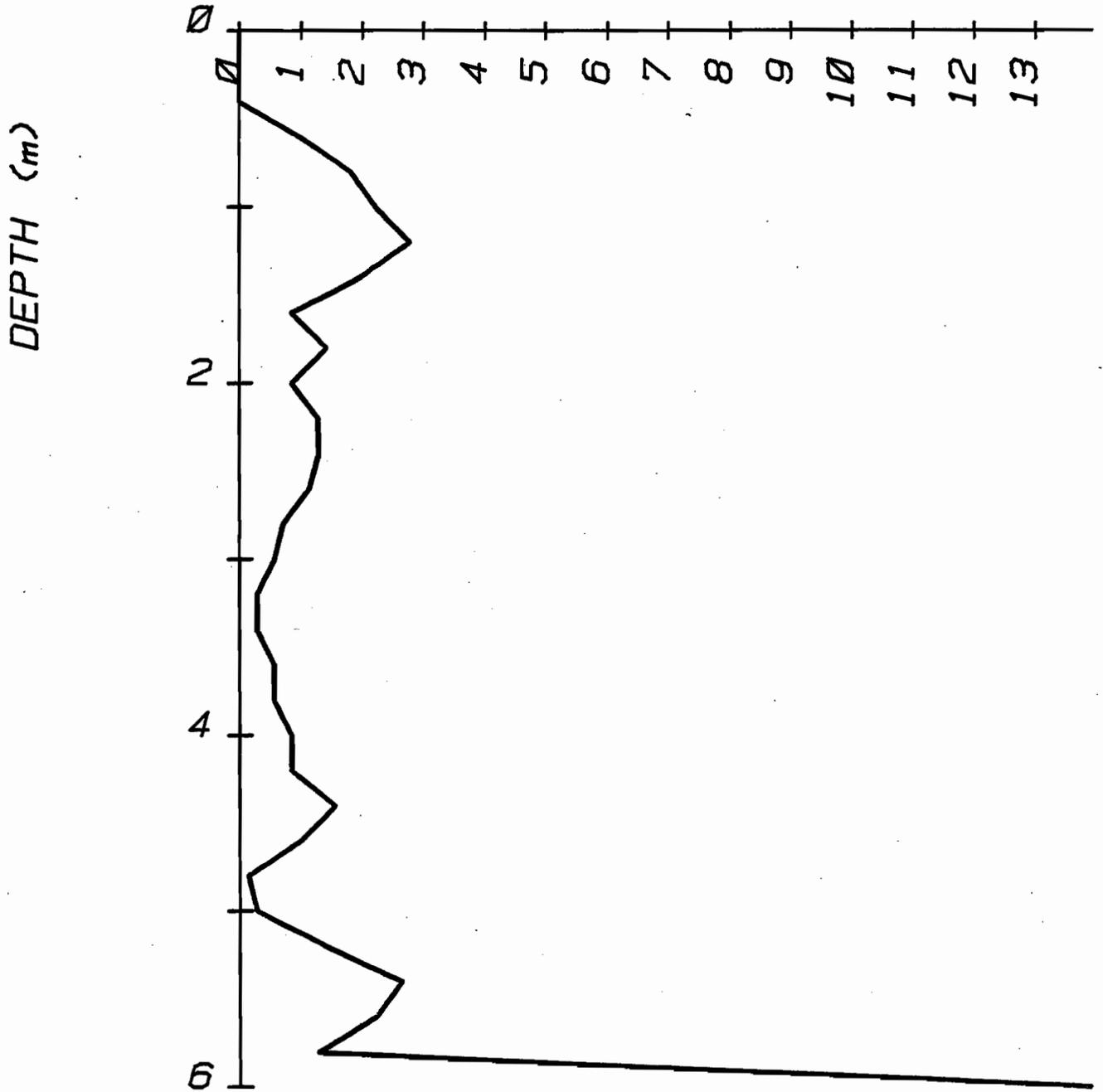
Cobbles - Augered Re. From 23.5

CONE RESISTANCE (TSF)



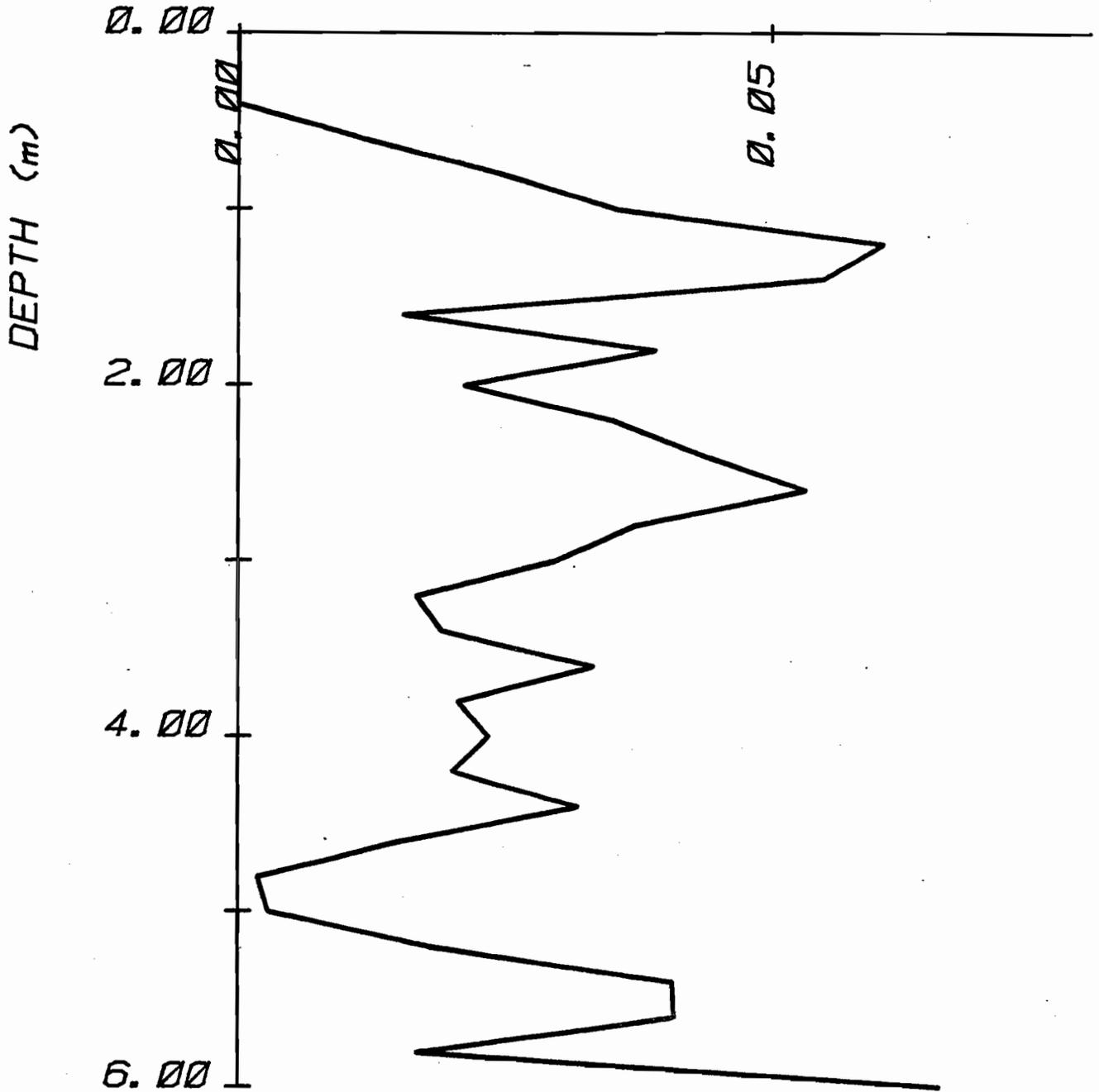
Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS4-3

SLEEVE RESISTANCE (TSF)



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS4-3

FRICITION RATIO



Clifty Creek Flyash Dam
Dam Raising Study
PROBESS4-3

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY IKEC
PROJECT CLIFTY CREEK Fly Ash DAM

BORING No. SS44 DATE 7-26-84 SHEET 1 OF 6

TYPE OF BORING SPT-CORR RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 7-26-84 BORING COMPLETED _____

GROUND ELEVATION 450.70 ± REFERRED TO _____ DATUM

FIELD PARTY: Rough + Lambert

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO								SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
									0	Boring moved to edge of woods
									1	Water - 7-31-84 12.0'
									2	
									3	
1	3.0	4.5	4	6	7	16"			4	Sandy clay - Reddish Br. moist, low to med. plasticity
									5	CL
									6	
									7	
									8	Same as sample NO 1
									9	
									10	
									11	
									12	
									13	
3	13.0	14.5	1	1	1	14"			14	Sandy silt - Br. saturated
									15	ML-EM
									16	
									17	
									18	
									19	
									20	
									21	
									22	
									23	
									24	
									25	
									26	
									27	
									28	
									29	
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									31	
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									95	
									96	
									97	
									98	
									99	
									100	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

BORING No. SS4-4 DATE _____ SHEET 2 OF 6

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE No.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								20	
								1	
								2	
								3	
5	23.0	23.2	50			2"		3	Lime stone frag.
								4	
								5	
								6	
								7	
								8	
6	28.0	29.5	9	12	17	7"		8	Clayey sand + GRAVEL - Gray BR.
								9	SATURATED, STRONG REACTION TO HCL
								30	poorly graded, 1" max size GRAVEL
									consist of lime stone frag.
								1	GC
								2	
								3	
7	33.0	33.1	50			1"		3	Gray calcareous shale + 1 1/4" piece of
								4	lime stone.
								5	
								6	
								7	
								8	
								9	
								0	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Boring No. SS4-4 Date 7-31-84 Sheet 3 of 6

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Date _____

Field Party: _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth, water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		33.3 34.8	28%	1.4			33.3-54.8 - Gray limestone w/ calcarenous shale
					34		Badly Broken
		34.8 39.8	51%	4.8	35		Core Lost ?
					36		Badly Broken
					37		
					38		
					39		
					40		Badly Broken

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring:	
Water Level	
Time	
Date	

Boring No. 554-4 Date _____ Sheet 4 of 6
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____
 Date _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RDD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
		39.8 44.8	60%	5.0			
					41		
					42		
					43		
					44		
		44.8 54.8	59%	10.0	45		
					46		Shale GRAY
							Shale GRAY
					47		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level	
Time	
Date	

Boring No. 559-4 Date _____ Sheet 5 of 1

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Da

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, note on drilling ease, etc.
					48		
					49		
					50		
							Shale Grey
					51		
					52		
							Shale Grey
					53		
					54		

JOB NO. _____

COMPANY IKEC

PROJECT Clifty CK Flyash Storage Area

BORING NO. 5544 DATE 7/26/84 SHEET 1 OF _____

TYPE OF BORING Split spoon RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 7/26/84 BORING COMPLETED _____

GROUND ELEVATION 450.70 REFERRED TO _____ DATUM _____

FIELD PARTY: CONNIE & MIKE

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO	BLOWS / 6"							
									0	
									1	Moved Boring to edge of woods
									2	
									3	
1	3.0	4.5	4	6	7	16			4	reddish brown clayey silt
									5	
									6	
									7	
									8	
2	8.0	9.5	3	4	3	14			9	moist reddish brown clayey silt
									0	
									1	
									2	
									3	
3	13.0	14.5	1	1	1	14			4	wet reddish brown clayey silt
									5	
									6	
									7	
									8	
	18.0	19.5	1	2	2	16			9	wet reddish brown clayey silt with rock frags
									0	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY IKEC

PROJECT Clifty CK

BORING No. 554-4 DATE 7/26/84 SHEET 2 OF _____

TYPE OF BORING splitspear RIG B61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 7/26/84 BORING COMPLETED _____

GROUND ELEVATION 450.70 REFERRED TO _____ DATUM _____

FIELD PARTY: Coonie + M.KE

LOCATION OF BORING:

WATER LEVEL

TIME

DATE

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								0	
								1	
								2	
								3	
5	23.0	24.5	50 2			2		4	wet Limestone
								5	
								6	
								7	
								8	
6	28.0	29.5	9	12	17	7		9	wet brownish gray clay with rock frag
								10	
								1	
								2	
								3	
7	33.0	34.5	30 1			1		4	gray clay shale + limestone
								5	Sealed Augers At 33.3'
								6	
								7	
								8	
								9	
								10	
								11	
								12	
								13	
								14	
								15	
								16	
								17	
								18	
								19	
								20	
								21	
								22	
								23	
								24	
								25	
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								27	
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								29	
								30	
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								32	
								33	
								34	
								35	
								36	
								37	
								38	
								39	
								40	
								41	
								42	
								43	
								44	
								45	
								46	
								47	
								48	
								49	
								50	

ENGINEER

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. 555-1 DATE _____ SHEET 2 OF 7

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								20	
								1	
5	22.5	24.0	8	15	10	12"		2	Silty Sand - yellowish Br., Poorly Graded, 100% Fine Grain, moist SM
								3	
								4	
								5	
								6	
6	17.5	29.3	10	50/3		6"		7	Clay - yellowish Br. moist, low to med. plasticity, strong reaction to HCL
								8	
								9	
								30	
								1	
								2	Same as Sample No 6
7	32.5	33.2	10	50/2		1"		3	
								4	
								5	
								6	
								7	Same as Sample No. 6 w/ small amount of weathered soft lime stone frag.
8	37.5	38.0	50			5"		8	
								9	
								40	Sandy

ENGINEER _____

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

Boring No. SS51 Date _____ Sheet 3 of 7

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
		43.4-45.2		1.4			
					44		43.4 - 43.6 BR. SILTY CLAY w/ LIME STONE FRAG.
							43.6 - 44.4 GRAY HARD LIMESTONE
					45		CORE LOST THIS AREA? BR. SILTY CLAY + BROKEN LIMESTONE FRAG.
		45.2-55.2	7%	4.3			45.5 LOST WATER CORE LOST? .3 LIMESTONE 1.5 BR SILTY CLAY w/ LIME STONE FRAG. 2.2 GRAY HARD LIMESTONE
					46		
					47		
					48		
					49		
					50		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring:	
Water Level	
Time	
Date	

Boring No. 555-1 Date _____ Sheet 4 of _____
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____ Datum _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
					51		
					52		
					53		
					54		
					55		
		55.2-60.8	2 1/8"	4.2			Core lost?
							.4 GRAY HARD limestone
							.9 SOFT GRAY CALCAREOUS SHALE
					56		.9 GRAY HARD limestone
							.6 SOFT GRAY CALCAREOUS SHALE
							.8 GRAY HARD limestone
							.6 SOFT CALCAREOUS SHALE
					57		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

555-1

Boring No. _____ Date _____ Sheet 5 of 7
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____
 _____ Date _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (In feet)	RDD	Recovery (In feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth, water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					58		
					59		
					60		
		60.8-65.2	62%	4.3			
					61		60.8-65.2 GRAY HARD LIMESTONE w/ LAMINATED LAYERS OF CALcareous SHALE
					62		62.1 1/2' SEAM OF GRAY CALcareous SHALE
					Below 63		62.7-62.9 SEAM OF GRAY CALcareous SHALE
					64		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. ⁵⁵⁵⁻¹ _____ Date _____ Sheet 6 of 7

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					65		
		65.2-75.2	43%	9.9			65.2-75.2 GRAY HARD limestone w/ laminated layers of calcareous shale
					66		
					67		
					68		
							68.6-68.8 Gray calcareous shale
					69		
							69.4-69.9 Gray calcareous shale
					70		
							70.2-70.5 Gray calcareous shale
					71		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

SSS-1

Boring No. _____ Date _____ Sheet 7 of 7

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					72		
							<i>72.3 - 72.7 GRAY CALcareous SHALE</i>
					73		
					74		
					75		
		<i>75.2-77.2</i>	<i>83%</i>	<i>2.1</i>			<i>75.2 - 77.2 GRAY limestone w/ laminated layers of CALcareous SHALE</i>
					76		
					77		
							<i>stopped boring 77.2</i>
					78		

Dutch Cone Test

Company IK EC

Probe No. SS5-3 Date 8-7-87

Job CLIFTY CREEK EAD

Drillers MAC Knight - Lambert

Order Lambert

Page 1 of 4 Elevation 451.00 ^{+361'}

ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE READINGS		CONE RESIST. (1000N)	SLEEVE RESIST. (1000N)	FRICTION RATIO (%)
				HIGH	LOW			
GAGE ZERO READINGS				0.0	-0.2			
		.2	C					
			C+S	Road Bed				
		.4	C					
			C+S					
		.6	C		2.8			
			C+S		5.6			
		.8	C		2.4			
			C+S		4.2			
		1.0	C		2.4			
			C+S		4.4			
		.2	C		2.4			
			C+S		4.4			
		.4	C		2.6			
			C+S		4.8			
		.6	C		2.6			
			C+S		4.8			
		.8	C		2.6			
			C+S		5.0			
		2.0	C		2.4			
			C+S		4.8			
		.2	C		2.2			
			C+S		4.6			
		.4	C		2.0			
			C+S		4.0			
		.6	C		1.8			
			C+S		3.2			
		.8	C		1.6			
			C+S		3.0			
		3.0	C		1.6			
			C+S		3.0			
		.2	C		1.2			
			C+S		2.0			
		.4	C		1.2			
			C+S		2.0			
		.6	C		1.2			
			C+S		1.8			
		.8	C		0.4			
			C+S		1.2			
		4.0	C		0.8			
			C+S		2.2			
		.2	C		6.2			
			C+S		8.2			
		.4	C					
			C+S					
		.6	C					
			C+S					
		.8	C					
			C+S					
		0	C					
			C+S					

Augered to 15.0 started
Rock core

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. SS-3 Date 8-8-84 Sheet 2 of 4

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		15.0 20.4	6%	2.2			GRAY lime stone w/ calcareous shale 15.0 - 20.4
					16		
					17		Core lost ?
					18		
					19		
					20		
		20.4 25.0	8%	3.4			Core lost ?
					21		
					22		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level	
Time	
Date	

Boring No. SSJ-3 Date _____ Sheet 3 of 3

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

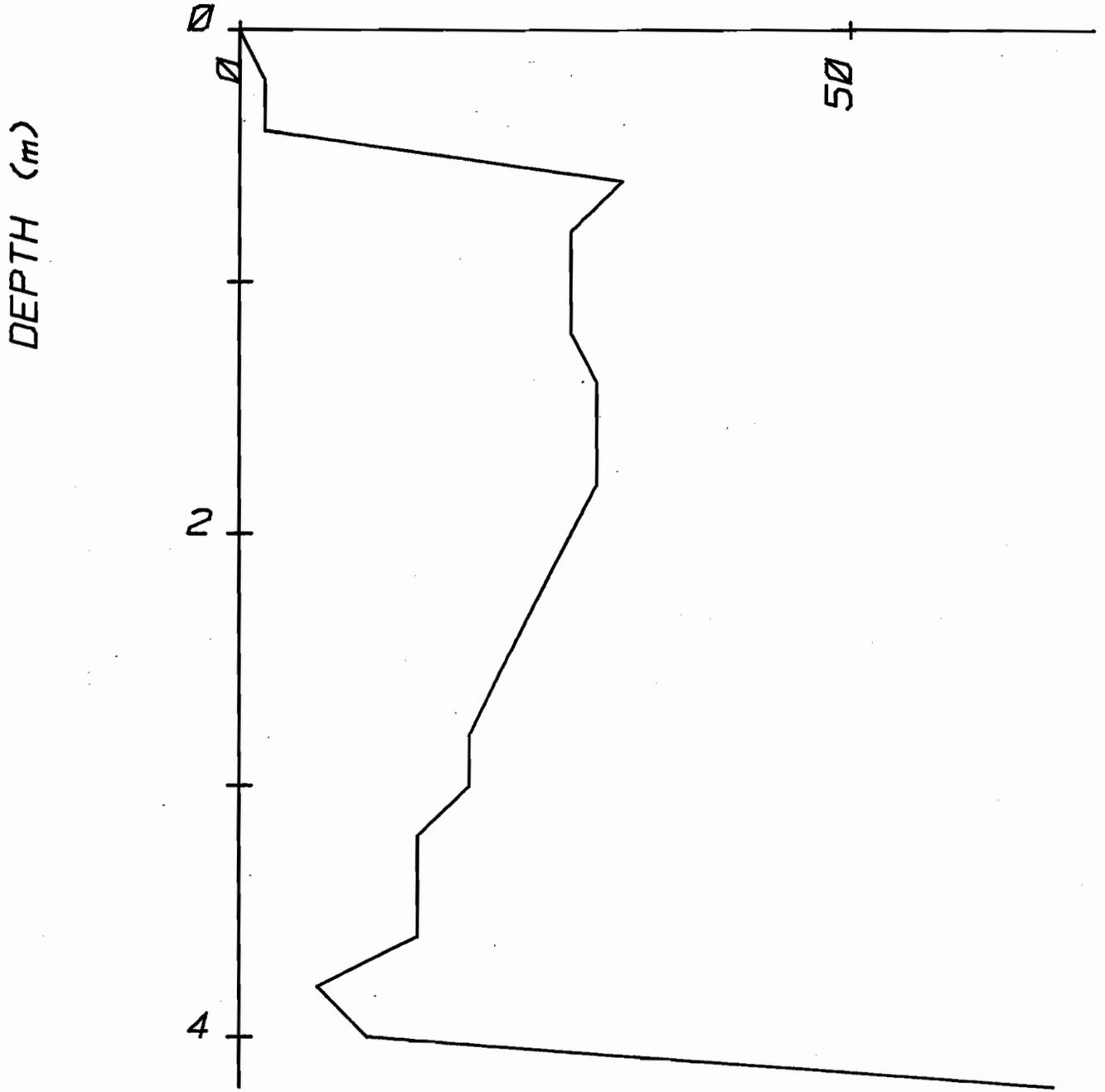
Ground Elevation _____ referred to _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
					23		
					24		
		25.0 35.0	53%	10.0	25		
					26		
					27		
					28		
					29		

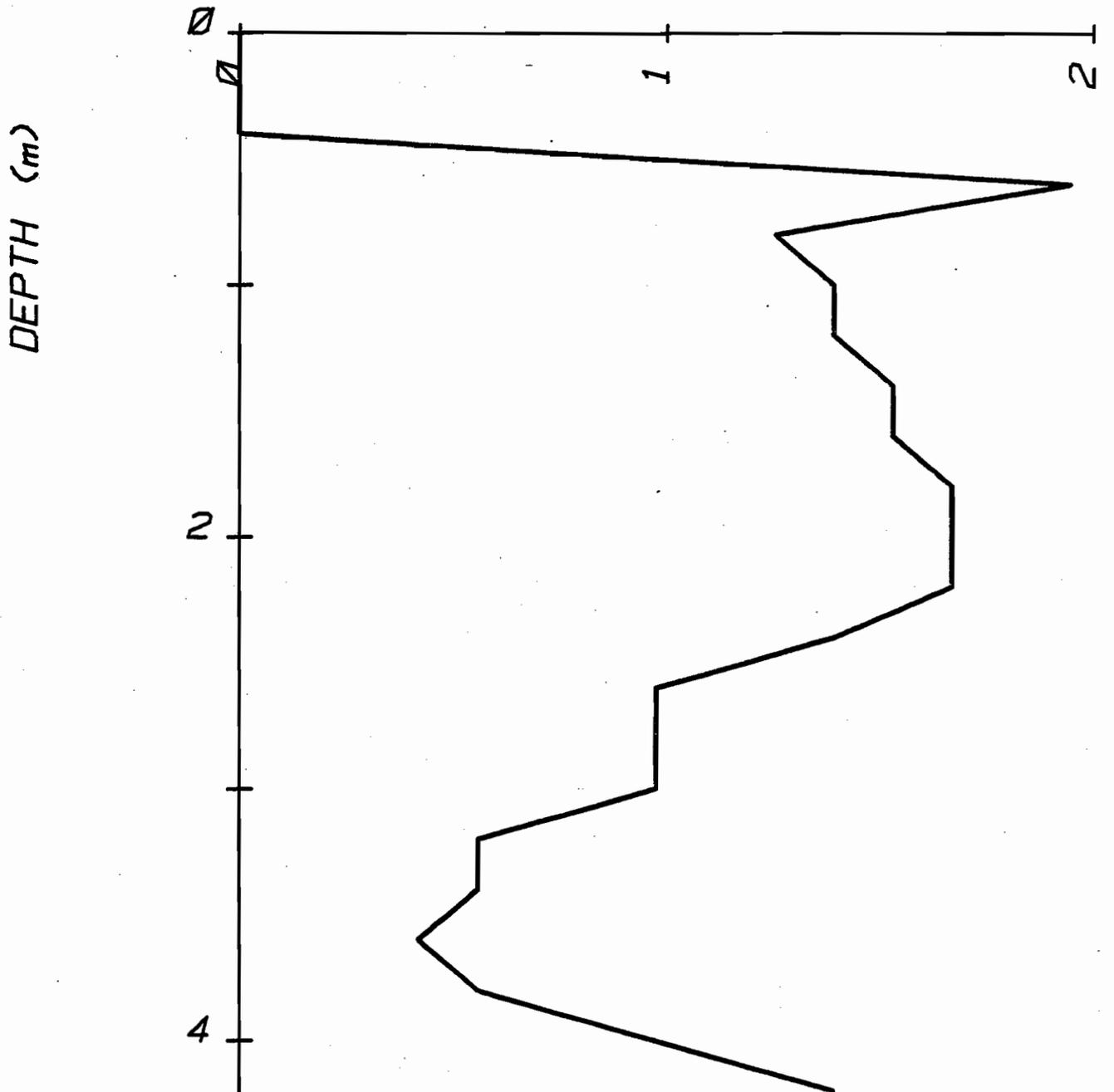
Rock Type, Color, Quality, Drilling observations, Depth of water lost, Observed fluctuations in water level, notes on drilling ease, etc.

CONE RESISTANCE (TSF)



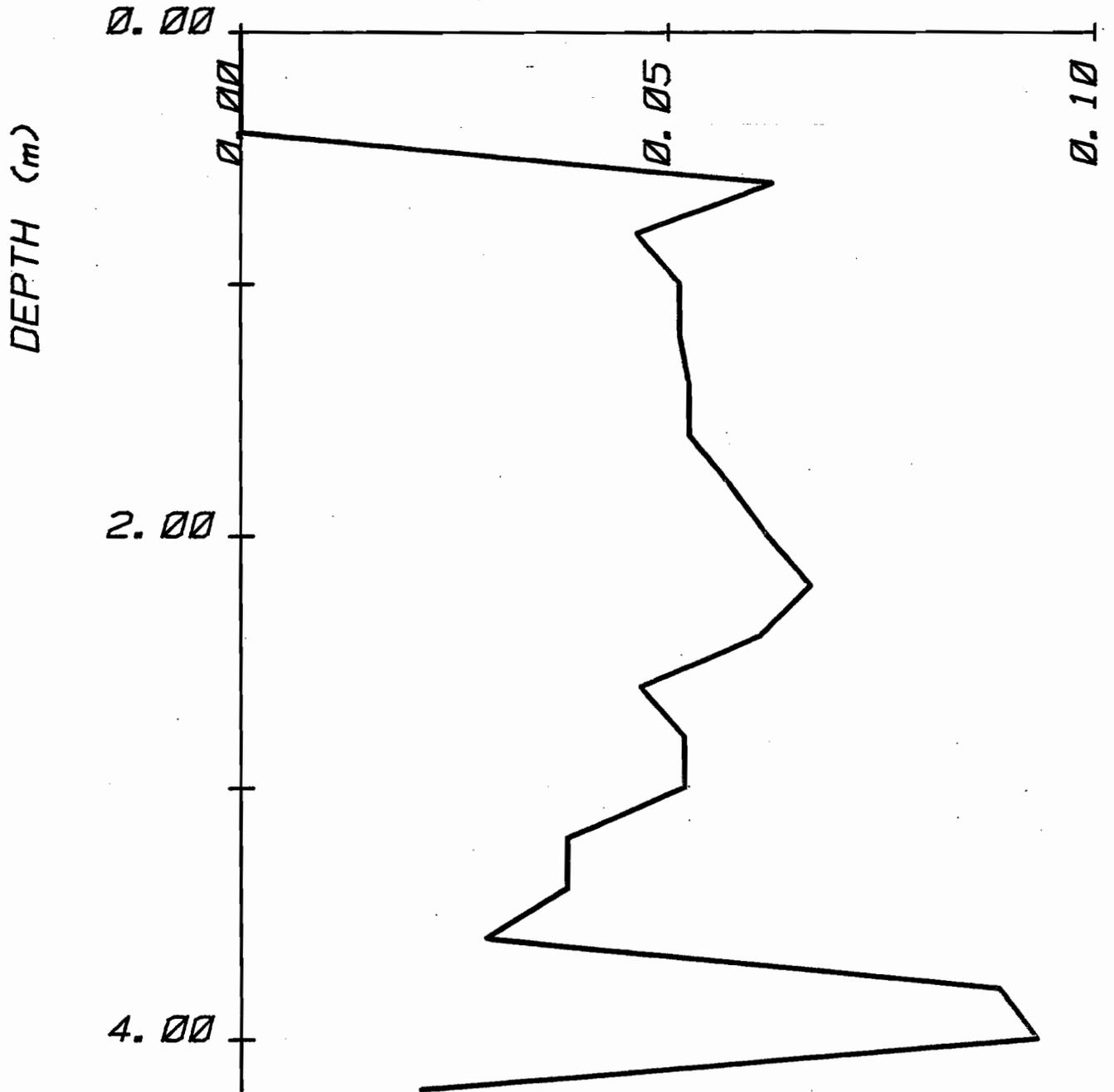
Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS5-3

SLEEVE RESISTANCE (TSF)



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS5-3

FRICTION RATIO



Clifty Creek Flyash Dam
Dam Raising Study
PROBE SS5-3

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

DB No. _____
COMPANY IKEC

PROJECT CLIFTY CREEK Fly Ash Dam

LOCATION OF BORING: _____

WATER LEVEL	<u>Dry to Top of Rock</u>
TIME	
DATE	

BORING No. SS6-1 DATE 7-11-84 SHEET 1 of 10

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 7-11-84 BORING COMPLETED 7-19-84

GROUND ELEVATION 566.0 REFERRED TO _____ DATUM _____

FIELD PARTY: Roush - Lambert

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO	BLOWS / 6"					
							0	
							1	<u>Moved boring 3' SOUTH WEST</u>
							2	
							3	
<u>1</u>	<u>3.0</u>	<u>3.7</u>	<u>13</u>	<u>50/2</u>	<u>3'</u>		4	
							5	
							6	
							7	
							8	
<u>2</u>	<u>8.0</u>	<u>8.0</u>	<u>59/0</u>		<u>0</u>		9	
							0	
							1	
							2	
							3	
<u>3</u>	<u>13.0</u>	<u>13.0</u>	<u>59/0</u>		<u>0</u>		4	<u>STARTED CORING AT 13.5</u>
							5	
							6	
							7	
							8	
							9	
							0	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Job No. _____

Company _____

Boring No. SS-1 / Date _____ Sheet 2 of 10

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		13.5 - 15.3		1.6	14		Core lost?
					14		Gray Hard Lime Stone
					15		Calcareous Shale Gray
		15.3 - 25.3	20%	6.7	15		Gray Hard Lime Stone
					16		Core lost?
					16		GRAY CALCAREOUS SHALE w/ FOUR LAYERS OF GRAY HARD LIME STONE
					17		
					18		
					19		
					20		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. SS6-1 Date _____ Sheet 3 of 10

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					21		
					22		
					23		
					24		
					25		
		25.3-35.3	17%	6.3			Gray Hard lime stone w/ laminated layers of calcareous shale
					26		
					27		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

Boring No. SS4-1 Date _____ Sheet 4 of 11

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					28		
					29		
					30		
					31		
					32		
					33		
					34		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Bor No. _____

Company _____

Boring No. SS6-1 Date _____ Sheet 5 of 10

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
					35		
		35.3 43.8	52%	7.7 6.9			
					36		
					37		
							Rust Br stain on joint
					38		
					39		
					40		
							Water level 40.0 7-16-84
					41		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

Boring No. SS6-7 Date _____ Sheet 6 of _____
Type of Boring _____ Rig _____
Casing used _____ Size _____ Drilling mud used _____
Boring begun _____ Boring completed _____
Ground Elevation _____ referred to _____ Datum _____
Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					42		
					x		
					x		
					43		
					x		
		43.8 48.2	9%	2.9			Gray calcareous shale
					44		
					45		
					46		
					47		
					48		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Boring No. SS6- / Date _____ Sheet 7 of 10

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum

Field Party: _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		48.2 51.8	35%	2.2			
					49		
					50		
					51		
		51.8 55.6	8%		52		WATER ELEV- 52.7 7-17-84 AFTER PACKER TEST WATER ELEV. 52.5 7/19/84
					53		GRAY HARD lime stone w/ calcareous shale layers
					54		
					54.5		BR RUST STAIN
					55		

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Boring No. SS6-1 Date _____ Sheet 8 of 11

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

Project _____
Location of Boring: _____
Water Level _____
Time _____
Date _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, note on drilling ease, etc.
		55.6 60.6					GRAY CALCAREOUS SHALE w/ LIME STONE LAYERS
					56		
					57		
					58		
					59		
					60		
		60.6 65.6	39%	5.0			GRAY HARD LIME STONE w/ CALCAREOUS SHALE LAYERS
					61		
					62		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. SS6-1 Date _____ Sheet 9 of 10

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					63		
					64		
					65		
		65.6 75.6	61%	9.9	66		GRAY HARD lime stone w/ CHICAGO shale layers
					67		
					68		
					69		

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

SPANY IKEC
PROJECT CHIFTY CREEK FLY ASH DAM

BORING No. SS6-2 DATE 7-24-84 SHEET 1 OF 4

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 7-24-84 BORING COMPLETED 7-25-84

GROUND ELEVATION 452.99 REFERRED TO _____ DATUM

FIELD PARTY: Roush - Lambert

LOCATION OF BORING:	
WATER LEVEL	<u>13.4</u>
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"							
									0	
									1	
									2	
									3	
<u>1</u>	<u>3.0</u>	<u>4.5</u>	<u>6</u>	<u>8</u>	<u>12</u>	<u>5"</u>			4	<u>Silty clay. DR. BR. moist, low to med plasticity, slight reaction to HCL</u> <u>CL</u>
									5	
									6	
									7	
									8	
<u>2</u>	<u>8.0</u>	<u>9.5</u>	<u>3</u>	<u>3</u>	<u>6</u>	<u>13"</u>			9	<u>Clayey sand - BR. poorly graded</u> <u>100% FINE GRAIN, moist</u> <u>SC</u>
									10	
									11	
									12	
									13	
<u>3</u>	<u>13.0</u>	<u>14.5</u>	<u>2</u>	<u>2</u>	<u>3</u>	<u>15"</u>			14	<u>Sandy clay BR. wet to saturated</u> <u>100% FINE GRAIN</u> <u>CL-SC</u>
									15	
									16	
									17	
									18	
									19	
	<u>18.0</u>	<u>19.5</u>	<u>1</u>	<u>2</u>	<u>2</u>	<u>15"</u>			20	<u>Clayey sand. BR. wet to saturated</u> <u>100% FINE GRAIN</u> <u>SC</u>

ENGINEER _____

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Boring No. SS6-2 Date _____ Sheet 2 of _____

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

Project _____
 Location of Boring: _____
 Water Level _____
 Time _____
 Date _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth: water lost, Observed fluctuations in water level, note on drilling ease, etc.
		20.0 24.5	12%	3.2			20.0 - 24.5 Gray Hard lime stone w/ laminated layers of calcareous shale - core lost?
					21		
					22		
					23		
		24.5 29.5	0	3.0	24		24.5 - 29.5 Gray Hard lime stone w/ laminated layers calcareous shale - core lost?
					25		
					26		
					27		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. 556-2 Date _____ Sheet 3 of 4

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Datum

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
	"				28		
					29		
		29.5 35.2		5.6			29.5 - 35.2 Gray HARD lime stone w/ laminated layers of calcareous shale
					30		
					31		
					32		
					33		
					34		

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

557-1

BORING NO. _____ DATE 6-19-84 SHEET 1 of 10

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 6-19-84 BORING COMPLETED 6-19-84

GROUND ELEVATION 504.17 REFERRED TO _____ DATUM

FIELD PARTY: Roush + Lambert

COMPANY IKCC
PROJECT CH. FT. CREEK Fly Ash Dam

LOCATION OF BORING:	
WATER LEVEL	<u>Dry To Top of Rock</u>
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"				TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO								
									0	Boring moved 15.0 FT S.E. OF STAKE
									1	
1	2.5	4.0	4	8	12	10"			2	Clay - Reddish Br, moist, med. plasticity
									3	CL
									4	
									5	
									6	
									7	
2	7.5	9.0	7	10	14	8"			8	Clay - yellowish Br. moist, med. plasticity. STRONG REACTION TO HCL
									9	CL
									10	
									1	
									2	
3	12.5	13.7	9	20	50/2	7"			3	GRAVELLY clay. yellowish Br. med. plasticity, STRONG REACTION TO HCL, GRAVEL CONSIST OF ANGULAR limestone FRAG, 3/4" MAX SIZE
									4	AUGER REFUSAL 14.4
									5	CL
									6	
									7	
									8	
									9	
									0	

ENGINEER _____

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level	
Time	
Date	

537-1

Boring No. _____ Date _____ Sheet 2 of 4

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		14.4-15.5	0	1.1	15		14.4-14.5 GRAY HARD LIME STONE
		15.5-25.5	0	9.2	16		15.5-15.6 GRAY HARD LIME STONE 15.6-16.2 SOFT GRAY + GRAY BR. SHALEY LIME STONE 16.2-16.4 GRAY HARD LIME STONE 16.4-17.0 GRAY BR. SOFT SHALEY LIME STONE
					17		
					18		BROKEN CORE LOSS THIS AREA? 21.2
					19		18.4-25.5 GRAY HARD LIMESTONE w/ LAMINATED LAYERS OF CALCAREOUS SHALE
					20		
					21		BROKEN

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring:	
Water Level	
Time	
Date	

SS7-1

Boring No. _____ Date _____ Sheet 3 of 10

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
							21.2-25.5 - GRAY Shaley limestone
							SOFT - Mod
							BROKEN
					22		
					23		
					24		
							BROKEN
					25		
	25.5-30.5		0	1			Piece of CORE jammed in END of CORE barrel
							limestone
					26		
					27		
					28		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level	
Time	
Date	

557-1

Boring No. _____ Date _____ Sheet 4 of _____
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____ Datum _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					28		
					29		
					30		
		30.5-33.5	6%	4.4			
					31		30.5-33.5 Gray Hard limestone w/ laminated layers of calcareous
					32		Sandy 31.5-32.0
					33		
					34		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

557-1

Boring No. _____ Date _____ Sheet S of 10

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum _____

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					35		
		35.5-45.5	64%	10.0	36		GRAY HARD LIME STONE - SILTY IN AREA
					37		
					38		
					39		
					40		
					41		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

557-1

Boring No. _____ Date _____ Sheet 6 of 1

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____ Datum

Field Party: _____

Project _____
Location of Boring: _____

Water Level	_____
Time	_____
Date	_____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION <small>Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.</small>
					42		41.6 - 42.3 VERTICAL CRACK. CORE NOT BROKEN
					43		
					44		
					45		
	45.5-55.5	19%	9.9	Broken			45.5 - 55.5 GRAY HARD LIMESTONE w/ LAMINATED LAYER OF CALCAREOUS SHALE HARD
					46		
					47		
					48		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level	_____
Time	_____
Date	_____

SS7-1

Boring No. _____ Date _____ Sheet 8 of 1
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____

Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		55.5-65.5	16%	9.9			
					56		GRAY Limestone w/ laminated layers of calcareous shale HARD
							calcareous shale
					57		calcareous shale
					58		
					59		calcareous shale
					60		
					61		
					62		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Boring No. SS7-1 Date _____ Sheet 9 of 10

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Date _____

Field Party: _____

Project _____
 Location of Boring: _____
 Water Level _____
 Time _____
 Date _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					63		
					64		
					65		
	655.75.5		48%	9.8			GRAY LIMESTONE w/ LAMINATED LAYERS OF CALCAREOUS SHALE
					66		
					67		
					68		
					69		

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level	_____
Time	_____
Date	_____

Boring No. SS7-1 Date _____ Sheet 10 of 1
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____ Datum _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					70		
					71		
					72		
					73		
					74		
					75		
							Stopped boring 75.5
							80 gal 564 LBs Cement Grouted Hole 75.5 To Surface 25 LBs Quick Jell

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

COMPANY IKFC
PROJECT CLIFTY CREEK NORTH F.A.D.

BORING NO. MS1-1 DATE 8-22-84 SHEET 1 OF 5

TYPE OF BORING SPT RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 8-22-84 BORING COMPLETED 8-23-84

GROUND ELEVATION 529.42 REFERRED TO _____ DATUM _____

FIELD PARTY: Roush + Lambert

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								0	
								1	
								2	
1	3.0	4.5	8	7	10	2"		3	
								4	Silty clay - Tan - moist - low to med plasticity - strong reaction to HCL CL
								5	
								6	
								7	
2	8.0	9.5	18	20	25	1"		8	lime stone frag.
								9	
								10	
								11	
								12	
								13	
3	13.0	14.5	25	22	17	6"		14	
								15	Silty clay - Tan - moist - low to med plasticity - strong reaction to HCL Trace of lime stone frag. CL
								16	
								17	
								18	
								19	
4	18.0	19.1	17	18	54	1"		20	lime stone frag.
								21	
								22	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

ANY _____

PROJECT _____

BORING No. MS-1 DATE _____ SHEET 2 OF 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

FIELD PARTY: _____ DATUM _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO	BLOWS / 6"						
								20	
								1	
								2	
								3	
5	23.0	24.5	16	30	36	6"			sandy clay - TAN - moist - low plasticity - w/ Br. sandy limestone in end of sample
								5	
								6	CL
								7	
								8	
								9	
								0	
								1	
								2	
								3	
								4	
								5	
								6	
								7	
								8	
								9	
								0	

ENGINEER _____

ACT CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. NS-1 Date _____ Sheet 3 of 5

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Date _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
					32		
		32.7 35.3	34%	2.4	33		Core lost? 32.7-45.3 Gray Hard lime stone w/ calcareous shale layers Be stain on joint
					34		Shale calcareous Gray
		35.3 39.0	22%	3.7	35		Gray calcareous shale
					36		Gray calcareous shale
					37		
					38		

ACET CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring:	
Water Level	
Time	
Date	

Boring No. NS17 Date _____ Sheet 4 of _____
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____ Datum _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		24.5					
		25.3	0	.6	25		Gray lime stone w/ .2 layer of lt. br. clay - HARD
		25.3					
		27.9	0	1.1			
					26		Gray lime stone w/ CALcareous shale layers - HARD - Core lost?
					27		
		27.9	9%	4.3			
		32.7					
					28		27.9 - 32.7 Gray limestone HARD w/ CALcareous shale layers core lost?
					29		
					30		
					31		

ACF CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. NS1-1 Date _____ Sheet 5 of 5

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Datum

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					39		
		39.0 45.3	53%	6.3			BRISTAIN ON JOINT
					40		
					41		Grey CALCAREOUS SHALE
					42		
					43		
					44		
					45		

Stopped boring 45.3

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

COMPANY TKEC
PROJECT CLIFTY CREEK NORTH FAD

BORING No. NS2-1 DATE 8-23-84 SHEET 1 of 2

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 8-23-84 BORING COMPLETED 8-23-84

GROUND ELEVATION 517.34 REFERRED TO _____

FIELD PARTY: Roush-Lambert

LOCATION OF BORING:	
WATER LEVEL	<u>dry</u>
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO					SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
						0	
						1	
						2	
						3	
<u>1</u>	<u>3.0</u>	<u>3.0</u>	<u>59</u>				<u>Auger cutting Gray clay shale</u>
						4	
						5	
						6	
						7	
						8	
						9	
						0	
						1	
						2	
						3	
						4	
						5	
						6	
						7	
						8	
						9	
						0	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

Boring No. NS2-1 Date _____ Sheet 2 of 2
Type of Boring _____ Rig _____
Casing used _____ Size _____ Drilling mud used _____
Boring begun _____ Boring completed _____
Ground Elevation _____ referred to _____
Date _____
Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
		7.2 12.6	27%	3.0			
	"				8		Gray limestone Hard w/ Gray calcareous shale core best?
					9		
					10		
					11		
					12		
					13		

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB NO. _____

COMPANY IKEC

PROJECT CLIFTY CREEK NORTH F.S.D

BORING NO. NS3-1 DATE 8-23-84 SHEET 1 OF _____

TYPE OF BORING SPT-CORE RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 8-23-84 BORING COMPLETED 8-23-84

GROUND ELEVATION 513.18 REFERRED TO _____

FIELD PARTY: Roush + Lambert

LOCATION OF BORING:	
WATER LEVEL	<u>Dry</u>
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								0	<u>Moved boring 25' W. OF STAKE</u>
								1	<u>WATER ELEV. 12.0 8-24-84</u>
								2	
								3	
<u>1</u>	<u>3.0</u>	<u>4.5</u>	<u>6</u>	<u>11</u>	<u>13</u>	<u>4"</u>		4	<u>SILTY clay - Tan - moist - low to med plasticity - TRACE OF limestone frag.</u>
								5	<u>cl</u>
								6	
								7	
								8	
<u>2</u>	<u>8.0</u>	<u>9.5</u>	<u>11</u>	<u>13</u>	<u>21</u>	<u>2"</u>		9	<u>limestone frag</u>
								10	
								11	
								12	
								13	
<u>3</u>	<u>13.0</u>	<u>14.3</u>	<u>11</u>	<u>20</u>	<u>50/3</u>	<u>0</u>		14	<u>STARTED CORE - 14.9</u>
								15	
								16	
								17	
								18	
								19	
								20	

ENGINEER

AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level _____
Time _____
Date _____

Boring No. NS3-1 Date _____ Sheet 2 of 2
Type of Boring _____ Rig _____
Casing used _____ Size _____ Drilling mud used _____
Boring begun _____ Boring completed _____
Ground Elevation _____ referred to _____ Date _____
Field Party: _____

ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	RQD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
					15		14.9-20.6 Gray Hard limestone w/ calcareous shale layers
		14.9 20.6	39%	5.6			
					16		
					17		
					18		Gray calcareous shale
					19		
					20		
					21		Stopped boring, 20.6

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

COMPANY IKEC

PROJECT CLIFTY CREEK F.A.D.

BORING No. ^{NS4-1} _____ DATE 9-11-84 SHEET 1 OF 4

TYPE OF BORING SPT RIG B-61

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN 9-11-84 BORING COMPLETED 9-11-84

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: Roush - Lambert

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	FROM	TO	BLOWS / 6"						
								0	
								1	
								2	
								3	
1	3.0	4.5	2	1	1	2"		3	Clay - Be moist - low to med. plasticity
								4	
								5	
								6	
								7	
								8	
2	8.0	8.9	6	50/4		5"		8	Gravelly clay - Blue-Gray - moist - poorly graded -
								9	GRAVEL - CONSIST OF LIME STONE FRAG
								10	
								11	
								12	
								13	
								14	
								15	
								16	
								17	
								18	
								19	
								20	

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____

Location of Boring: _____

Water Level _____

Time _____

Date _____

Boring No. ^{NS 4-1} _____ Date _____ Sheet 2 of _____

Type of Boring _____ Rig _____

Casing used _____ Size _____ Drilling mud used _____

Boring begun _____ Boring completed _____

Ground Elevation _____ referred to _____

_____ Date _____

Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION
							10.9-30.0 Gray HARD lime stone w/ CALCARCEOUS shale layers
		10.9 20.0	44%	8.9	11		
					12		shale calcareous GRAY
					13		" "
					14		shale calcareous GRAY
					15		Gray calcareous shale
							Gray calcareous shale
					16		GRAY CALCARCEOUS shale
					17		Gray CALCARCEOUS shale

**AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

Job No. _____

Company _____

Project _____
Location of Boring: _____

Water Level	
Time	
Date	

Boring No. ^{NS4-1} _____ Date _____ Sheet 2 of 4
 Type of Boring _____ Rig _____
 Casing used _____ Size _____ Drilling mud used _____
 Boring begun _____ Boring completed _____
 Ground Elevation _____ referred to _____
 Date _____
 Field Party: _____

ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.
							10.9-30.0 Gray HARD limestone w/ CALCARCEOUS shale layers
		10.9 20.0	44%	8.9	11		
					12		shale CALCARCEOUS GRAY
					13		" "
					14		shale CALCARCEOUS GRAY
					15		GRAY CALCARCEOUS shale
							GRAY CALCARCEOUS shale
					16		GRAY CALCARCEOUS shale
					17		GRAY CALCARCEOUS shale

Indiana - Kentucky Electric Corporation
Clifty Creek Plant
Madison, Indiana

Flyash Dam Raising Feasibility Report

APPENDIX H
Boring Location Plan and Profile
Drawings CE-CCBP - 1 to 7 - 84

January 31, 1985

Prepared by

Soils, Foundation and Hydro Section
Civil Engineering Division
American Electric Power Service Corporation

BORING LOG TERMINOLOGY

Stratum Depth:

Distance in feet and/or inches below ground surface.

Stratum Elevation:

Elevation in feet below ground surface elevation.

Description of Materials:

Major types of soil material existing at boring location. Soil classification based on one of the following systems: Unified Soil Classification System, Ohio State Highway Classification System, Highway Research Board Classification System, Federal Aviation Authority Classification System, Visual Classification.

Sample No.:

Sample numbers are designated consecutively, increasing with depth for each boring.

Sample Type:

"A" Split spoon, 2" O.D., 1-3/8" I.D., 18" in length.

"B" One of the following:

Power Auger Sample

Piston Sample

Diamond Bit NX: BX: AX:

Housel Sample

Wash Sample

Denison Sample

"C" Shelby Tube 3" O.D. except where noted.

Sample Depth:

Depth below top of ground at which appropriate sample was taken.

Blows per 6" on Sampler:

The number of blows required to drive a 2" O.D., 1-3/8" I.D., split spoon sampler, using a 140 pound hammer with a 30 inch free fall, is recorded for 6" drive increments. (Example: 3/8/9)

"N" Blows/Ft.:

Standard penetration resistance. This value is based on the total number of blows required for the last 12" of penetration. (Example: 3/8/9 ∴ N = 8 + 9 = 17)

Water Observations:

Depth of water recorded in test boring is measured from top of ground to top of water level. Initial depth indicates water level during boring, completion depth indicates water level immediately after boring, and depth after "X" number hours indicates water level after letting water rise or fall over a time period. Water observations in pervious soil are considered reliable ground water levels for that date. Water observations in impervious soils can not be considered accurate ground water measurements for that date unless records are made over several days' time. Factors such as weather, soil porosity, etc., will cause the ground water level to fluctuate for both pervious and impervious soils.

SOIL DESCRIPTION

Color:

When the color of the soil is uniform throughout, the color recorded will be such as brown, grey, black and may be modified by adjectives such as light and dark. If the soil's predominant color is shaded by a secondary color, the secondary color precedes the primary color, such as: grey-brown, yellow-brown. If two major and distinct colors are swirled throughout the soil, the colors will be modified by the term mottled, such as: mottled brown and grey.

Particle Size	Visual	Soil Components	
		Major Component:	Minor Component Term
Boulders	Larger than 8"	Gravel	Trace 1-10%
Cobbles	8" to 3"	Sand	Some 11-35%
Gravel—Coarse	3" to 3/4"	Silt	And 36-50%
Fine	2 mm. to 3/4"	Clay	
Sand —Coarse	2 mm.-0.6 mm. (Pencil lead size)		
—Medium	0.6 mm.-0.2 mm. (Table sugar and salt size)		
—Fine	0.2 mm.-0.06 mm. (Powdered sugar and human hair size)		
Silt	0.06 mm.-0.002 mm.		
Clay	0.002 and smaller (Particle size of both Silt and Clay not visible to naked eye)		

		Moisture Content	
		Term	Relative Moisture
		Dry	Powdery
		Damp	Moisture content below plastic limit
		Moist	Moisture content above plastic limit but below liquid limit
		Wet	Moisture content above liquid limit

**Condition of Soil Relative to Compactness
Granular Material**

Very Loose	5 blows/ft. or less
Loose	6 to 10 blows/ft.
Medium Dense	11 to 30 blows/ft.
Dense	30 to 50 blows/ft.
Very Dense	51 blows/ft. or more

**Condition of Soil Relative to Consistency
Cohesive Material**

Very Soft	3 blows/ft. or less
Soft	4 to 5 blows/ft.
Medium Stiff	6 to 10 blows/ft.
Stiff	11 to 15 blows/ft.
Very Stiff	16 to 30 blows/ft.
Hard	31 blows/ft. or more

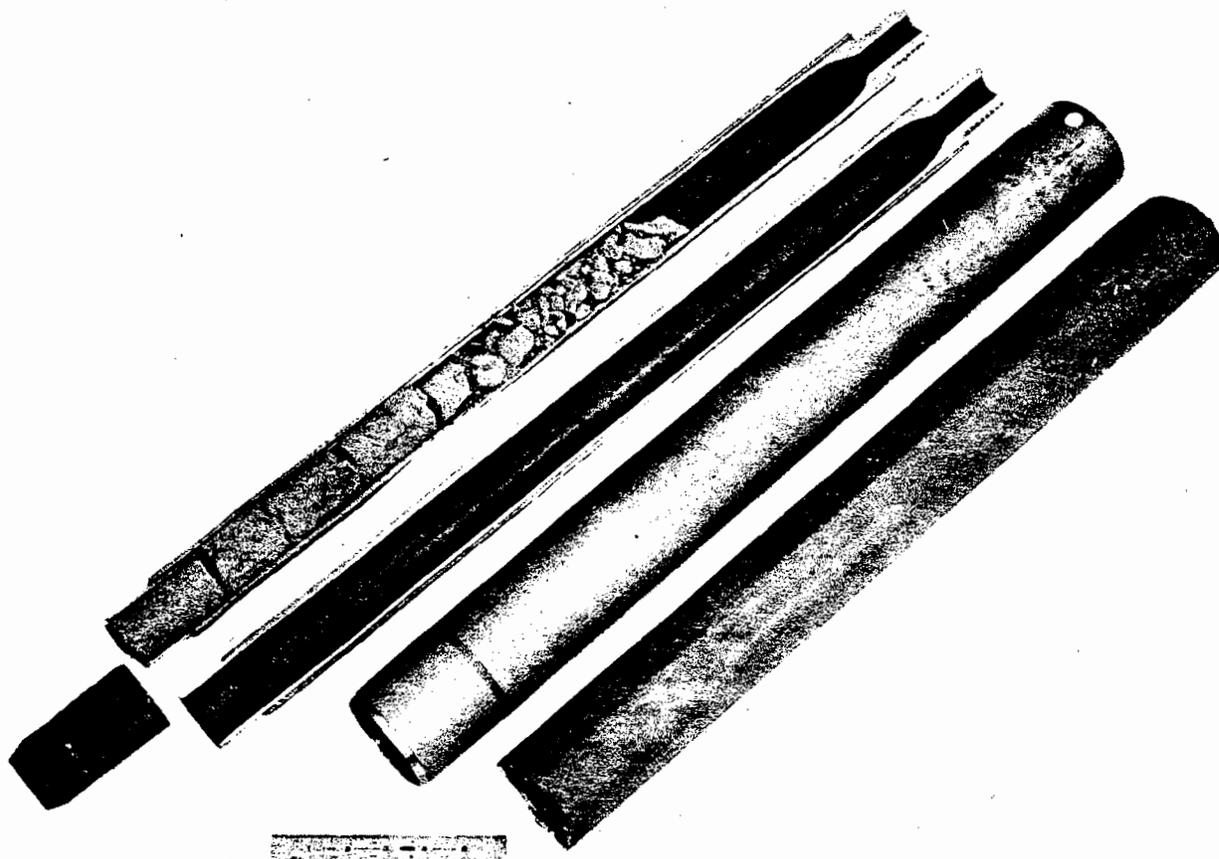
STANDARD PENETRATION RESISTANCE (ASTM D1586)

The purpose of this test is to determine the relative consistency of the soils in a boring, or from boring to boring over the site. This method consists of making a hole in the ground and driving a 2 inch O.D. split spoon sampler into the soil with a 140 pound hammer dropped from a height of 30 inches. The sampler is driven 18 inches and the number of blows recorded for each 6 inches of penetration. Values of standard penetration (N) are determined in blows per foot, summarizing the blows required for the last two 6 inch increments of penetration.

Example: 2-6-8; N = 14

THIN-WALLED SAMPLER (ASTM D1587)

The purpose of the thin-walled sampler is to recover a relatively undisturbed soil sample for laboratory tests. The sampler is a thin-walled seamless tube with a 3 inch outside diameter, which is hydraulically pressed into the ground, at a constant rate. The ends are then sealed to prevent soil moisture loss, and the tube is returned to the laboratory for tests.



BOWSER-MORNER, INC.

420 DAVIS AVENUE, P.O. BOX 51, DAYTON, OHIO 45401

ENGINEERING REPORT

REPORT TO: American Electric Power Service Corp. REPORT DATE: October 8, 1984
1 Riverside Plaza
P.O. Box 16631 REPORT NO.: 28089-1084-436
Columbus, Ohio 43216

Attention: Mr. Kevin C. Miller,
Civil Engineering Division

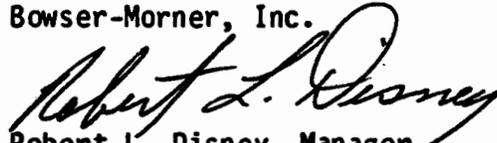
REPORT ON: Cone Penetration Test Results, Indiana-Kentucky Electric
Corporation, Clifty Creek Power Station Flyash Impoundment,
Madison, Indiana

Bowser-Morner, Inc., submitted copies of field boring logs in Bowser-Morner Report No. 28089-1084-425, dated October 2, 1984. Results of the cone penetration tests performed by Woodward-Clyde Consultants were not available at that time.

The results of the cone penetration tests are included with this report. If there are any questions regarding the test results, please contact the writer, or if you prefer, contact Mr. Frederick W. Johnson, Senior Project Engineer, Woodward-Clyde Consultants.

Respectfully submitted,

Bowser-Morner, Inc.


Robert L. Disney, Manager
Soils Exploration Department

RLD/mjj(#95)
2-Client
2-File

11 East Adams Street
Suite 1500
Chicago, Illinois 60603
312-939-1000
Telex 253875 (WOODWARD CGO)

Woodward-Clyde Consultants

October 3, 1984
84C3092

Bowser-Morner
420 Davis Avenue
P.O. Box 51
Dayton, Ohio 45401

Attention: Mr. Robert Disney

Re: Cone Penetration Test Results
AEP Indiana-Kentucky Electric Corporation
Clifty Creek Plant Flyash Impoundment
Near Madison, Indiana

Gentlemen:

This letter transmits the results of four cone penetration test (CPT) borings made at the referenced project on September 19, 20, 21 and 22, 1984. These CPT borings were made in accordance to our August 31, 1984 proposal. The CPT data are summarized in Tables 1 thru 4 and are shown in Figures 3 thru 6. These figures show the cone resistance in lb/in^2 vs depth for each of the CPT borings. A brief discussion of the WCC cone penetrometer system has also been included at the request of Mr. Kevin Miller of AEP.

It was a pleasure to do business with you. If additional information is required please contact us.

Very truly yours,



Frederick W. Johnson
Senior Project Engineer



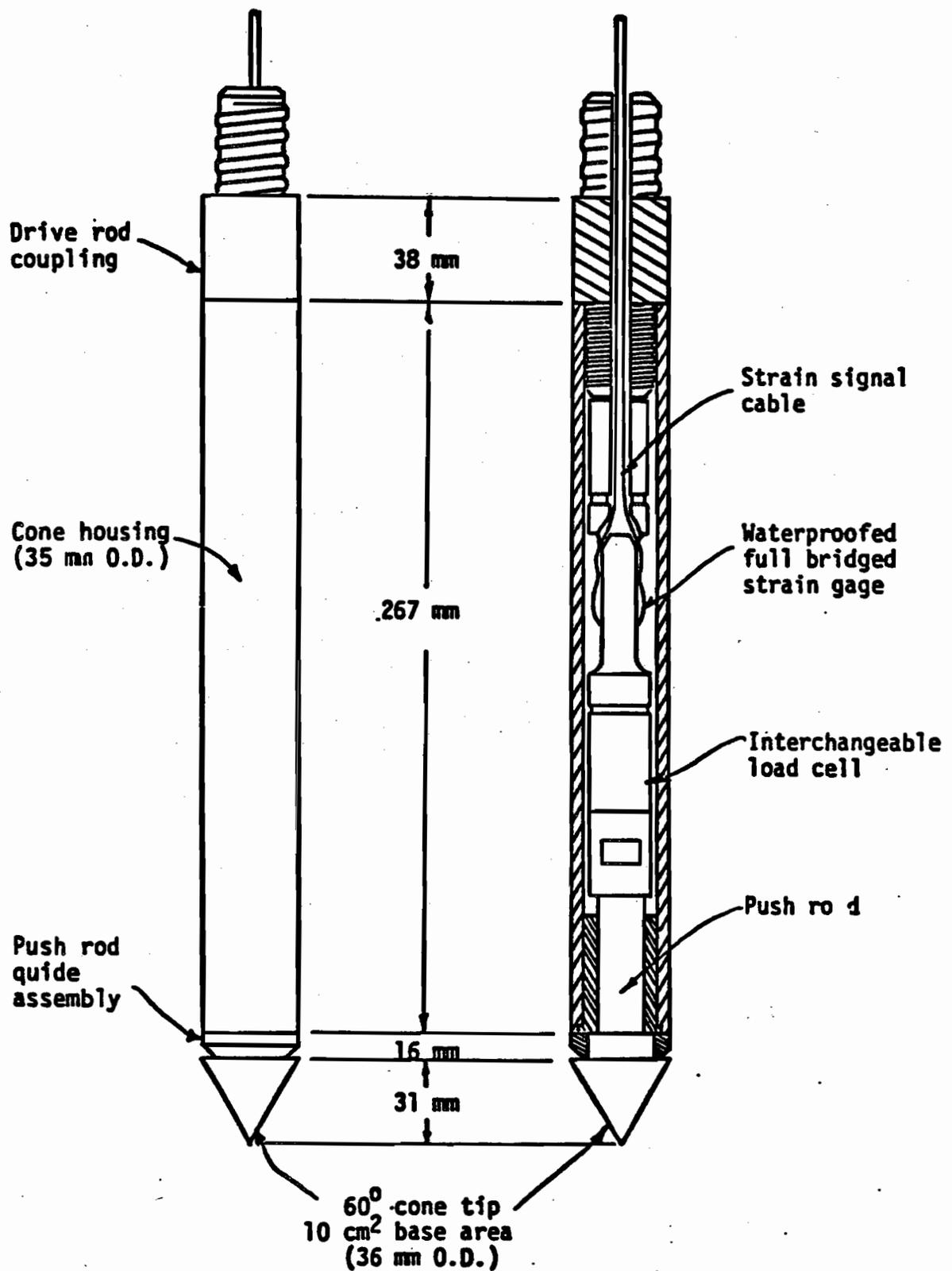
THE WOODWARD-CLYDE CONSULTANTS CONE-PENETROMETER SYSTEM

The Woodward-Clyde Consultants (WCC) cone penetrometer system is patterned after the 60 degree, 10 cm² electronic Dutch cone employed in continuous (static) penetration testing and is shown in Figure 1. During cone-penetration resistance testing (CPT), the drill-rig advances the cone at a rate of about 5+1 ft/minute and continuous cone penetration is usually obtained at intervals of at least 5 feet, Figure 2.

The point resistance (q_c) is sensed by the load-cell just above the cone and depth events are sensed by a 360° potentiometer attached to the drive system. The analog (voltage) signals from the load-cell are sent via a signal cable to a strip chart recorder. There the signals are conditioned and displayed in real time. Three signal amplification ranges are available to provide an appropriate scale for the q_c reaction range anticipated.

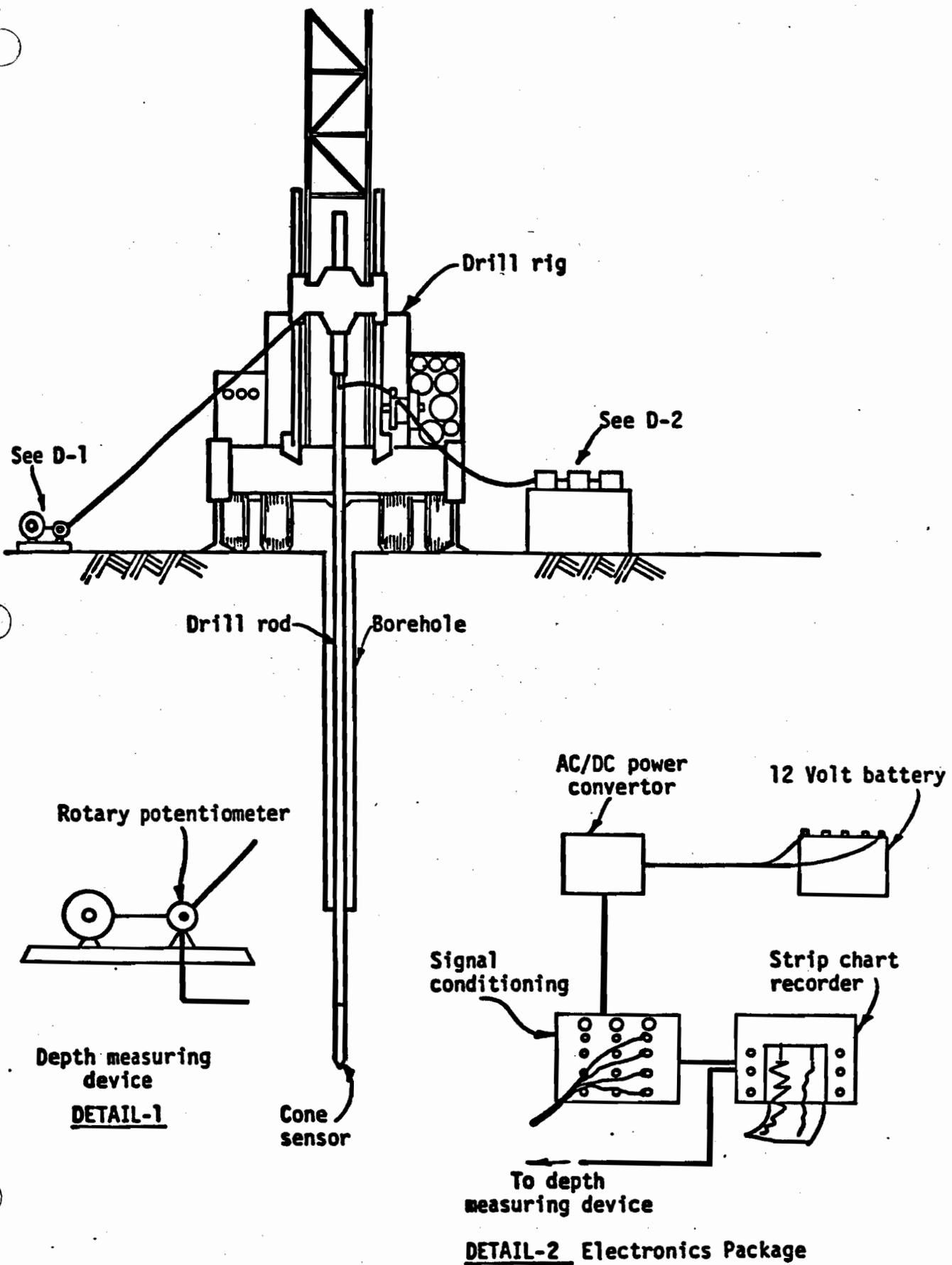
As the strip chart record represents a voltage analog of q_c , the record must be converted to load. A conversion factor is given for each of three signal amplification ranges and a load factor, given for the load-cell, is applied to convert the analog record of each CPT run.

CPT data can be simply reduced by scaling the field record with a proportional divider, by conversion with a hand-held calculator, or alternatively, with a small computer program using either a commercial time-share facility or an in-house microcomputer. The program input for each continuous advance is the analog q_c , the test depth, the signal amplification range selected and the corresponding load factor.



WCC ELECTRICAL CONE PENETROMETER

Fig. 1



CONE PENETRATION TEST - APPARATUS SCHEMATIC

LOG OF BORING No. **504-1**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/22/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/22/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE					
2.0'	TOP OF WATER					
9.5'	FLYASH					
80.0'	BOTTOM OF BORING					
NOTE! CONE PENETRATION TEST BORING						

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B. C. - K. G.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

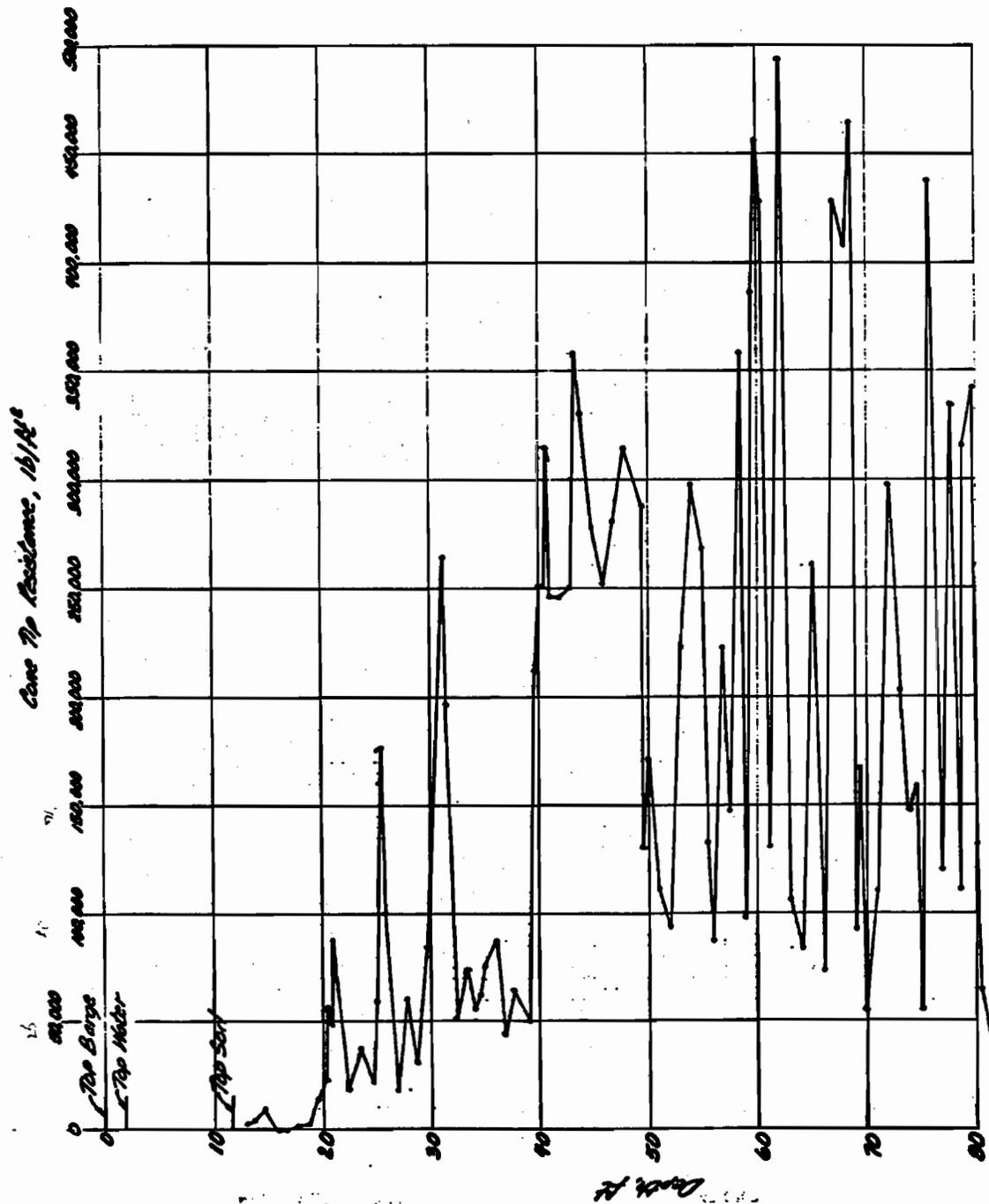
TYPE SAMPLER:
 A. SPLIT SPOON
 B.
 C. SHELBY TUBE

Woodward-Clyde Consultants CHICAGO	SUBJECT <u>Table 1 Cone Resistance</u>	PROJECT <u>Bousser-Manner</u>
	<u>vs Depth</u>	FILE NO. <u>84C3092</u>
	COMPUTED <u>JVH</u>	CHECKED <u>FwJ</u>
		DATE <u>9-30-84</u> PAGE <u>1</u> OF <u>2</u> PAGES

<u>depth, ft</u>	<u>Resistance lb/ft²</u>	<u>depth, ft</u>	<u>Resistance lb/ft²</u>	<u>Boring SO-4-1</u>
12.7	2788	38.3	18588	
13.7	3717	39.1	50187	
14.8	9294	39.6	213762	
15.8	0	40.1	250938	
16.8	0	40.5	250938	
17.8	2323	40.75	315996	
18.8	3717	41.0	246291	
19.8	14870	42.0	246291	
20.3	23235	43.0	250938	
20.6	57622	43.4	358748	
20.8	48328	44.0	330864	
21.0	87363	45.0	277703	
22.3	18588	46.0	204468	
23.3	37176	47.0	232350	
24.3	21376	47.8	315996	
24.8	69705	49.4	283114	
25.5	176596	49.6	130116	
25.8	106831	49.9	171939	
26.8	18588	51.0	111528	
27.8	60411	52.0	94798	
28.8	30205	53.0	223056	
29.8	84575	54.0	297408	
31.1	264879	55.0	269526	
31.4	195174	55.5	130116	
32.3	51117	56.0	83646	
33.3	74352	56.9	223056	
34.1	55764	57.3	148704	
34.6	63199	58.2	358748	
35.0	66916	58.8	97587	
35.3	55764	59.6	386630	
36.2	78069	60.0	456335	
36.9	44611	60.3	428453	
37.6	65058			

Table 1

<u>depth, ft</u>	<u>Resistance</u> <u>lb/ft²</u>	<u>Boring</u> <u>SO-4-1</u>
61.0	130116	
62.2	493511	
63.0	111528	
64.0	83646	
65.0	260232	
66.0	74352	
67.0	428453	
68.0	409865	
68.5	465529	
69.0	92940	
69.3	167292	
70.0	55764	
71.0	111528	
72.0	297408	
73.0	204468	
74.0	148704	
74.5	157998	
75.0	55764	
75.9	437747	
77.0	126822	
77.8	335513	
78.5	111528	
78.8	316925	
79.7	391277	
80.2	130116	
80.5	65050	
81.4	37176	
81.9	116175	
82.3	51117	



CONE PENETRATION TEST
 CLIFTY CREEK POWER PLANT
 BOWSER-MADONER

Fig. 3

WCC 875302

BORING SD-4-1

DRIILLED 9-28-1964

LOG OF BORING No. **SO 4.5-1**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/6/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/6/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE					
2.0'	WATER					
16.0'	FLYASH	1A	18.5-20.0	0-0-0	0	0"
		2A	23.5-25.0	0-0-0	0	0"
		3A	28.5-30.0	1-1-0	1	14"
		1C	33.5-35.5	-	-	0"
		4A	38.5-40.0	1-2-3	5	18"
42.0'	HARD GRAY SILT, TRACE OF SAND - MOIST	5A	43.5-45.0	15-26-31	57	16"
48.5	VERY STIFF BROWN SILT, SOME SAND - WET	6A	48.5-50.0	3-6-12	18	16"
		7A	53.5-55.0	4-8-11	19	16"

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B.C.-B.C.M.-K.G.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B.
 C. SHELBY TUBE

BOWSER-MORNER

LOG OF BORING No. **SO 4.5-1****CLIFTY CREEK FLYASH IMPOUNDMENT**BORING LOCATION: **AS DIRECTED**DATE STARTED: **9/6/84**SURFACE ELEVATION: **502.2' (WATER)**DATE COMPLETED: **9/6/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
59.0'	MEDIUM STIFF GRAY SILT, TRACE OF SAND - MOIST	8A	58.5-60.0	4-4-5	9	16"
	(BECOMES HARD AT 63.5')	9A	63.5-65.0	14-19-24	43	16"
68.5'	HARD GRAY SILT, SOME SAND - WET	10A	68.5-70.0	12-20-29	49	17"
		11A	73.5-75.0	11-22-30	52	18"
79.0'	HARD GRAY SILT, SOME CLAY, TRACE OF SAND - MOIST	12A	78.5-80.0	13-21-32	53	16"
		13A	83.5-85.0	11-20-29	49	18"
		14A	88.5-90.0	14-21-30	51	16"
		15A	90.5-92.0	13-19-26	45	16"
92.0'	BOTTOM OF BORING					

DRILLING METHOD: **HOLLOW STEM
AUGER**DRILLER: **B.C. - B.C.M. - K.G.**JOB NO.: **28089**

WATER OBSERVATIONS

INITIAL DEPTH: **2.0'**COMPLETION DEPTH: **2.0'**

DEPTH AFTER HRS.

TYPE SAMPLER:

 A. SPLIT SPOON B. C. SHELBY TUBE

BOWSER-MORNER

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB NO. _____

COMPANY _____

PROJECT _____

BORING NO. SD 4.5-1 DATE _____ SHEET 2 OF 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								20	
								1	
								2	
								3	
	33.5	35.5						4	Shelby Tube
								5	
								6	
								7	
								8	
4	38.5	40.0	1	2	3	18"		9	FLY ASH
								30	
								1	
								2	
								3	
5	43.5	45.0	15	26	31	16"		4	SILT - GRAY BR. - MOIST TO WET - TRACE OF 1/2 FINE SAND
								5	ML
								6	
								7	
								8	
	49.5	50.0	3	6	12	16"		9	CLAYEY SAND - BR. - SATURATED - POORLY GRAINED - 100% FINE GRAIN
								0	SC

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

504.5-1

BORING No. _____ DATE _____ SHEET 3 of 5

COMPANY _____

TYPE OF BORING _____ RIG _____

PROJECT _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								50	
								1	
								2	
								3	
7	53.5	55.0	4	8	11	16"		4	clayey silt - yellowish BR - moist to wet - slight plasticity
								5	ML
								6	
								7	
								8	
8	58.5	60.0	4	9	5	16"		9	clayey silt - Gray BR - wet to saturated slight plasticity - trace of sand
								60	CL-ML
								1	
								2	
								3	
9	63.5	65.0	14	19	24	16"		4	sand-silt - Gray BR - moist w/ fine grain sand lens - slight reaction to HCL
								5	ML
								6	
								7	
								8	
	68.5	70.0	12	20	29	17		9	same as sample no. 9
								70	

ENGINEER _____

**AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING**

JOB No. _____

Boring No. 504.5-1 DATE _____ SHEET 4 OF 5

COMPANY _____

TYPE OF BORING _____ RIG _____

PROJECT _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

LOCATION OF BORING:	
WATER LEVEL	_____
TIME	_____
DATE	_____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
								70	
								1	
								2	
								3	
11	73.5	75.0	11	22	30	18"		4	clayey silt - Gray BR. Low plasticity Slight Reaction to HCL - moist
								5	ML-CL
								6	
								7	
								8	
12	78.5	80.0	13	21	32	16"		9	Same as Sample No. 11
								80	
								1	
								2	
								3	
13	83.5	85.0	11	20	29	18"		4	Same as Sample No. 11
								5	
								6	
								7	
								8	
14	88.5	89.0	14	21	30	16"		9	Same as Sample No. 11
								90	

ENGINEER _____

LOG OF BORING No. **SO 4.5-2**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/5/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/5/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE					
2.0'	WATER					
7.5'	FLYASH	1A	7.5-9.0	1-1-0	1	12"
		2A	9.0-10.5	0-0-0	0	0"
		3A	10.5-12.0	0-0-0	0	10"
		4A	12.0-13.5	2-1-2	3	10"
		5A	13.5-15.0	0-0-2	2	10"
		6A	15.0-16.5	0-0-0	0	12"
		7A	16.5-18.0	2-0-1	1	10"
		8A	18.0-19.5	0-2-1	3	8"

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B.C.-B.C.M.-K.G.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B.
 C. SHELBY TUBE

LOG OF BORING No. **SO 4.5-2**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/5/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/5/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
		9A	19.5-21.0	1-1-1	2	11"
		10A	21.0-22.5	3-2-1	3	13"
23.0'	LOOSE BROWN SAND, SOME SILT, TRACE OF CLAY-MOIST	11A	22.5-24.0	1-2-7	9	9"
		1C	24.0-26.0	-	-	20"
26.0'	VERY STIFF BROWN SILT, SOME CLAY, TRACE OF SAND-MOIST	12A	26.0-27.5	5-7-13	20	14"
27.5'	VERY STIFF BROWN & GRAY SILT SOME CLAY, TRACE OF SAND-MOIST	13A	27.5-29.0	9-10-17	27	16"
		14A	29.0-30.5	6-11-15	26	12"
31.0'	VERY STIFF BROWN SILT, SOME SAND, TRACE OF CLAY-MOIST	15A	30.5-32.0	9-14-16	30	14"
		16A	32.0-33.5	8-13-15	28	14"
33.5'	MEDIUM DENSE BROWN SAND, SOME SILT, TRACE OF CLAY-MOIST	17A	33.5-35.0	7-12-16	28	17"

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B.C. - B.C.M. - K.G.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B.
 C. SHELBY TUBE

LOG OF BORING No. **SO 4.5-2**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/5/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/5/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
35.0'	MEDIUM DENSE BROWN & GRAY SAND, TRACE SILT, TRACE OF CLAY-MOIST	18A	35.0-36.5	8-11-13	24	16"
36.5'	VERY STIFF BROWN SILT, SOME SAND, TRACE OF CLAY-MOIST	19A	36.5-38.0	9-13-14	27	18"
38.0'	VERY STIFF GRAY SILT, SOME SAND, TRACE OF CLAY-MOIST	20A	38.0-39.5	10-11-17	28	16"
39.5'	STIFF GRAY SILT, SOME CLAY, SOME ORGANIC, TRACE OF SAND-MOIST	21A	39.5-41.0	4-6-7	13	18"
41.0'	VERY STIFF BROWN & GRAY CLAY, SOME SILT, TRACE OF SAND-DAMP	22A	41.0-42.5	5-10-13	23	18"
	(BECOME STIFF AT 42.5')	23A	42.5-44.0	4-6-8	14	18"
		24A	44.0-45.5	4-7-8	15	16"
	(BECOMES VERY STIFF AT 45.5')	25A	45.5-47.0	5-8-10	18	17"
47.0'	STIFF BROWN & GRAY SILT, SOME SAND, TRACE OF CLAY-WET	26A	47.0-48.5	3-4-8	12	16"

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B. C. - B. C. M. - K. G.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B.
 C. SHELBY TUBE

LOG OF BORING No. **SO 4.5-2**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/5/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/5/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
	(BECOMES VERY STIFF AT 48.5')	27A	48.5-50.0	12-13-17	30	15"
	(BECOMES HARD AT 50.0')	28A	50.0-51.5	15-16-16	32	18"
	(BECOMES STIFF AT 51.5')	29A	51.5-53.0	3-5-8	13	16"
	(BECOMES VERY STIFF AT 53.0')	30A	53.0-54.5	8-9-12	21	16"
	(BECOMES HARD AT 54.5')	31A	54.5-56.0	19-19-35	54	18"
56.0'	VERY STIFF GRAY SILT, TRACE OF CLAY - DAMP	32A	56.0-57.5	9-10-19	29	18"
	(BECOMES HARD AT 57.5')	33A	57.5-59.0	19-27-29	56	18"
		34A	59.0-60.5	27-21-20	41	12"
		35A	60.5-62.0	11-13-19	32	16"

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B.C. - B.C.M. - K.G.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B.
 C. SHELBY TUBE

BOWSER-MORNER

LOG OF BORING No. **SO 4.5-2**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/5/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/5/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
		36A	62.0-63.5	11-19-40	59	16"
		37A	63.5-65.0	17-21-40	61	12"
65.0'	VERY DENSE GRAY SAND AND SILT, TRACE OF CLAY-MOIST	38A	65.0-66.5	16-23-30	53	16"
		39A	66.5-68.0	22-25-27	52	16"
		40A	68.0-69.5	19-21-23	44	14"
		41A	69.5-71.0	17-19-22	41	16"
		42A	71.5-73.0	15-20-19	39	14"
		43A	73.0-74.5	19-24-26	50	15"
		44A	74.5-76.0	18-23-24	47	16"

DRILLING METHOD: **HOLLOW STEM AUGER**

DRILLER: **B.C. - B.C.M. - K.G.**

JOB NO.: **28089**

WATER OBSERVATIONS

INITIAL DEPTH: **2.0'**

COMPLETION DEPTH: **2.0'**

DEPTH AFTER HRS.

TYPE SAMPLER:

A. SPLIT SPOON

B.

C. SHELBY TUBE

BOWSER-MORNER

LOG OF BORING No. **SO 4.5-2**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/5/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/5/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
	(BECOMES MEDIUM DENSE AT 76.0')	45A	76.0-77.5	8-12-14	26	18"
	(BECOMES DENSE AT 77.5')	46A	77.5-79.0	12-16-19	35	18"
	(BECOMES MEDIUM DENSE AT 79.0')	47A	79.0-80.5	10-15-11	26	16"
	(BECOMES DENSE AT 80.5')	48A	80.5-82.0	9-13-17	30	17"
82.0'	BOTTOM OF BORING					

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B.C. - B.C.M. - K.S.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B.
 C. SHELBY TUBE

BOWSER-MORNER

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

PANY _____

PROJECT _____

BORING No. 504.5-2 DATE _____ SHEET 2 OF 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								20	
10	21.0	22.5	3	2	1	13"		1	FLY ASH TOP OF SAMPLE
11	22.5	24.0	1	2	7	9"		2	CLAYEY SILT - GRAY GREEN - WET - TRACE OF SAND ML
								3	CLAYEY SAND - BR. WET - 100% FINE GRAIN QUARTZ SC
	24.0	26.0				20"		4	3" Shelby Tube
								5	
	26.0	27.5	5	7	13	14"		6	SANDY SILT - BR. - moist to wet - slight plasticity - trace of org. mat. ML
								7	
13	27.5	29.0	9	10	17	16"		8	SANDY SILT - GRAY + BR. moist to wet ORG. MAT. ML
14	29.0	30.5	6	11	15	12"		9	SANDY SILT - BR. - moist - slight plasticity ML
								30	
15	30.5	32.0	9	14	16	14"		1	Same as sample No. 14
16	32.0	33.5	8	13	15	14"		2	Same as sample No. 14
								3	
17	33.5	35.0	7	12	16	17"		4	SAND - BR. moist 100% FINE GRAIN QUARTZ - TRACE OF SILT SC-SP
18	35.0	36.5	8	11	13	16"		5	CLAYEY SILT - GRAY BR. moist - TRACE OF SAND ML
								6	
19	36.5	38.0	9	13	14	18"		7	CLAY SILT - BR. moist - TRACE OF SAND ML
20	38.0	39.5	10	11	17	16"		8	CLAYEY SILT - GRAY BR. moist - TRACE OF ORG. MAT. - TRACE OF SAND ML
								9	
								40	

ENGINEER _____

AMERICAN ELECTRIC POWER SERVICE CORPORATION
AEP CIVIL ENGINEERING LABORATORY
LOG OF BORING

JOB No. _____

COMPANY _____

PROJECT _____

BORING No. 5045-2 DATE _____ SHEET 3 of 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

_____ DATUM

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
21	39.5	41.0	4	6	7	18"		40	clayey silt - Gray Br. moist - trace org. mat. trace of sand Mb
								1	
22	41.0	42.5	5	10	13	18"		2	silty clay - Br. - moist - low to med plasticity - Cl
23	42.5	44.0	4	6	8	18"		3	clayey silt - Br. - wet to saturated. trace of sand. Mb
								4	
24	44.0	45.5	4	7	9	16"		5	same as sample no. 23
								6	
25	45.5	47.0	5	8	10	17"		7	same as sample no. 23
								8	
26	47.0	48.5	3	4	8	16"		9	same as sample no. 23
								10	
27	48.5	50.0	12	13	17	15"		11	sandy silt - Br. wet to saturated 100% fine grain Mb
								12	
28	50.0	51.5	15	16	16	18"		13	same as sample no. 27
								14	
29	51.5	53.0	3	5	8	16"		15	same as sample no. 27 (saturated)
								16	
30	53.0	54.5	8	9	12	16"		17	same as sample no. 27
								18	
31	54.5	56.0	19	19	35	18"		19	same as sample no. 27 (gray Br)
								20	
32	56.0	57.5	9	10	19	18"		21	silty clay - Gray Br moist - low to med. plasticity - strong reaction to HCl - Cl
								22	
33	57.5	59.0	19	27	29	18"		23	silt - Gray Br moist to wet - trace of sand - moderate reaction to HCl - Mb
								24	
34	59.0	60.5	27	21	20	12"		25	same as sample no. 33
								26	
								27	
								28	
								29	
								30	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NO. _____

COMPANY _____

PROJECT _____

BORING NO. 504S-2 DATE _____ SHEET 4 OF 5

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE BLOWS / 6"			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO							
35	60.5	62.0	11	13	19	16"		60	SILT - GRAY MOIST - TRACE OF SAND MODERATE REACTION TO HCL ML
36	62.0	63.5	11	19	40	16"		2	CLAYEY SILT - GRAY MOIST - LOW PLASTICITY - MODERATE REACTION TO HCL ML
37	63.5	65.0	17	21	40	12"		3	SILT - GRAY MOIST - STRONG REACTION TO HCL - TRACE OF SAND ML
38	65.0	66.5	16	23	30	16"		5	SILT - GRAY MOIST - SLIGHT PLASTICITY - STRONG REACTION TO HCL - W/ FINE GRAIN SAND & QUARTZ ML
39	66.5	68.0	22	25	27	16"		6	SAME AS SAMPLE NO. 38
40	68.0	69.5	19	21	23	14"		8	SAME AS SAMPLE NO. 38
41	69.5	71.0	17	19	22	16"		9	SAME AS SAMPLE NO. 38
42	71.5	73.0	15	20	19	14"		70	
43	73.0	74.5	14	24	26	15"		1	SILT - GRAY MOIST TO WET - SLIGHT PLASTICITY - SLIGHT REACTION TO HCL ML
44	74.5	76.0	18	23	24	16"		2	SAME AS SAMPLE NO. 42
45	76.0	77.5	8	12	14	18"		3	SAME AS SAMPLE NO. 42 (MOIST)
46	77.5	79.0	12	16	19	18"		4	CLAYEY SILT - GRAY MOIST - SLIGHT PLASTICITY - SLIGHT REACTION TO HCL ML
47	79.0	80.5	10	15	11	16"		5	SAME AS SAMPLE NO. 45
								6	SILT - GRAY MOIST - SLIGHT PLASTICITY SLIGHT REACTION TO HCL ML
								7	
								8	
								9	
								0	

ENGINEER _____

LOG OF BORING No. **505-1**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/21/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/21/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE					
2.0'	WATER					
4.5'	FLYASH					
80.0'	BOTTOM OF BORING					
NOTE: CONE PENETRATION TEST BORING						

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B.C.-K.G.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

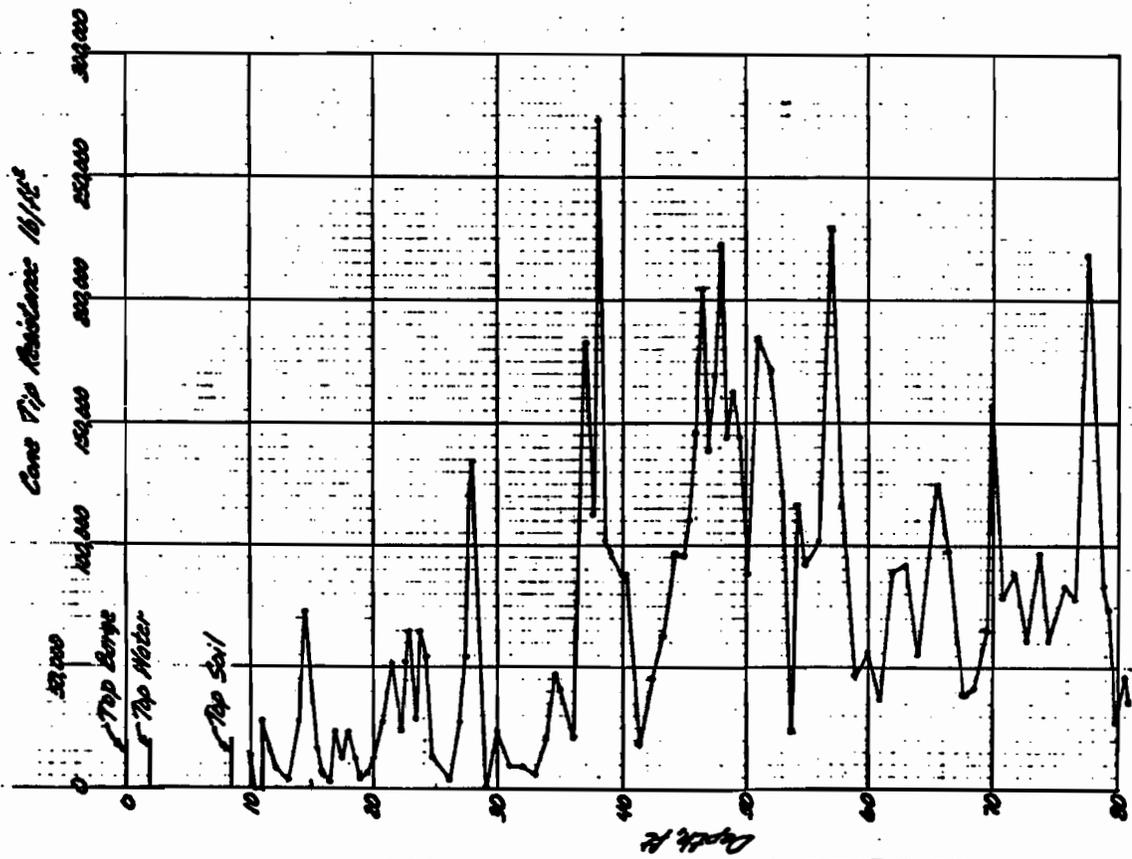
TYPE SAMPLER:
 A. SPLIT SPOON
 B.
 C. SHELBY TUBE

Woodward-Clyde Consultants  CHICAGO	SUBJECT <u>Table 2 Cone Resistance</u>	PROJECT <u>Bowser-Morner</u>
	<u>vs Depth</u>	FILE NO. <u>84C3092</u>
	COMPUTED <u>JHS</u>	CHECKED <u>FWS</u>

<u>Depth, ft</u>	<u>Resistance</u> <u>lb/ft²</u>	<u>Depth, ft</u>	<u>Resistance</u> <u>lb/ft²</u>	<u>Boring</u> <u>50-5-1</u>
10.0	16729	27.8	120022	
10.3	0	28.0	134763	
10.8	0	29.0	0	0
11.0	26023	30.0	27235	1309 psf
12.0	9294	31.0	9294	
13.0	3717	32.0	9294	367 psf
14.0	27417	33.0	5576	107 psf
14.5	72493	34.0	20446	
15.3	17658	34.4	48328	
16.0	5576	34.8	21376	
16.5	3717	35.0	37964	
17.0	23235	36.0	20446	
17.5	10223	37.0	18530	
18.0	23235	37.3	111528	
18.5	14870	38.0	278820	
19.0	4647	38.5	102234	
19.5	5576	39.0	97587	
20.0	13941	40.0	88293	
20.2	13941	40.25	88293	
20.8	288114	41.25	18528	
21.4	52046	42.25	46470	
22.1	24164	43.25	65058	
22.2	53905	44.25	97587	
22.4	44611	45.0	46470	
22.8	64128	45.5	60411	
23.2	28814	46.0	144057	
23.8	64128	46.5	204468	
24.2	55764	47.0	139410	
24.8	13011	47.5	167292	
26.0 <i>organic</i>	3717	48.0	223056	
27.0	20446	48.5	144057	
27.5	53905	49.0	162645	

Table 2

<u>depth, ft</u>	<u>Resistance lb/ft²</u>	<u>depth, ft</u>	<u>Resistance lb/ft²</u>
49.5	144057	76.75	78999
50.0	88293	77.75	218409
51.2	185880	78.75	83646
52.2	171939	79.3	74352
53.2	120822	79.6	27882
53.7	23235	80.6	46470
54.2	116175	80.9	35529
55.0	92940		
56.0	102234		
57.0	227703		
58.0	116175		
59.0	46470		
60.0	55764		
60.25	37176		
61.25	37176		
62.25	88293		
63.25	92940		
64.25	55764		
65.5	125469		
66.5	97587		
67.5	37176		
68.5	41823		
69.5	65058		
69.9	157998		
70.9	78999		
71.9	88293		
72.9	60411		
73.9	97587		
74.3	60411		
74.75	23235		
75.75	83646		



CONE PENETRATION TEST
CLIFTY CREEK PUMPER PLANT
BOWSER-MORNER

Fig. 4
12/2/1984

WCC SMC309E

BORING SO-5-1
DRILLED 9-21-1984

LOG OF BORING No. SO 5.5-1

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/7/84

SURFACE ELEVATION: 502.2 (WATER)

DATE COMPLETED: 9/7/84

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE					
2.0'	WATER					
8.0'	FLYASH	1A	8.5-10.0	1-1-1	2	18"
		2A	13.5-15.0	0-0-0	0	18"
16.5'	HARD BROWN SILT, TRACE SAND, TRACE OF CLAY - DAMP	3A	18.5-20.0	14-23-31	54	18"
22.0'	VERY STIFF BROWN SILT, SOME SAND, TRACE OF CLAY - MOIST (1" TO 6" CLAY LAYERS)	4A	23.5-25.0	7-14-16	30	14"
	(BECOMES HARD AT 28.5')	5A	28.5-30.0	19-28-36	64	18"
32.0'	HARD BROWN SILT, SOME SAND, TRACE CLAY, TRACE OF ROCK FRAGMENTS - DAMP	6A	33.5-34.0	50/3"	50/3"	0"
		7A	38.5-39.0	100/4"	100/4"	4"

DRILLING METHOD: HOLLOW STEM AUGER
 DRILLER: B. C. - B. C. M. - R. G.
 JOB NO.: 28089

WATER OBSERVATIONS
 INITIAL DEPTH: 2.0'
 COMPLETION DEPTH: 2.0'
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B. NXWL ROCK CORE
 C. SHELBY TUBE

AMERICAN ELECTRIC POWER SERVICE CORPORATION

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

FORM CE-5
5/81

J. No. _____

C. NY _____

PROJECT _____

BORING No. 505.5-1 DATE _____ SHEET 2 of 3

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATUM _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
ME	
TE	

SAMPLER NO.	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"						
								20	
								1	
	23.5	25.0	7	14	16	14"		2	Clayey silt - yellowish Br. - moist - slight plasticity.
								3	Cl
								4	
								5	
								6	
								7	
5	28.5	30.0	19	28	36	18"		8	Silty clay - lt. Br. - moist. Strong Reaction to HCl - Trace of lime stone frag.
								9	Cl
								30	
								1	
								2	
								3	
6	33.5	33.8	59/3			0		4	
								5	
								6	
								7	
								8	
	38.5	38.9	100/4			4"		8	Clayey sand + gravel - yellowish Br. moist - poorly graded - 1" max size (sand + gravel limestone)
								9	GC
								40	

ENGINEER _____

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB No. _____

BORING No. 505.5-1 DATE _____ SHEET 3 OF 3

TYPE OF BORING _____ RIG _____

CASING USED _____ SIZE _____ DRILLING MUD USED _____

BORING BEGUN _____ BORING COMPLETED _____

GROUND ELEVATION _____ REFERRED TO _____ DATE _____

FIELD PARTY: _____

LOCATION OF BORING:	
WATER LEVEL	
TIME	
DATE	

SAMPLE NO	SAMPLE DEPTH IN FEET		STANDARD PENETRATION RESISTANCE		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION
	FROM	TO	BLOWS / 6"					SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
							40	
							1	
							2	
							3	
8	43.5	43.9	47	53/4	8"		3	Gray calcareous shale + lime stone Frag.
							4	
							5	
							6	Stopped boring 43.9
							7	
							8	
							9	
							0	
							1	
							2	
							3	
							4	
							5	
							6	
							7	
							8	
							9	
							0	

ENGINEER _____

LOG OF BORING No. **SO 6-1****CLIFTY CREEK FLYASH IMPOUNDMENT**BORING LOCATION: **AS DIRECTED**DATE STARTED: **9/8/84**SURFACE ELEVATION: **502.2' (WATER)**DATE COMPLETED: **9/8/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARSA					
2.0'	WATER					
6.0'	FLYASH	1A	8.5-10.0	0-1-0	1	14"
		2A	13.5-15.0	0-1-1	2	16"
		3A	18.5-20.0	0-1-0	1	16"
		4A	23.5-25.0	0-0-1	1	15"
		5A	28.5-30.0	2-3-2	5	18"
32.0'	STIFF BROWN SILT, SOME SAND TRACE OF CLAY - MOIST	6A	33.5-35.0	3-4-9	13	16"
39.0'	STIFF GRAY SILT, TRACE SAND TRACE OF CLAY - MOIST	7A	38.5-40.0	4-5-8	13	12"
		8A	43.5-45.0	5-7-11	18	18"
DRILLING METHOD: HOLLOW STEM AUGER		WATER OBSERVATIONS		TYPE SAMPLER:		
DRILLER: B.C. - K.G. - B.C.M.		INITIAL DEPTH: 2.0'		<input checked="" type="checkbox"/> A. SPLIT SPOON		
JOB NO.: 28089		COMPLETION DEPTH: 2.0'		<input type="checkbox"/> B.		
		DEPTH AFTER HRS.		<input type="checkbox"/> C. SHELBY TUBE		

BOWSER-MORNER
TESTING LABORATORIES, INC.

LOG OF BORING No. 506-1

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/8/84

SURFACE ELEVATION: 502.2' (WATER)

DATE COMPLETED: 9/8/84

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
	(BECOMES HARD AT 48.5')	9A	48.5-50.0	9-13-18	31	16"
		10A	53.5-55.0	8-14-19	33	18"
	(BECOMES VERY STIFF AT 58.5')	11A	58.5-60.0	6-9-13	22	18"
63.5'	VERY STIFF GRAY SILT, TRACE SAND, TRACE CLAY, TRACE ORGANICS - MOIST	12A	63.5-65.0	7-8-12	20	18"
69.5'	VERY STIFF GRAY CLAY WITH LIMESTONE LAYERS - DAMP	13A	68.5-70.0	19-11-8	19	18"
		14A	73.5-75.0	17-13-16	29	4" #
		15A	78.5-80.0	11-14-17	31	8"
81.5'	AUGER REFUSAL (APPARENT ROCK) BOTTOM OF BORING					

DRILLING METHOD: HOLLOW STEM AUGER DRILLER: B.C.-K.F.-B.C.M. JOB NO.: 28089	WATER OBSERVATIONS INITIAL DEPTH: 2.0' COMPLETION DEPTH: 2.0' DEPTH AFTER HRS.	TYPE SAMPLER: <input checked="" type="checkbox"/> A. SPLIT SPOON <input type="checkbox"/> B. <input type="checkbox"/> C. SHELBY TUBE
---	--	---

2B12 * ROCK IN DRIVE SHOE

BOWSER-MORNER TESTING LABORATORIES, INC.

LOG OF BORING No. **S06-2**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/20/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/20/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE			NOTE: CONE PENETRATION TEST BEING TO 28'		
2.0'	WATER					
7.0'	FLYASH					
28.0'	WEATHERED SHALE/LIMESTONE FRAGMENTS	1A	28-28.5	100/1"	100/1"	1"
		2A	34-34.5	100/3"	100/3"	3"
39.0'	GRAY SHALE	3A	39-39.5	100/4"	100/4"	4"
		4A	44-44.5	100/3"	100/3"	3"
50.0'	LIMESTONE/ SHALE	5A	49-49.5	100/1"	100/1"	0"
60.0'	BOTTOM OF BORING	1B	50-60	-	-	110"

DRILLING METHOD: **HOLLOW STEM AUGER**

DRILLER: **B. C. - K. G.**

JOB NO.: **28089**

WATER OBSERVATIONS

INITIAL DEPTH: **2.0'**

COMPLETION DEPTH: **2.0'**

DEPTH AFTER HRS.

TYPE SAMPLER:

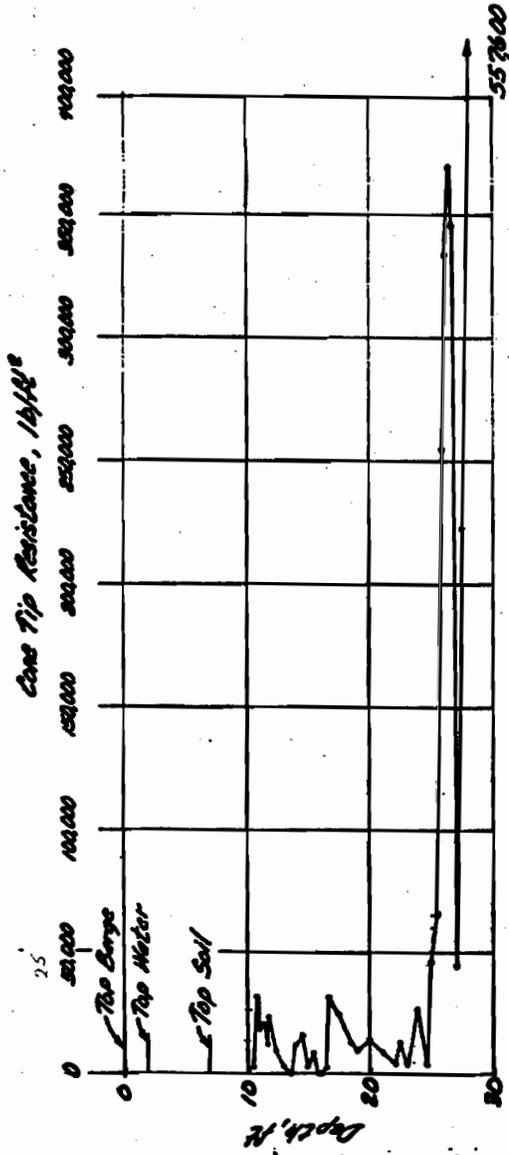
- A. SPLIT SPOON
- B. NXW ROCK CORE
- C. SHELBY TUBE

BOWSER-MORNER TESTING LABORATORIES, INC.

<u>depth, ft</u>	<u>Resistance lb/ft²</u>	<u>depth, ft</u>	<u>Resistance lb/ft²</u>
10.5	3345	26.0	324580
10.75	32343	26.25	371760
11.0	17286	26.5	345700
11.25	20075	27.0	44610
11.5	13383	27.5	223050
11.8	23420	28.0	557040
12.3	9479		
13.0	2230		
13.5	557		
13.8	557		
14.0	11152		
14.5	16729		
15.0	3345		
15.5	7806		
15.75	0		
16.0	0		
16.5	2230		
16.8	31600		
17.5	24165		
18.5	13010		
19.0	10223		
19.5	15800		
20.1	14870		
21.0	9295		
22.0	4460		
22.25	13940		
23.0	4460		
23.75	27325		
24.5	4460		
25	46840		
25.5	66916		
25.75	255580		

Boring 50-6-2

Table 3



BORING 50-6-2
 DRILLED 9-20-1984

**CONE PENETRATION TEST
 CLIFTY CREEK POWER PLANT
 BOWSER-MARNER**

Fig. 5

1 Oct 1984

HCC 8943012

LOG OF BORING No. **SO 6.5-1**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/9/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/9/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE					
2.0'	WATER					
4.5'	FLYASH	1A	4.5-6.0	0-0-0	0	18"
		2A	6.0-7.5	0-0-0	0	18"
		1C	7.5-9.5	-	-	0"
		3A	9.5-11.0	0-0-0	0	18"
		4A	11.0-12.5	0-0-0	0	18"
		5A	12.5-14.0	0-0-0	0	18"
		6A	14.0-15.5	0-1-1	2	16"
		7A	15.5-17.0	0-0-1	1	18"

DRILLING METHOD: HOLLOW STEM AUGER DRILLER: B. C. B.C.M. - K. G. JOB NO.: 28089	WATER OBSERVATIONS INITIAL DEPTH: 2.0' COMPLETION DEPTH: 2.0' DEPTH AFTER HRS.	TYPE SAMPLER: <input checked="" type="checkbox"/> A. SPLIT SPOON <input checked="" type="checkbox"/> B. NXWL ROCK CORE <input checked="" type="checkbox"/> C. SHELBY TUBE
--	--	--

BOWSER-MORNER

LOG OF BORING No. 50 6.5-1

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/9/84

SURFACE ELEVATION: 502.2' (WATER)

DATE COMPLETED: 9/9/84

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
		8A	17.0-18.5	0-1-1	2	18"
		2C	18.5-20.5	—	—	21"
21.0'	HARD BROWN SILT, SOME SAND, TRACE OF CLAY - MOIST	9A	20.5-22.0	15-17-16	33	16"
		3C	22.0-24.0	—	—	13"
24.0'	VERY SOFT BROWN SILT, AND SAND - WET	10A	24.0-25.5	3-1-2	3	16"
	(BECOMES MEDIUM STIFF AT 25.5')	11A	25.5-27.0	3-4-4	8	18"
	(BECOMES SOFT AT 27.0')	12A	27.0-28.5	2-2-3	5	14"
	(BECOMES MEDIUM STIFF AT 28.5')	13A	28.5-30.0	1-2-5	7	18"
30.0'	VERY STIFF BROWN CLAY, SOME SILT, TRACE OF SAND - MOIST	14A	30.0-31.5	6-8-10	18	17"
	(BECOMES HARD AT 31.5')	15A	31.5-33.0	12-14-17	31	16"
DRILLING METHOD: HOLLOW STEM AUGER		WATER OBSERVATIONS		TYPE SAMPLER:		
DRILLER: B.C. - B.C.M. - K. S.		INITIAL DEPTH: 2.0'		<input checked="" type="checkbox"/> A. SPLIT SPOON		
JOB NO.: 28089		COMPLETION DEPTH: 2.0'		<input checked="" type="checkbox"/> B. NXWL ROCK CORE		
		DEPTH AFTER HRS.		<input checked="" type="checkbox"/> C. SHELBY TUBE		

BOWSER-MORNER

LOG OF BORING No. **SO 6.5-1**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/9/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/9/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
34.0	VERY STIFF BROWN SILT, SOME SAND, TRACE OF CLAY-MOIST	16A	33.0-34.5	11-13-16	29	18"
		17A	34.5-36.0	6-12-16	28	14"
	(BECOMES HARD AT 44.0')	18A	36.0-37.5	21-21-23	44	16"
37.0	HARD GRAY CLAY, SOME SILT, TRACE OF SAND-MOIST	19A	37.5-39.0	11-14-18	31	18"
	(BECOMES VERY STIFF AT 39.0')	20A	39.0-40.5	5-8-13	21	18"
		21A	40.5-42.0	11-12-13	25	18"
		22A	42.0-43.5	6-7-13	20	18"
		23A	43.5-45.0	6-9-14	23	18"
		24A	45.0-46.5	6-9-11	20	18"

DRILLING METHOD: **HOLLOW STEM AUGER**

DRILLER: **B.C. B.C.M. K.G.**

JOB NO.: **28089**

WATER OBSERVATIONS

INITIAL DEPTH: **2.0'**

COMPLETION DEPTH: **2.0'**

DEPTH AFTER HRS.

TYPE SAMPLER:

- A. SPLIT SPOON
- B. NX1 ROCK CORE
- C. SHELBY TUBE

BOWSER-MORNER

LOG OF BORING No. **SO 6.5-1**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/9/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/9/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
		25A	465-480	4-10-10	20	18"
48.0'	VERY STIFF GRAY CLAY, SOME SILT, ROCK FRAGMENTS - MOIST	26A	480-495	9-10-13	23	15"
	(BECOMES HARD AT 49.5')	27A	495-510	13-16-15	31	17"
		28A	510-520	14-70/3"	89/9"	9"
		29A	525-535	40-60/2"	100/8"	4"
55.0'	LIMESTONE/SHALE	1B	550-650	-	-	110"
65.0'	BOTTOM OF BORING					

DRILLING METHOD: **HOLLOW STEM AUGERS**
 DRILLER: **B. C. - B. C. M. - K. S.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B. NXWL ROCK CORE
 C. SHELBY TUBE

BOWSER-MORNER

LOG OF BORING No. **S07-1**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/19/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/19/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE					
2.0'	TOP OF WATER					
5.0'	FLYASH					
	1" TO 3" LIMESTONE LAYERS					
56.0'	1" TO 1' WEATHERED SHALE					
59.0'	LIMESTONE / SHALE	1B	59'-69'	-	-	120"
69.0'	BOTTOM OF BORING					
NOTE: CONE PENETRATION TEST BORING						

DRILLING METHOD: **HOLLOW STEM AUGER**
 DRILLER: **B.C. - K.G.**
 JOB NO.: **28089**

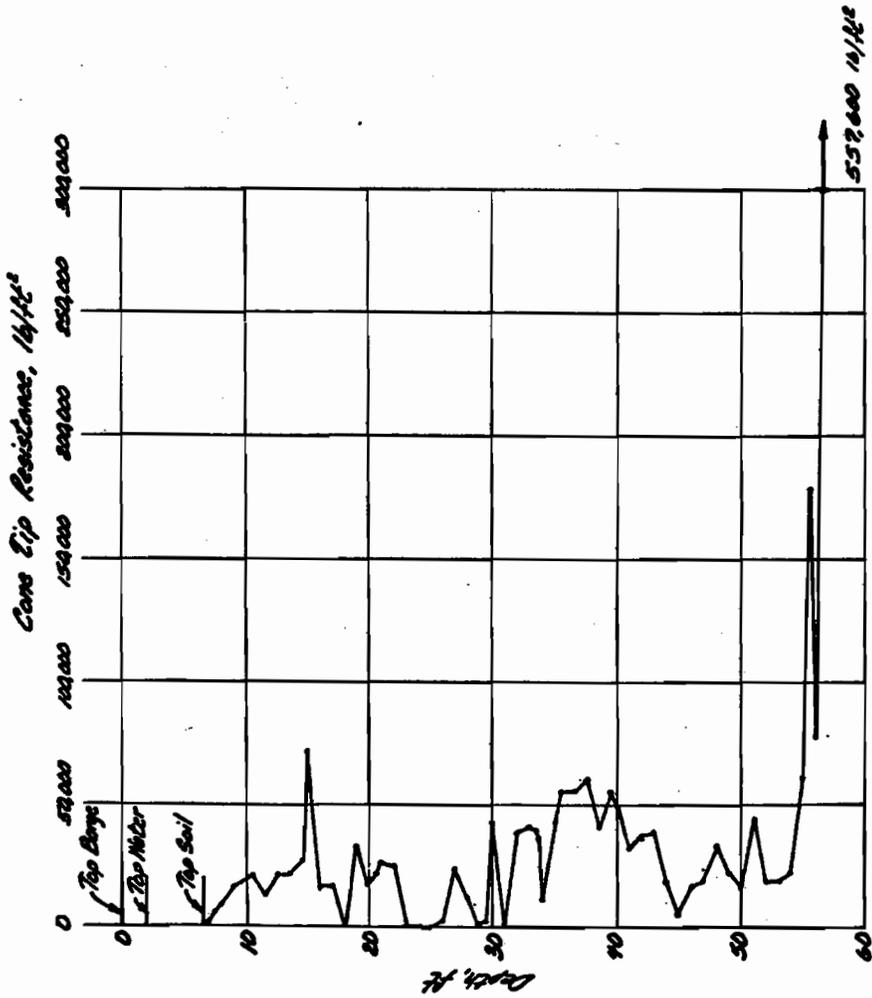
WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B. NXWL ROCK CORE
 C. SHELBY TUBE

Woodward-Clyde Consultants CHICAGO	SUBJECT <u>Table 4 Cone Resistance</u>	PROJECT <u>Bowser-Morner</u>
	<u>vs Depth</u>	FILE NO. <u>84C3092</u>
COMPUTED <u>JTB</u>	CHECKED <u>Fw)</u>	DATE <u>9-20-84</u> PAGE <u>1</u> OF <u>1</u> PAGES

<u>depth, ft</u>	<u>Resistance lb/ft²</u>	<u>depth, ft</u>	<u>Resistance lb/ft²</u>	Boring SD7-7
7.0	1115	35.0	43681	
7.5	5576	35.5	55764	
8.0	8364	36.5	55764	
9.0	16729	37.5	61340	
10.5	21747	38.5	41823	
11.5	13941	39.5	55764	
12.5	22305	40.0	50187	
13.5	22305	41.0	33458	
14.5	27882	42.0	36246	
15.0	72493	43.0	39634	
16.0	16729	44.0	19517	
17.0	16729	45.0	5576	
18.0	0	46.0	16724	
19.0	33458	47.0	19517	
20.0	16729	48.0	33458	
20.5	21376	49.0	22305	
21.0	26023	50.0	16729	
22.0	25093	51.0	44611	
23.0	557	52.0	19517	
24.4	0	53.0	19517	
24.9	0	54.0	22305	
25.9	3345	55.0	61340	
26.9	24164	55.3	66916	
27.9	12082	55.5	178444	
28.9	1115	56.0	78069	
29.4	6691	56.3	557640	
30.0	43681			
31.0	2230			
32.0	39034			
33.0	41823			
33.5	39034			
33.8	33458			
34.0	11152			

Table 4



BORING SD-7-1
 DRILLED 9-17-1984

CONE PENETRATION TEST
 CLIFTY CREEK POWER PLANT
 BOWSER-MARKER
 Fig. 6
 WCC 896308 1 Oct. 1984

LOG OF BORING No. **SO 7-2**

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: **AS DIRECTED**

DATE STARTED: **9/10/84**

SURFACE ELEVATION: **502.2' (WATER)**

DATE COMPLETED: **9/10/84**

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE					
2.0'	WATER					
5.0'	FLYASH	1A	50-6.5	0-1-1	2	12"
		2A	8.5-10.0	1-2-2	4	14"
		3A	13.5-15.0	0-0-0	0	10"
		4A	18.5-20.0	0-0-0	0	10"
		5A	23.0-25.0	1-1-2	3	16"
28.0'	MEDIUM STIFF GRAY CLAY, SOME SILT, TRACE OF ORGANICS-DAMP	6A	28.5-30.0	1-2-4	6	10"
34.0'	VERY STIFF BROWN & GRAY CLAY, SOME SILT-DAMP	7A	33.5-35.0	4-9-17	26	14"

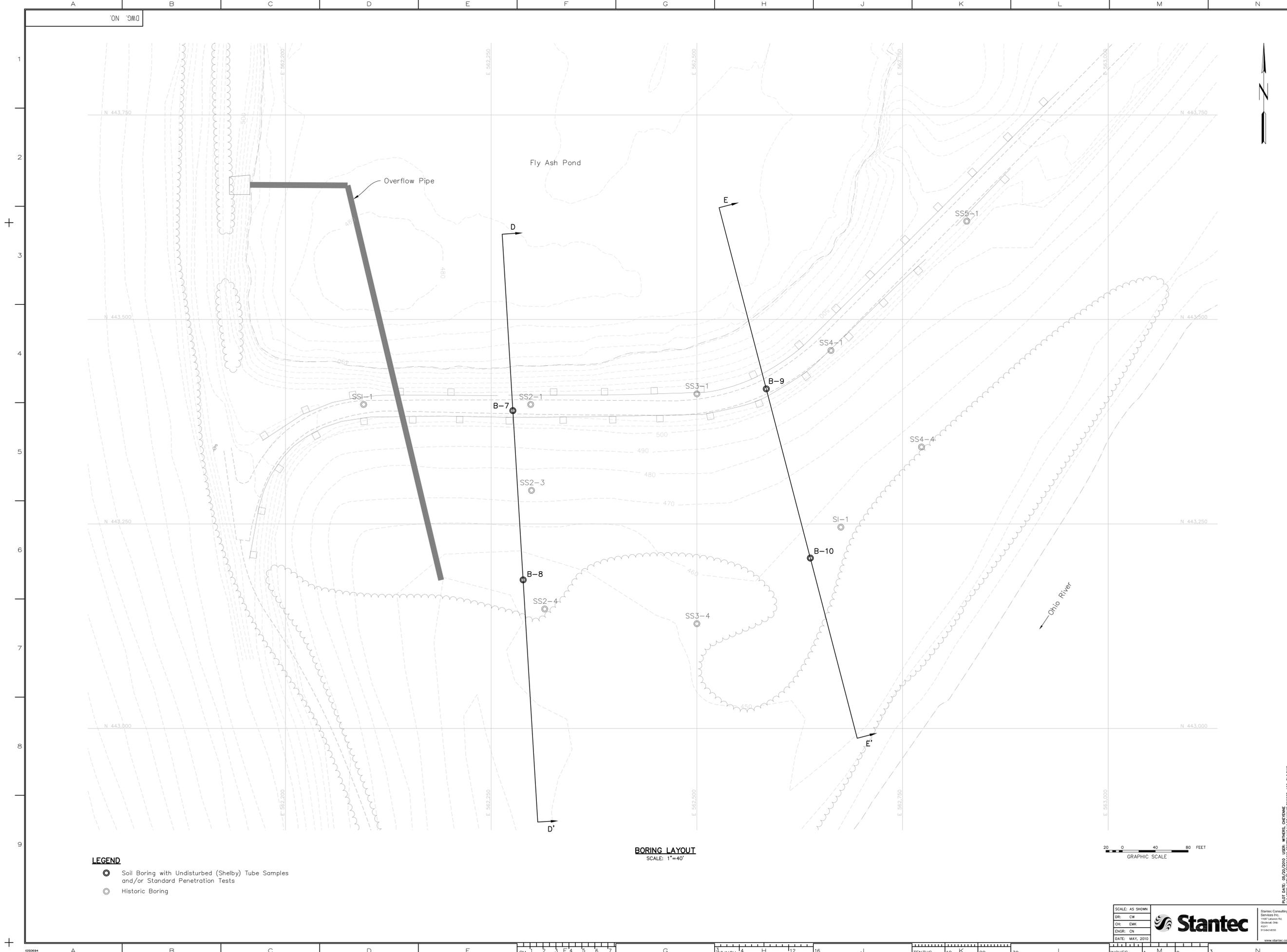
DRILLING METHOD: **HOLLOW-STEM AUGER**
 DRILLER: **B.C. - B.C.M. - K.G.**
 JOB NO.: **28089**

WATER OBSERVATIONS
 INITIAL DEPTH: **2.0'**
 COMPLETION DEPTH: **2.0'**
 DEPTH AFTER HRS.

TYPE SAMPLER:
 A. SPLIT SPOON
 B. NXW ROCK CORE
 C. SHELBY TUBE

BOWSER-MORNER

Stantec (2016)



NOTES

REFERENCE DRAWINGS

DATE	NO.	DESCRIPTION	APPR.
REVISIONS			

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INDIANA - KENTUCKY ELECTRIC CO.
CLIFTY CREEK PLANT
 MADISON INDIANA
 GEOTECHNICAL EXPLORATION
 LANDFILL RUNOFF
 COLLECTION POND DAM
 BORING LAYOUT

DWG. NO. CIVIL ENGINEERING DIVISION

APPROVED BY: [Signature]
 DATE: [Blank]
 ENGR: [Blank]
 ARCH: [Blank]
 CH: [Blank]
 DR: [Blank]

AMERICAN ELECTRIC POWER
 AEP SERVICE CORP.
 1 RIVERSIDE PLAZA
 COLUMBUS, OH 43215

BORING LAYOUT
 SCALE: 1"=40'



- LEGEND**
- Soil Boring with Undisturbed (Shelby) Tube Samples and/or Standard Penetration Tests
 - Historic Boring

SCALE: AS SHOWN
 DR: CW
 CH: EMK
 ENGR: CN
 DATE: MAY, 2010

Stantec Consulting Services Inc.
 11011 Lorain Rd.
 Dublin, Ohio 43017
 614-244-0000
 www.stantec.com

AMERICAN ELECTRIC POWER
 AEP SERVICE CORP.
 1 RIVERSIDE PLAZA
 COLUMBUS, OH 43215



LANDFILL RUNOFF COLLECTION POND:
2009 GEOTECHNICAL EXPLORATION

Project Number	175539022	Location	Crest: LRCP Dam	
Project Name	AEP Clifty Creek / Ash Ponds	Boring No.	B-7	Total Depth 29.0 ft
County	Jefferson, IN	Surface Elevation	503.4 ft	
Project Type	Geotechnical Exploration	Date Started	11/12/09	Completed 11/12/09
Supervisor	C. Nisingizwe Driller M. Wethington	Depth to Water	Dry	Date/Time 11/12/09
Logged By	C. Nisingizwe	Depth to Water	N/A	Date/Time N/A

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	
503.4'	0.0'	Top of Hole							
502.9'	0.5'	Asphalt pavement and gravel base							
		Lean Clay, yellow and light gray, moist, stiff							
				ST-1	23.0 - 25.0	2.0		--	
				ST-2	25.0 - 27.0	2.0		20	
				ST-3	27.0 - 29.0	2.0		20	
474.4'	29.0'	No Refusal / Bottom of Hole							

STANTEC/FNSM_LEGACY 175539022 CLIFTY CREEK.GPJ FNSM-GRAPHIC.LOG.GDT 4/16/10

Project Number	175539022	Location	Toe: LRCP Dam	
Project Name	AEP Clifty Creek / Ash Ponds	Boring No.	B-8	Total Depth 31.0 ft
County	Jefferson, IN	Surface Elevation	441.5 ft	
Project Type	Geotechnical Exploration	Date Started	11/19/09	Completed 11/19/09
Supervisor	C. Nisingizwe Driller Danny Jessie	Depth to Water	Dry	Date/Time 11/19/09
Logged By	C. Nisingizwe	Depth to Water	N/A	Date/Time N/A

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	
441.5'	0.0'	Top of Hole							
		Silty Clay, yellow and light gray, damp to moist							
425.5'	16.0'	Lean Clay, yellowish brown and light gray, moist							
				ST-1	25.0 - 27.0	2.0		25	
				ST-2	27.0 - 29.0	2.0		26	
412.5'	29.0'			ST-3	29.0 - 31.0	2.0		23	
410.5'	31.0'	Lean Clay With Sand, yellowish brown and light gray, moist							

No Refusal /
Bottom of Hole

STANTEC/FNSM_LEGACY 175539022 CLIFTY CREEK.GPJ FNSM-GRAPHIC.LOG.GDT 4/16/10

Project Number	<u>175539022</u>	Location	<u>Crest: LRCP Dam</u>	
Project Name	<u>AEP Clifty Creek / Ash Ponds</u>	Boring No.	<u>B-9</u>	Total Depth <u>22.0 ft</u>
County	<u>Jefferson, IN</u>	Surface Elevation	<u>504.3 ft</u>	
Project Type	<u>Geotechnical Exploration</u>	Date Started	<u>11/12/09</u>	Completed <u>11/12/09</u>
Supervisor	<u>C. Nisingizwe</u> Driller <u>M. Wethington</u>	Depth to Water	<u>Dry</u>	Date/Time <u>11/12/09</u>
Logged By	<u>C. Nisingizwe</u>	Depth to Water	<u>N/A</u>	Date/Time <u>N/A</u>

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	
504.3'	0.0'	Top of Hole							
503.8'	0.5'	Asphalt pavement and gravel base							
		Lean Clay, yellowish brown and light gray, damp to moist							
				ST-1	16.0 - 18.0	2.0		22	
				ST-2	18.0 - 20.0	2.0		19	
				ST-3	20.0 - 22.0	2.0		20	
482.3'	22.0'								

No Refusal /
Bottom of Hole

STANTEC/FNSM_LEGACY 175539022 CLIFTY CREEK.GPJ FNSM-GRAPHIC.LOG.GDT 4/16/10

Project Number	<u>175539022</u>	Location	<u>Toe: LRCP Dam</u>	
Project Name	<u>AEP Clifty Creek / Ash Ponds</u>	Boring No.	<u>B-10</u>	Total Depth <u>18.0 ft</u>
County	<u>Jefferson, IN</u>	Surface Elevation	<u>457.3 ft</u>	
Project Type	<u>Geotechnical Exploration</u>	Date Started	<u>11/19/09</u>	Completed <u>11/19/07</u>
Supervisor	<u>C. Nisingizwe</u> Driller <u>Danny Jessie</u>	Depth to Water	<u>Dry</u>	Date/Time <u>11/19/07</u>
Logged By	<u>C. Nisingizwe</u>	Depth to Water	<u>N/A</u>	Date/Time <u>N/A</u>

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	Remarks
Elevation	Depth		Rock Core						
457.3'	0.0'	Top of Hole							
		Silty Clay With Sand, yellow and light gray, damp to moist							
444.1'	13.2'			ST-1	12.0 - 14.0	1.5		17	
441.3'	16.0'	Silty Sand, gray to brown, damp to moist		ST-2	14.0 - 16.0	2.0		10	
439.3'	18.0'	Silty Clay With Sand, yellow and light gray, damp to moist		ST-3	16.0 - 18.0	2.0		25	

No Refusal /
Bottom of Hole

STANTEC/FNSM_LEGACY 175539022 CLIFTY CREEK.GPJ FNSM-GRAPHIC LOG.GDT 4/16/10

LANDFILL RUNOFF COLLECTION POND:
2015 GEOTECHNICAL EXPLORATION

Project Number	175553022	Location	Landfill Runoff Collection Pond Dam		
Project Name	CCR Rule - AEP Clifty Creek	Boring No.	B-12	Total Depth	101.5 ft
County	Jefferson, IN	Surface Elevation	503.9 (estimated)		
Project Type	Geotechnical Exploration	Date Started	7/6/15	Completed	7/7/15
Supervisor	C. Nisingizwe	Driller	E. Caudill	Depth to Water	60.0 ft
Logged By	C. Nisingizwe	Depth to Water	N/A	Date/Time	7/7/15
				Date/Time	N/A

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	Remarks
Elevation	Depth		Rock Core						
503.9	0.0	Top of Hole							
(estimated)	0.4	Asphalt and base							
		Lean Clay With Sand, gray, damp, medium stiff to stiff		SPT-1	1.0 - 2.5	1.5	1-2-5	21	Pocket Penetrometer (PP) = 2.50 tsf
				SPT-2	5.0 - 6.5	1.5	3-3-4	20	PP = 2.50 tsf
				SPT-3	10.0 - 11.5	1.2	3-4-5	23	PP = 3.50 tsf
				SPT-4	15.0 - 16.5	1.0	3-3-5	19	PP = 2.50 tsf
				SPT-5	20.0 - 21.5	0.9	4-6-9	18	PP = 2.50 tsf
				SPT-6	25.0 - 26.5	1.1	3-5-7	18	PP = 4.25 tsf
				SPT-7	30.0 - 31.5	1.3	2-5-8	19	PP = 4.50 tsf
				SPT-8	35.0 - 36.5	0.9	WOH-3-4	18	PP = 4.00 tsf

STANTEC\FISM_LEGACY_GINT.LOG.GPJ FISM-GRAPHIC.LOG.GDT 8/6/15

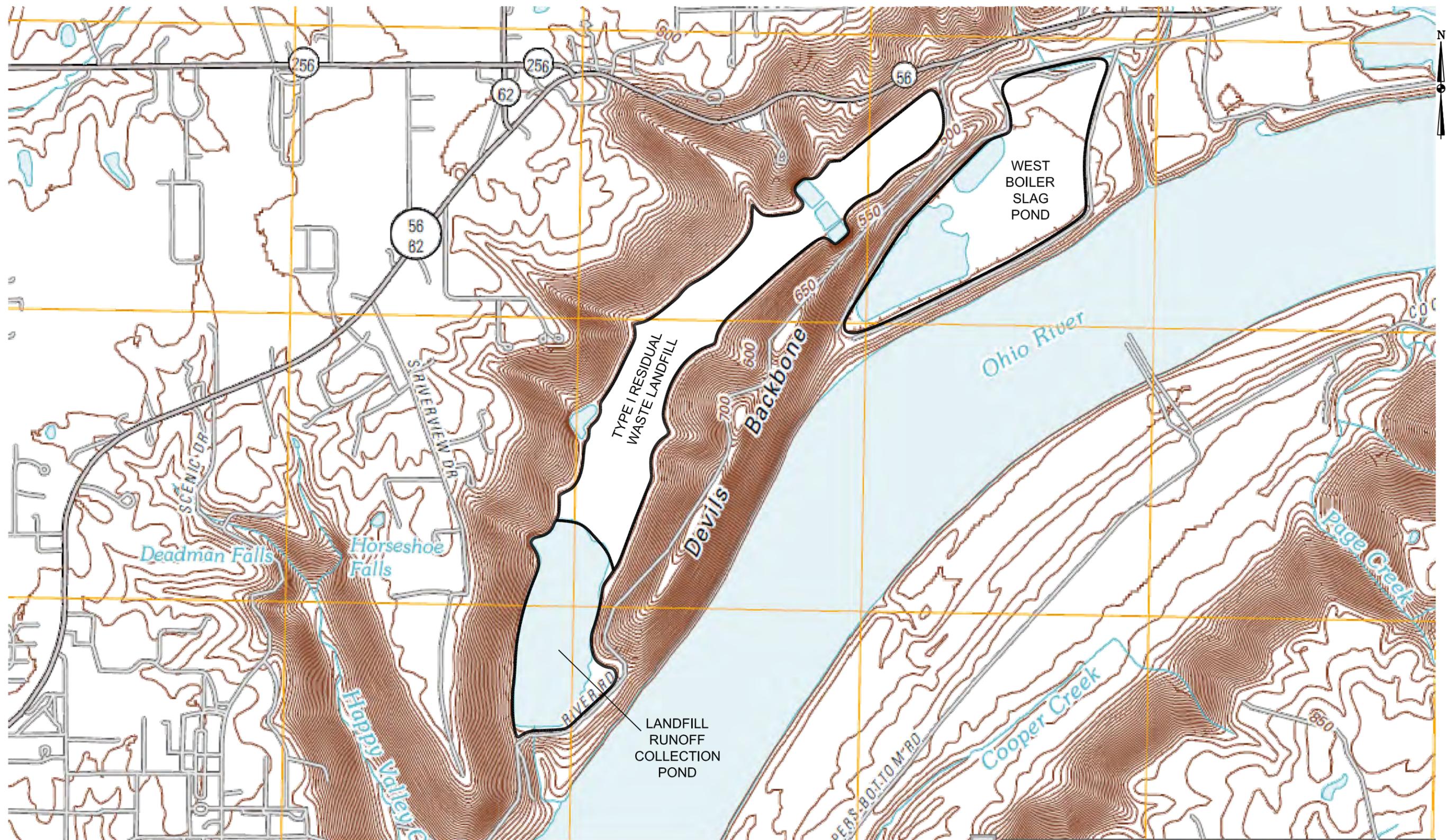
Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	
	40.0	Lean Clay With Sand, gray, damp, medium stiff to stiff <i>(Continued)</i>							
	50.0	Silty Clay With Sand, brown, moist, medium stiff to very stiff		SPT-9	40.0 - 41.5	1.5	6-8-8	16	
			SPT-10	45.0 - 46.5	1.5	1-3-5	19		
	58.0	Silt With Sand, grayish light brown, moist, medium stiff to stiff		SPT-11	50.0 - 51.5	1.5	2-3-3	22	
			SPT-12	55.0 - 56.5	1.0	2-5-8	20		
	63.5	Silty Sand, grayish light brown, damp, very stiff		SPT-13	60.0 - 61.5	1.4	3-11-17	15	
	70.0	Silt With Sand, grayish light brown, wet, stiff		SPT-14	65.0 - 66.5	1.5	2-3-8	28	
	78.0	Sand, mottled gray and brown, moist to wet, medium stiff to stiff		SPT-15	70.0 - 71.5	1.5	3-5-5	22	
			SPT-16	75.0 - 76.5	1.3	2-3-5	28		

STANTECFRSM_LEGACY_GINT.LOG.GPJ_FNSM-GRAPHIC.LOG.GDT_8/6/15

Project Number		175553022			Location		Landfill Runoff Collection Pond Dam				
Project Name		CCR Rule - AEP Clifty Creek			Boring No.		B-12		Total Depth		101.5 ft
Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	Remarks		
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth			
		Silt, gray, moist to wet, medium stiff to stiff		SPT-17	80.0 - 81.5	1.5	6-9-6	26			
			SPT-18	85.0 - 86.5	1.5	2-3-5	28				
	90.0	Lean Clay, gray, moist, medium stiff to very stiff		SPT-19	90.0 - 91.5	1.5	2-4-4	25	PP = 2.25 tsf		
			SPT-20	95.0 - 96.5	1.5	5-8-11	23	PP = 3.75 tsf			
	101.5		SPT-21	100.0 - 101.5	1.5	4-6-8	27	PP = 3.50 tsf			
		No Refusal / Bottom of Hole									

STANTEC\FM\LEGACY_GINT\LOG.GPJ_FMSM\GRAPHIC\LOG.GDT_8/6/15

AGES (2018)



SOURCE: USGS MADISON WEST 7.5 MINUTE TOPOGRAPHIC QUADRANGLE, 2010.

DRAWN BY	JM
DATE	
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JOB NO.	2015067-CLIF
DWG FILE	IKEC_Clifty MW Install_USGS TOPO b05.dwg
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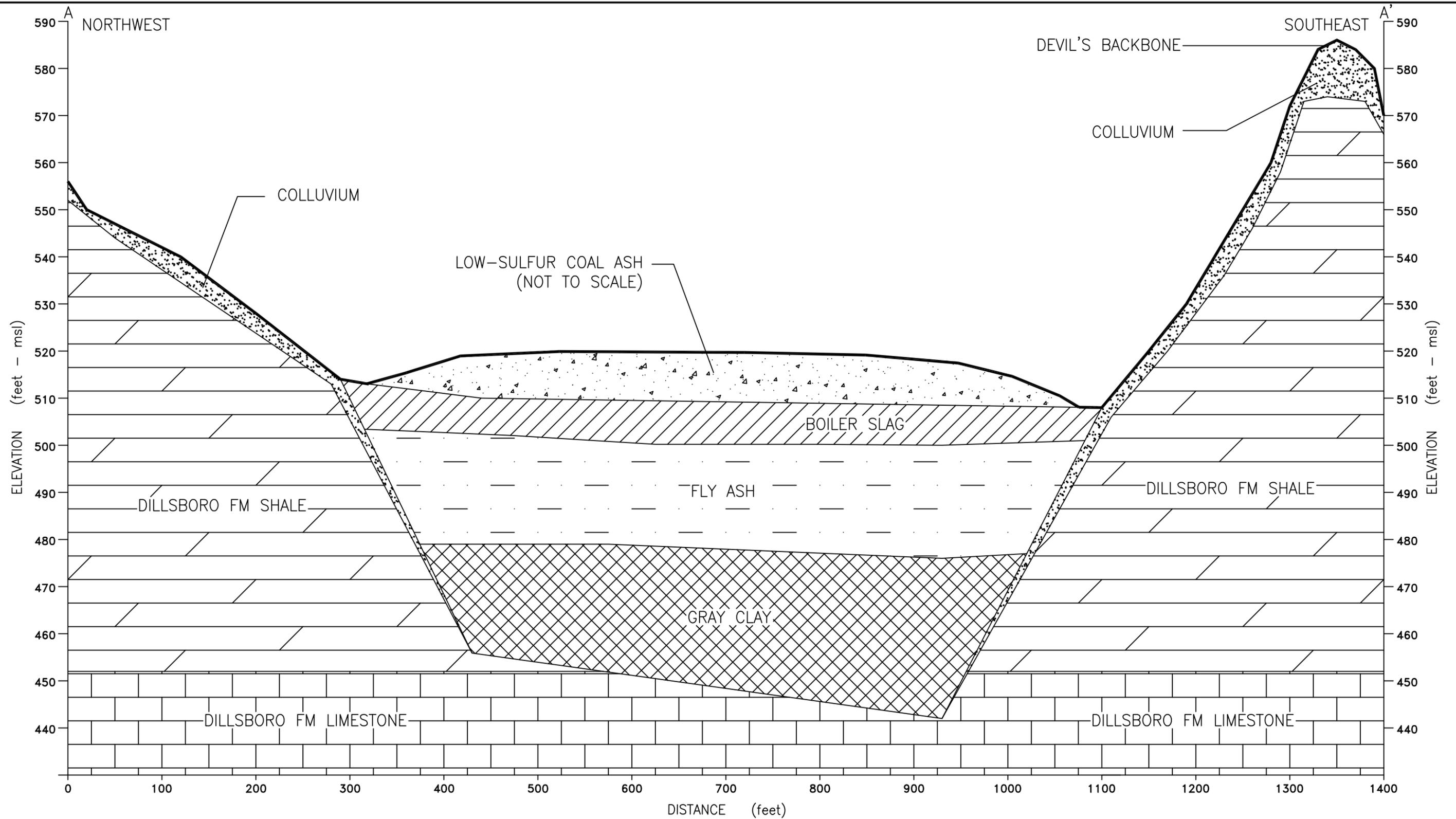
CLIFTY CREEK STATION
MADISON, INDIANA
TOPOGRAPHIC MAP

DRAWING NAME

FIGURE 2

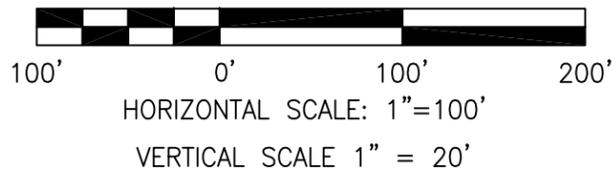
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NOTE:
CROSS-SECTION LOCATION SHOWN
ON FIGURE 4.

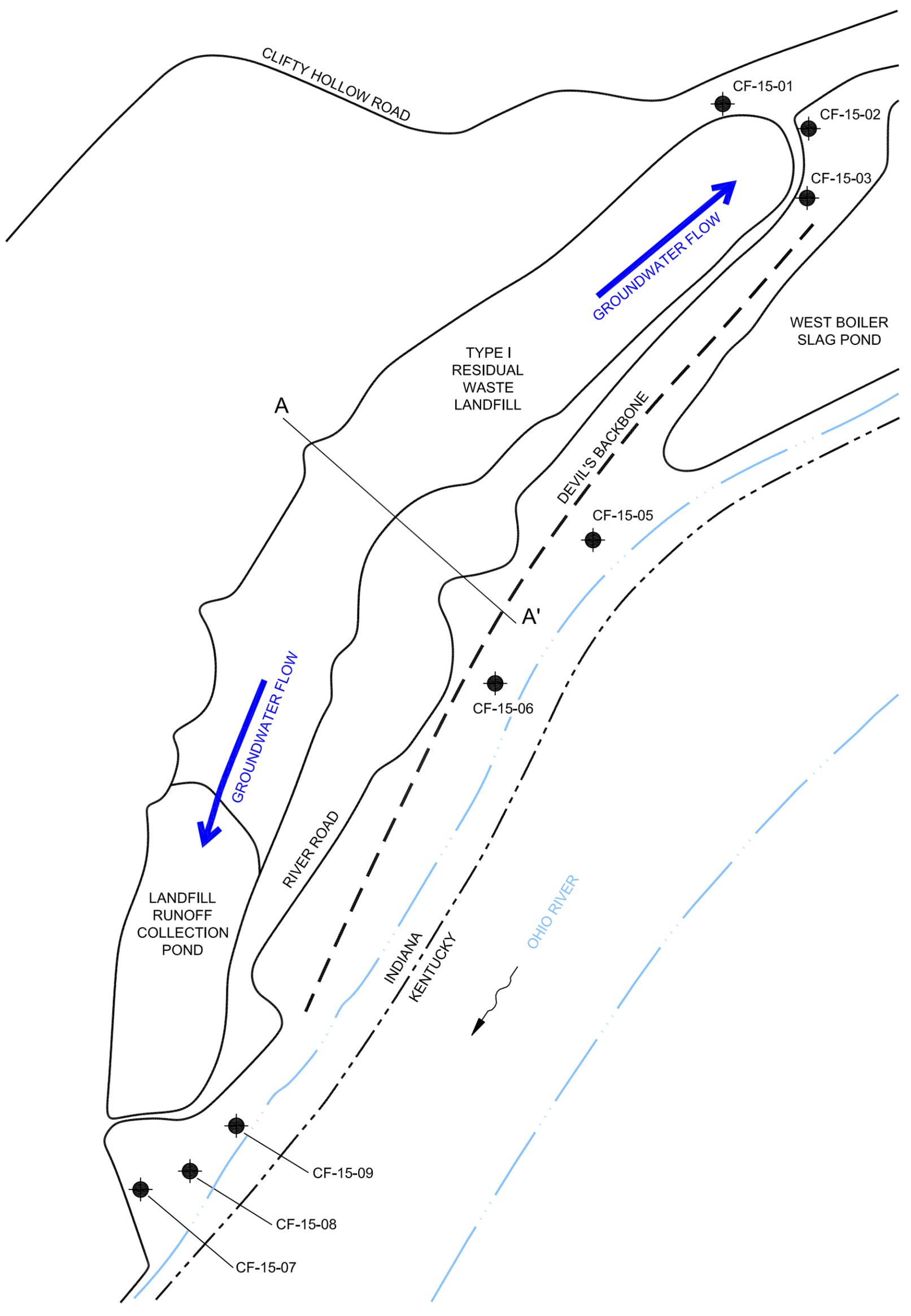
SOURCE: HYDROGEOLOGIC STUDY REPORT,
AGES, APRIL 2007.



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CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND GENERALIZED GEOLOGIC CROSS-SECTION	
DRAWING NAME	FIGURE 3
REV.	0



LEGEND:

● MONITORING WELL LOCATION

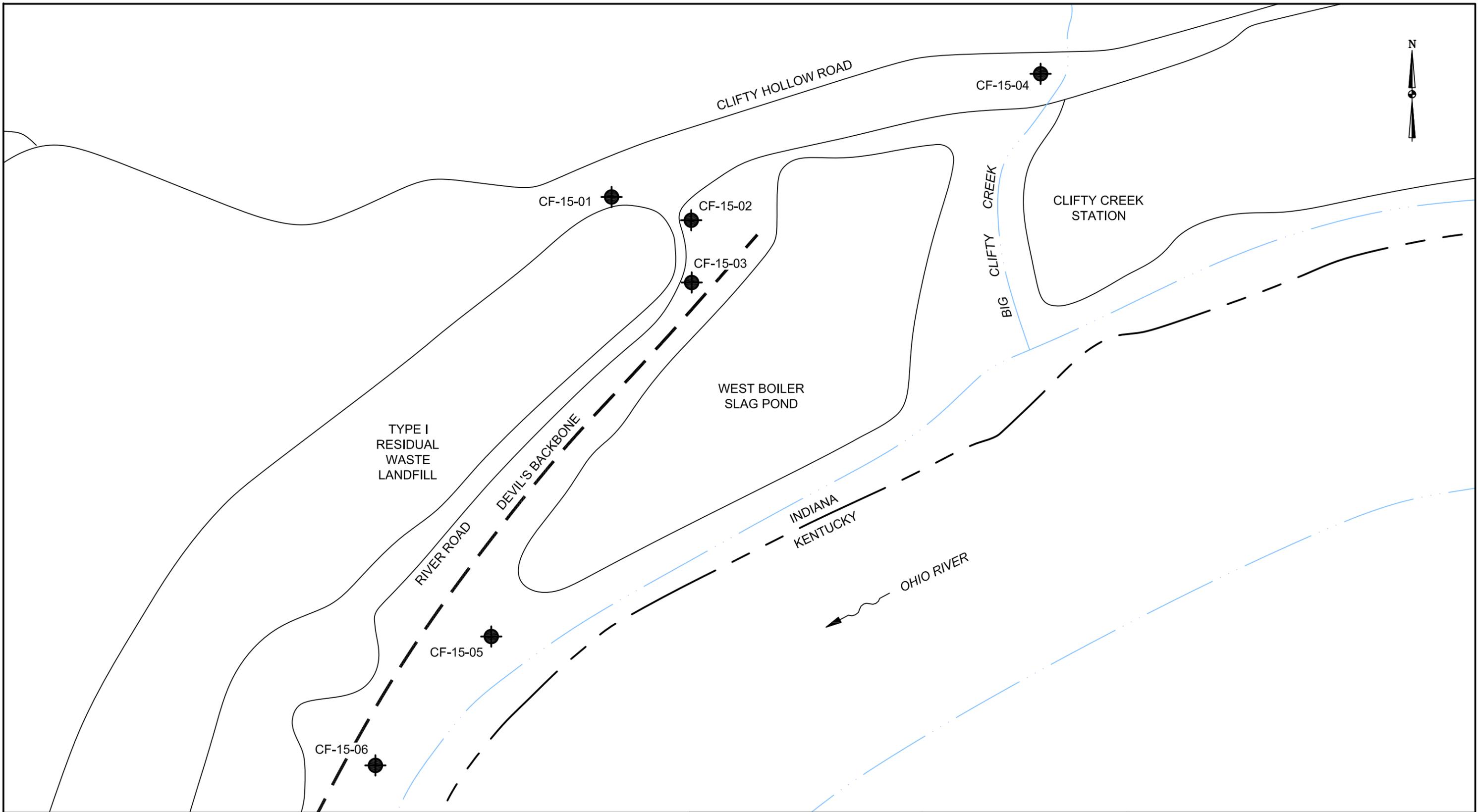
NOTE:

SEE FIGURE 4 FOR LOCATION OF BACKGROUND WELL CF-15-04.

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CHECKED BY	
JOB NO.	2015067-CLIF
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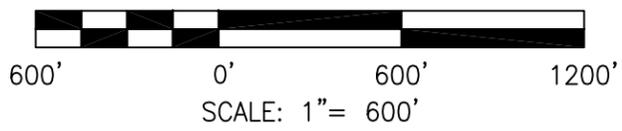
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CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND MONITORING WELL LOCATIONS AND GENERALIZED GROUNDWATER FLOW	
DRAWING NAME	FIGURE 4
REV.	0



LEGEND:

 MONITORING WELL LOCATION

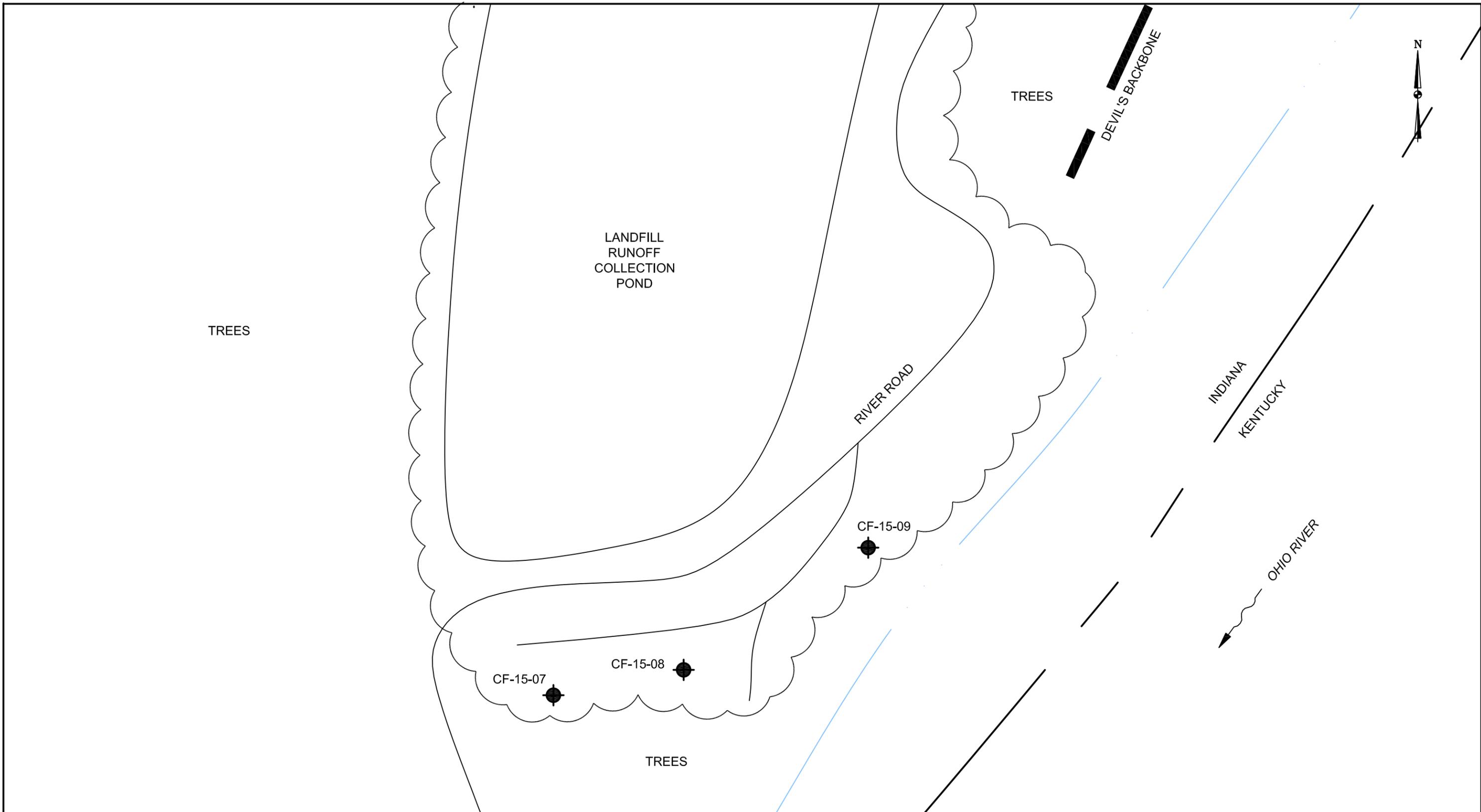


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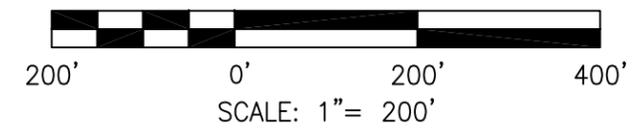
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INDIANA-KENTUCKY ELECTRIC CORPORATION	
CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND - NORTHEAST END MONITORING WELL LOCATIONS NORTHEAST END & BACKGROUND	
DRAWING NAME	FIGURE 7
REV.	0



LEGEND:

 MONITORING WELL LOCATION



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JOB NO.	2015067-CLI
DWG. FILE	IKEC_Clifty MW Install_MWs_b02-b03-b04.dwg
DRAWING SCALE	AS SHOWN



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INDIANA-KENTUCKY ELECTRIC CORPORATION	
CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND - SOUTHWEST END MONITORING WELL LOCATIONS LANDFILL - SOUTHWEST END	
DRAWING NAME	FIGURE 8
REV.	0

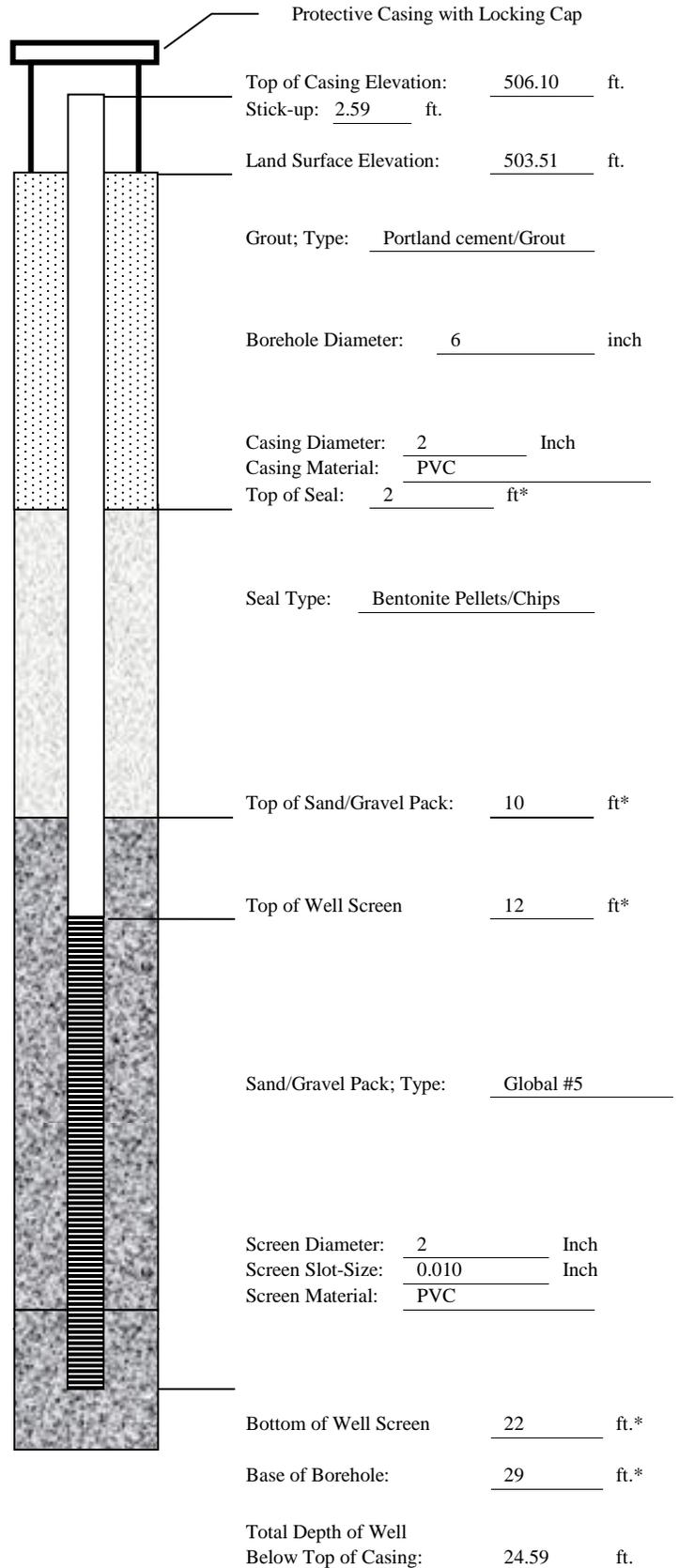
APPENDIX B

BORING & WELL LOGS

WELL CONSTRUCTION LOG

WELL NO. CF-15-01

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill Northeast End</u>
Installation Date(s):	<u>12/1/2015</u>
Drilling Method:	<u>Roto-Sonic</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/14/15</u>
Development Method:	<u>Submersible Pump, Peristaltic Pump, Bailer</u>
<u>Field parameters stabilized. Turbidity = 3.12 NTUs</u>	
Volume Purged:	<u>33</u>
Static Water-Level*:	<u>15.35'</u>
Top of Well Casing Elevation:	<u>506.10'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>450793.03</u>
Easting (X):	<u>566812.11</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Michael Gelles</u>



- CONSTRUCTION MATERIALS USED:**
- 6 Bags of Sand
 - 5 Bags/Buckets Bentonite Pellets
 - Bags Portland for Grout
 - Bags Concrete/Sakrete

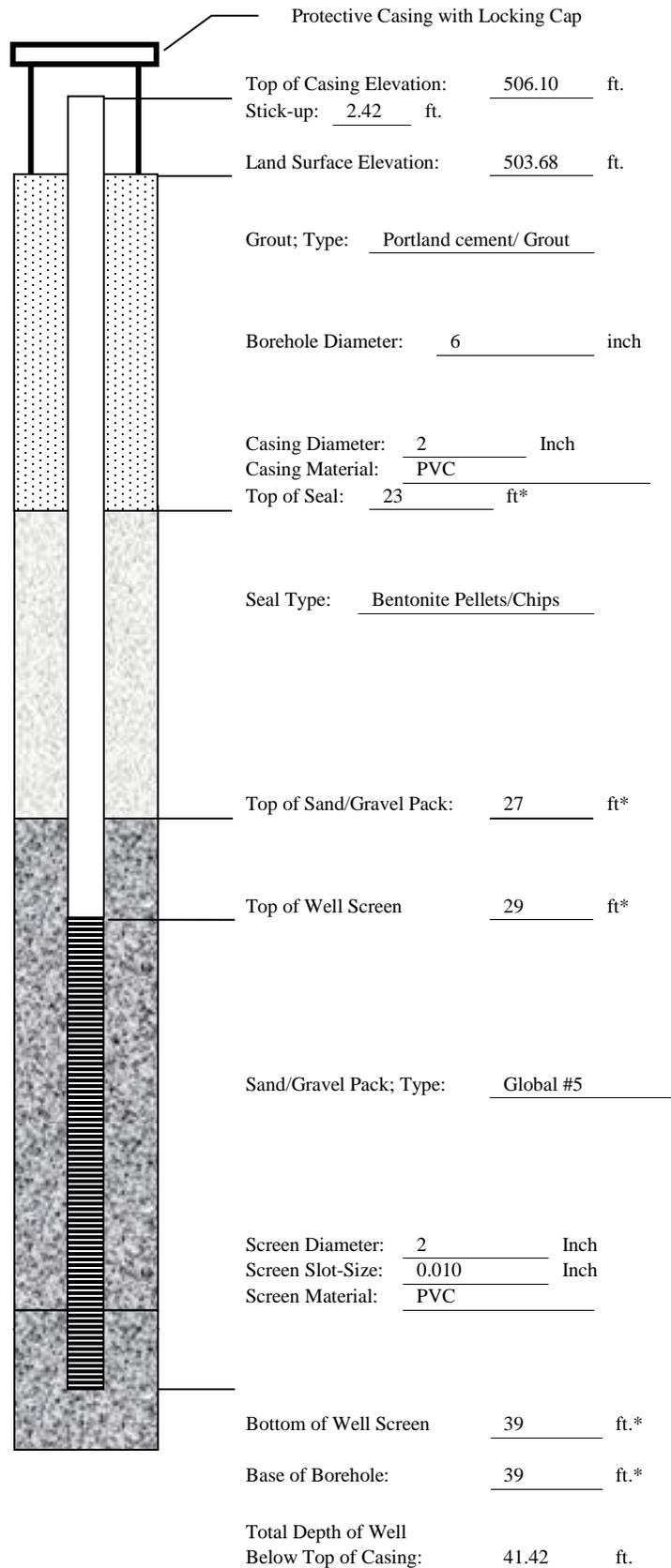
*Indicates Depth Below Land Surface

WELL CONSTRUCTION LOG

WELL NO. CF-15-02

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill Northeast End</u>
Installation Date(s):	<u>12/1/15</u>
Drilling Method:	<u>Roto-Sonic</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/7/15</u>
Development Method:	<u>Submersible Pump, Peristaltic Pump, Bailer</u>
<u>Field parameters stabilized. Turbidity = 3.69 NTUs</u>	
Volume Purged:	<u>114.5 gallons</u>
Static Water-Level*:	<u>15.89'</u>
Top of Well Casing Elevation:	<u>506.10'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>450449.42</u>
Easting (X):	<u>566908.15</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Michael Gelles</u>

CONSTRUCTION MATERIALS USED:	
<u>6</u>	<u>Bags of Sand</u>
<u>2</u>	<u>Bags/Buckets Bentonite Pellets</u>
<u>6</u>	<u>Bags Portland for Grout</u>
<u> </u>	<u>Bags Concrete/Sakrete</u>



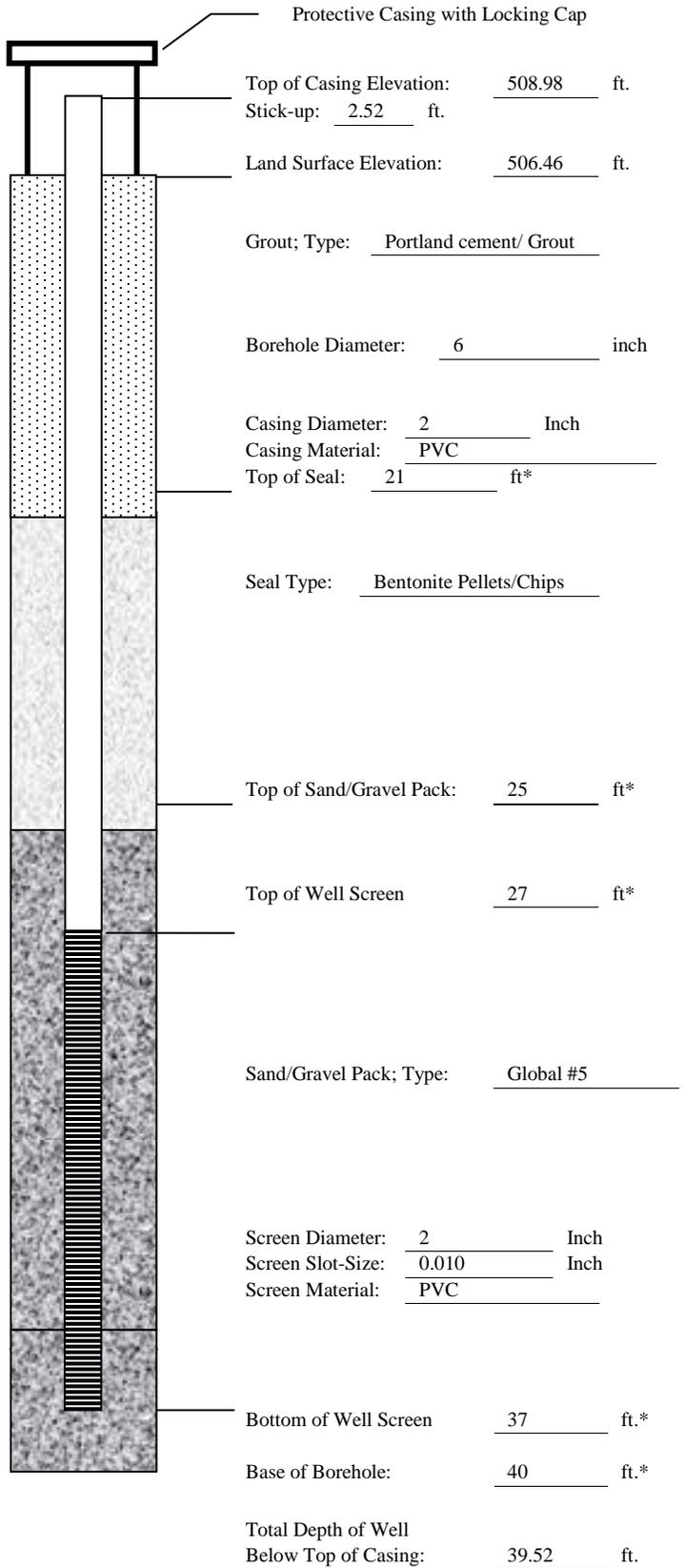
*Indicates Depth Below Land Surface

WELL CONSTRUCTION LOG

WELL NO. CF-15-03

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill Northeast End</u>
Installation Date(s):	<u>12/2/15-12/3/15</u>
Drilling Method:	<u>Roto-Sonic</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/15/15</u>
Development Method:	<u>Submersible Pump</u>
Field parameters stabilized:	<u>Turbidity = 3.99 NTUs</u>
Volume Purged:	<u>32.25 gallons</u>
Static Water-Level*:	<u>19.25'</u>
Top of Well Casing Elevation:	<u>508.98'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>450262.60</u>
Easting (X):	<u>566915.99</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Michael Gelles</u>

CONSTRUCTION MATERIALS USED:	
<u>6.5</u>	Bags of Sand
<u>2</u>	Bags/Buckets Bentonite Pellets
<u>3</u>	Bags Portland for Grout
<u> </u>	Bags Concrete/Sakrete



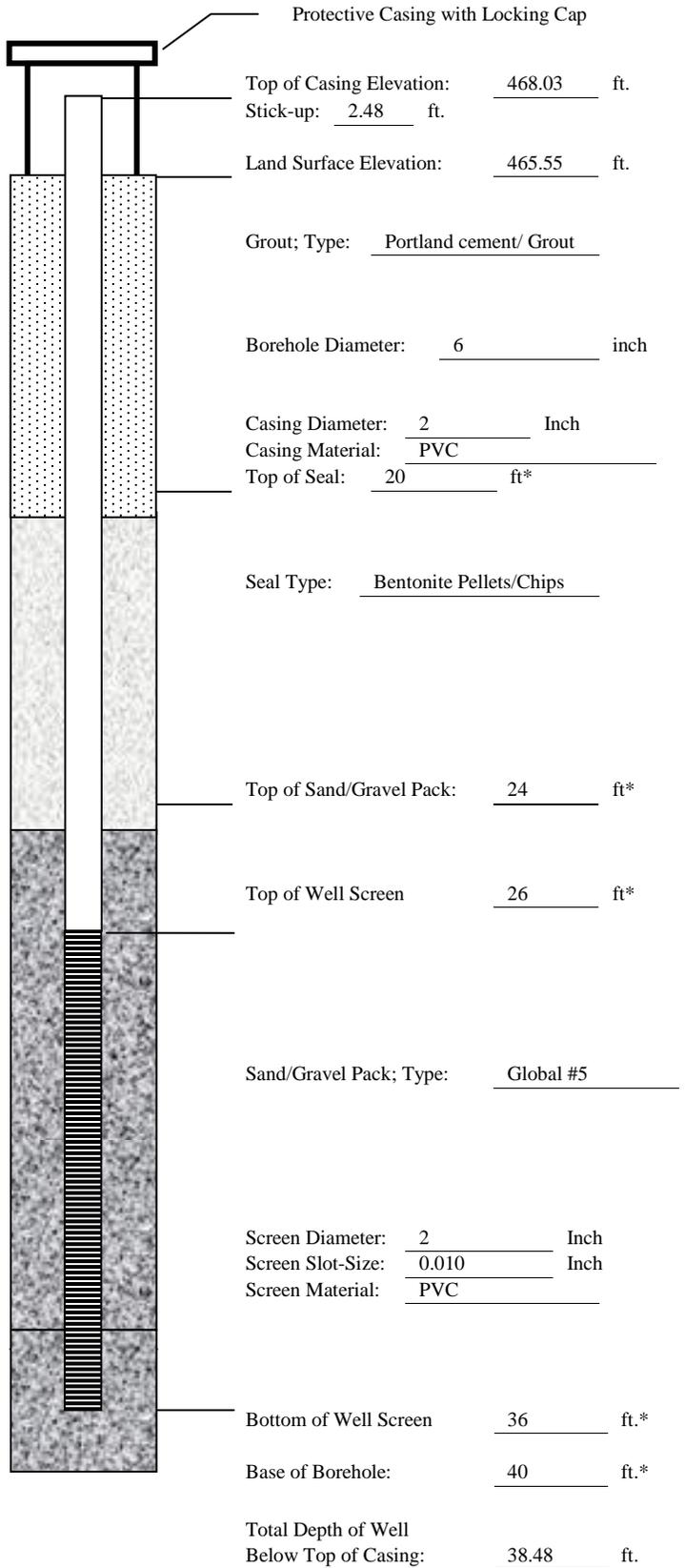
*Indicates Depth Below Land Surface

WELL CONSTRUCTION LOG

WELL NO. CF-15-04

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill Northeast End</u>
Installation Date(s):	<u>12/3/15</u>
Drilling Method:	<u>Roto-Sonic</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/9/15</u>
Development Method:	<u>Submersible Pump</u>
Field parameters stabilized:	<u>Turbidity = 0.91 NTUs</u>
Volume Purged:	<u>65 gallons</u>
Static Water-Level*:	<u>28.53'</u>
Top of Well Casing Elevation:	<u>468.03'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>451482.81</u>
Easting (X):	<u>569307.19</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Michael Gelles</u>

CONSTRUCTION MATERIALS USED:	
<u>6</u>	Bags of Sand
<u>2</u>	Bags/Buckets Bentonite Pellets
<u>6</u>	Bags Portland for Grout
<u> </u>	Bags Concrete/Sakrete



*Indicates Depth Below Land Surface

BORING NO. CF-15-05
SAMPLE/CORE LOG

Project Number: <u>2015067</u>	Log Page <u>1</u> of <u>1</u>		
Project Location: <u>Clifty Creek Plant Landfill South End</u>	Drilling Contractor: <u>Bowser Morner</u>		
Drilling Date(s): <u>11/29/15-11/30/15</u>	AGES Geologist: <u>Joe Webster</u>		
Drilling Method: <u>HSA</u>	Coring Device Size: <u>NA</u>	Hammer Wt. <u>160lb</u>	and Drop <u>2ft</u>
Sampling Method: <u>NA</u>	Borehole Diameter: <u>4.25"</u>	Drilling Fluid Used: <u>Water</u>	
Sampling Interval: <u>NA</u>	Borehole Depth: <u>27'</u>	Surface Elevation: <u>439.85' MSL</u>	
NOTES/COMMENTS: _____ _____			

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10		NA	Advance augers – no samples	N/A
10-12	2	2-2-2-2	Brown clay, little silt, very moist to wet	N/A
12-14	2	1-2-2-3	Brown clay, little silt, wet.	N/A
14-16	2	2-2-2-2	Brown clay, little silt, very moist to wet	N/A
16-18	2	2-3-2-2	Brown to olive gray clay, little silt, trace sand, very moist to wet	N/A
18-20	1.33	1-1-2-1	Olive gray clay, some silt, wet	N/A
20-22	2	2-2-3-2	Olive gray clay, some silt, wet	N/A
22-24	2	WH-WH-2-2	Gray clay, some silt, trace fine sand, moist to wet	N/A
24-26	2	1-1-2-2	Gray clay, some silt, trace fine sand, moist	N/A
26-27	0.1	10-50/1	Brown to gray weathered shale with limestone	N/A
				N/A

WELL CONSTRUCTION LOG

WELL NO. CF-15-05

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill South End</u>
Installation Date(s):	<u>11/29/15-12/1/15</u>
Drilling Method:	<u>Hollow Stem Auger</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/16/15</u>
Development Method:	<u>Peristaltic Pump, Bailer</u>
Field parameters stabilized:	<u>Turbidity = 4.28 NTUs</u>
Volume Purged:	<u>46 gallons</u>
Static Water-Level*:	<u>11.23'</u>
Top of Well Casing Elevation:	<u>442.58'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>447491.91</u>
Easting (X):	<u>565533.64</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Joe Webster</u>

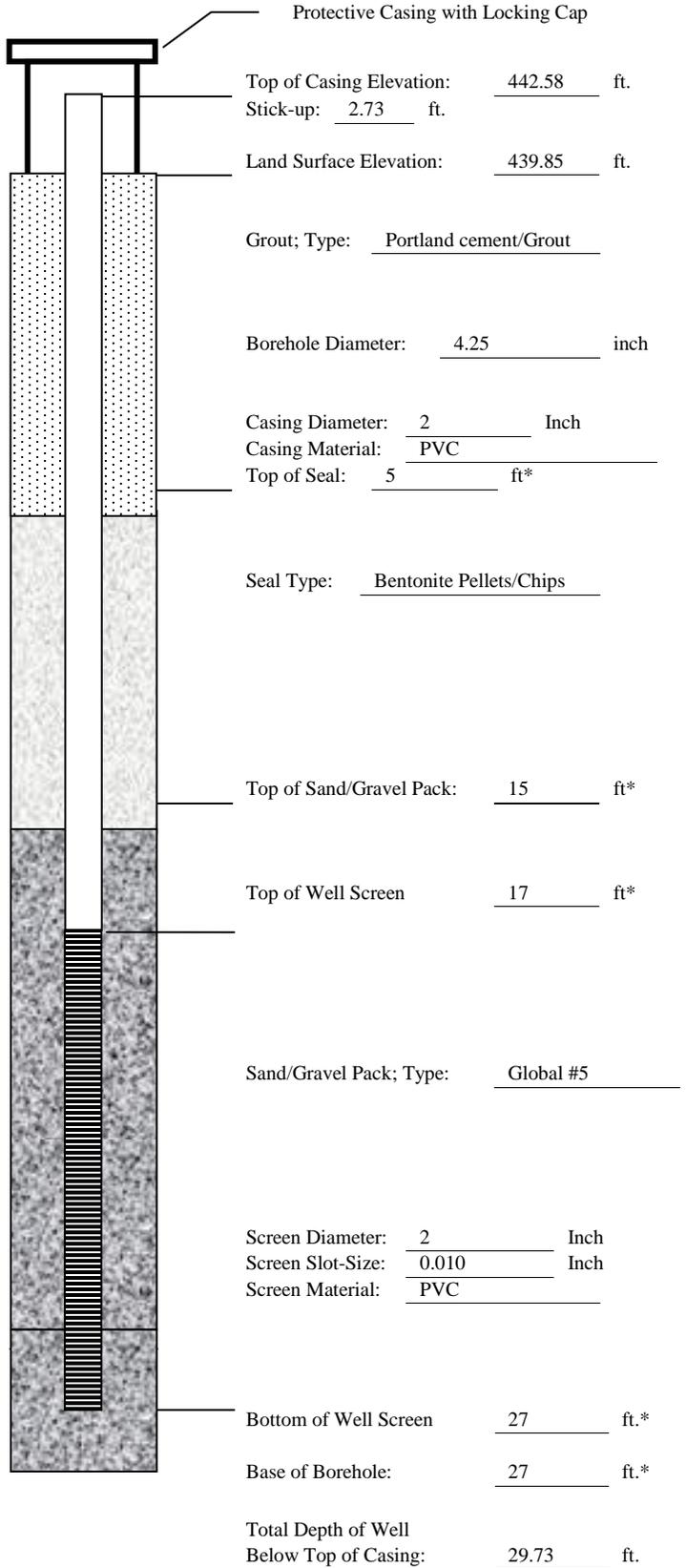
CONSTRUCTION MATERIALS USED:

_____ Bags of Sand

_____ Bags/Buckets Bentonite Pellets

_____ Bags Portland for Grout

_____ Bags Concrete/Sakrete



*Indicates Depth Below Land Surface

WELL CONSTRUCTION LOG

WELL NO. CF-15-06

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill South End</u>
Installation Date(s):	<u>11/29/15-11/30/15</u>
Drilling Method:	<u>Hollow Stem Auger</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/16/15</u>
Development Method:	<u>Peristaltic Pump, Bailer</u>
Field parameters stabilized:	<u>Turbidity = 5.59 NTUs</u>
Volume Purged:	<u>6.95 gallons</u>
Static Water-Level*:	<u>17.65'</u>
Top of Well Casing Elevation:	<u>440.40'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>447026.92</u>
Easting (X):	<u>565190.31</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Joe Webster</u>

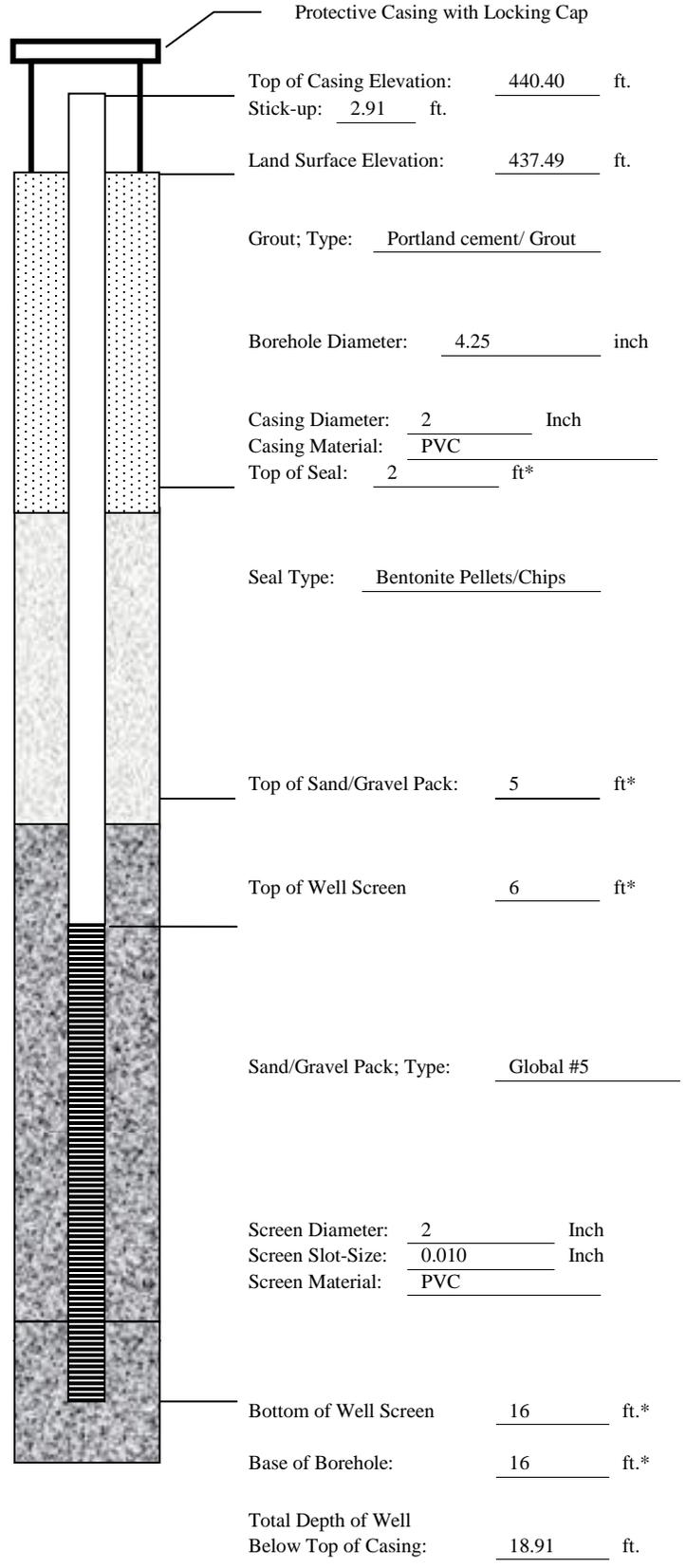
CONSTRUCTION MATERIALS USED:

_____ Bags of Sand

_____ Bags/Buckets Bentonite Pellets

_____ Bags Portland for Grout

_____ Bags Concrete/Sakrete



*Indicates Depth Below Land Surface

WELL CONSTRUCTION LOG

WELL NO. CF-15-07

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill South End</u>
Installation Date(s):	<u>11/23/15</u>
Drilling Method:	<u>Hollow Stem Auger</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/15/15</u>
Development Method:	<u>Peristaltic Pump, Bailer</u>
Field parameters stabilized:	<u>Turbidity = 4.42 NTUs</u>
Volume Purged:	<u>12.5 gallons</u>
Static Water-Level*:	<u>5.92'</u>
Top of Well Casing Elevation:	<u>441.11'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>443135.08</u>
Easting (X):	<u>562259.25</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Joe Webster</u>

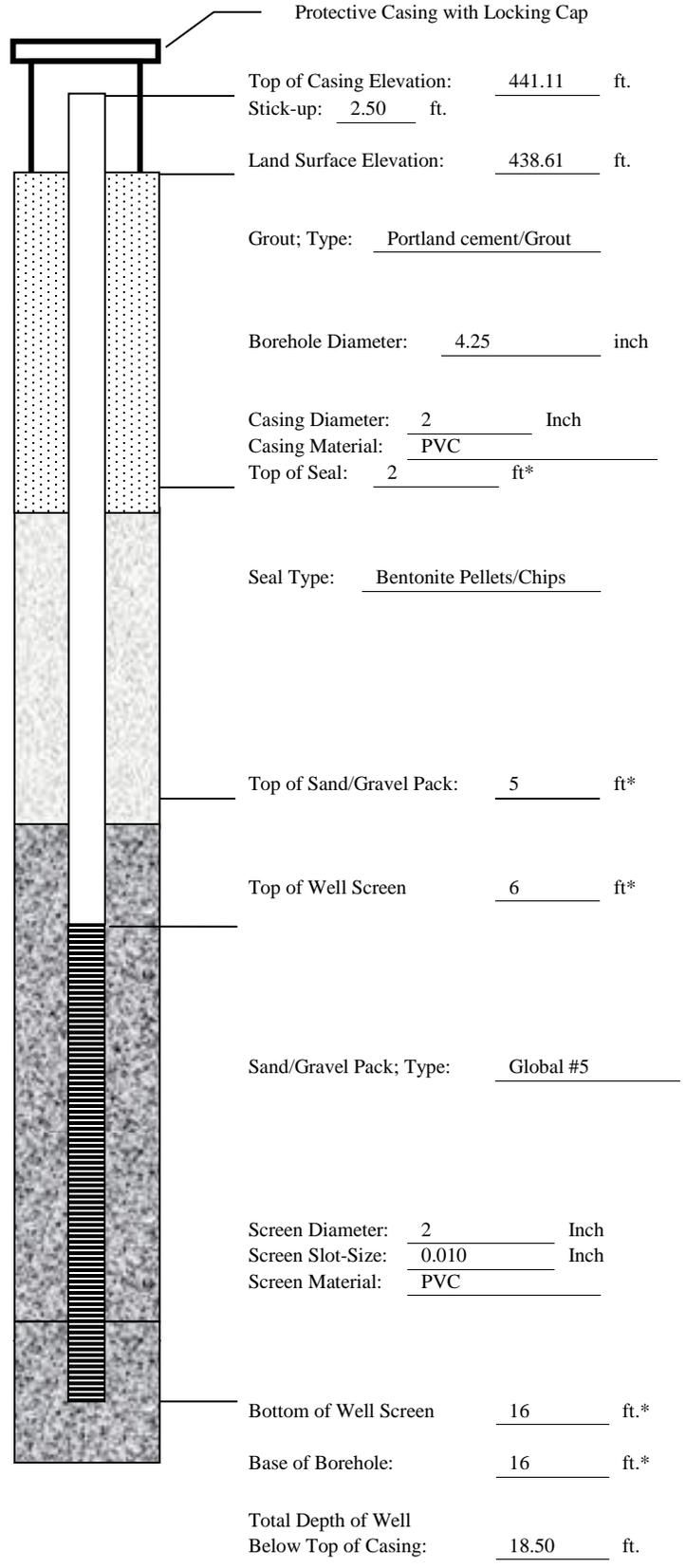
CONSTRUCTION MATERIALS USED:

_____ Bags of Sand

_____ Bags/Buckets Bentonite Pellets

_____ Bags Portland for Grout

_____ Bags Concrete/Sakrete



*Indicates Depth Below Land Surface

BORING NO. CF-15-08
SAMPLE/CORE LOG

Project Number: <u>2015067</u>	Log Page <u>1</u> of <u>1</u>
Project Location: <u>Clifty Creek Plant Landfill South End</u>	Drilling Contractor: <u>Bowser Morner</u>
Drilling Date(s): <u>11/17/15-11/19/15</u>	AGES Geologist: <u>Mike Gelles</u>
Drilling Method: <u>HSA</u>	Coring Device Size: <u>NA</u> Hammer Wt. <u>160lb</u> and Drop <u>2ft</u>
Sampling Method: <u>NA</u>	Borehole Diameter: <u>4.25"</u> Drilling Fluid Used: <u>Water</u>
Sampling Interval: <u>NA</u>	Borehole Depth: <u>40'</u> Surface Elevation: <u>460.33' MSL</u>
NOTES/COMMENTS: _____ _____	

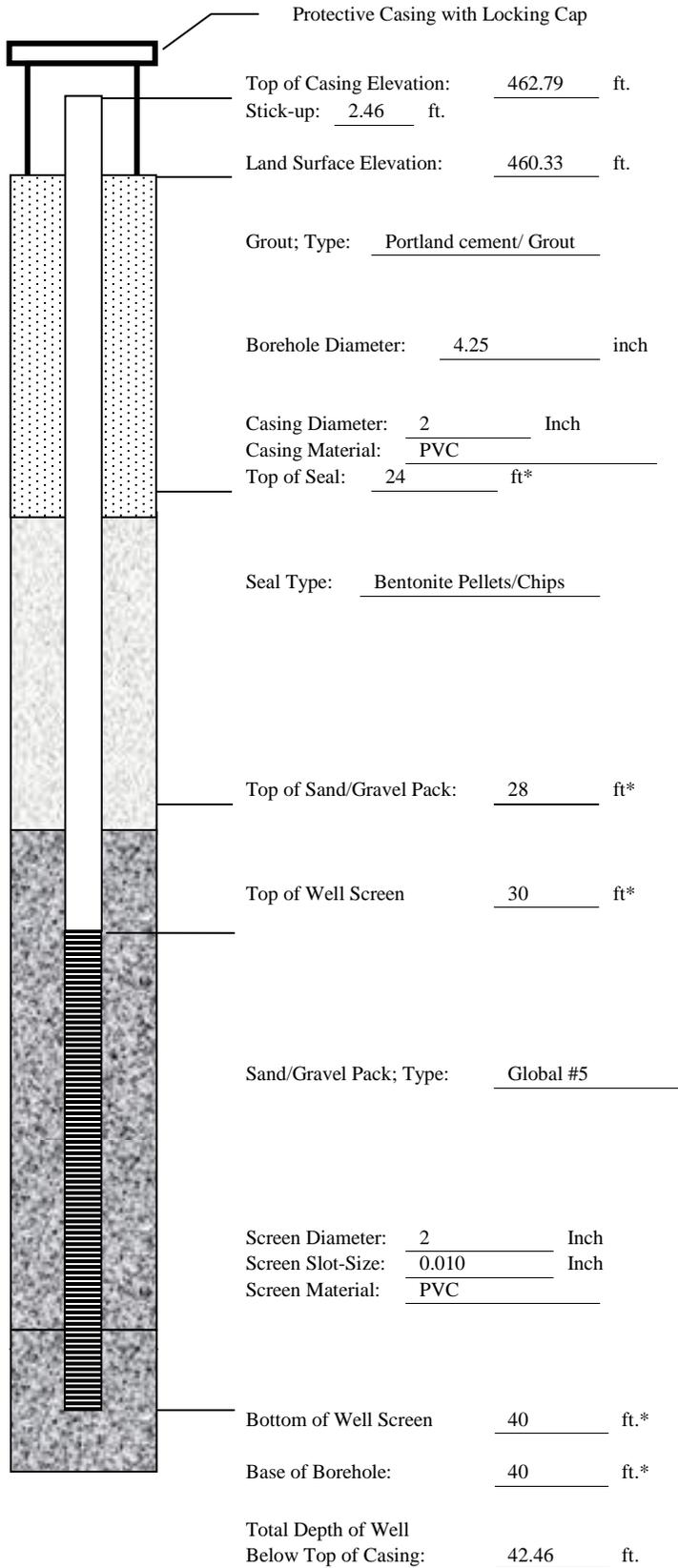
Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10		NA	Advance augers – no samples	N/A
10-12	2	3-6-6-7	Orange brown silty clay, fine sand, slightly plastic, moist	N/A
12-14	1.4	5-7-10-10	Light brown silt, loose, moist	N/A
14-16	1.6	4-8-12-10	Light brown silt, loose, moist	N/A
16-18	1.6	7-6-9-7	Light brown silt, loose, moist	N/A
18-20	1.6	3-6-4-4	18'-19' Light brown silt, loose, moist; 19'-20' Light brown silt, loose, wet	N/A
20-22	1.2	2-3-6-6	Light brown silt, trace clay, wet	N/A
22-24	0.1	2-3-3-3	Brown silt, clay, wet	N/A
24-26	2	2-4-6-7	Brown silt, clay, wet	N/A
26-28	2	3-5-5-5	Brown fine and medium sand, trace silt, trace clay, wet	N/A
28-30	2	3-5-9-12	Brown fine and medium sand, trace silt, trace clay, wet	N/A
30-32	1.2	1-2-2-2	Brown fine and medium sand, medium gravel, trace silt, trace clay, wet	N/A
32-34	2	4-5-5-9	Brown fine and medium sand, fine and medium gravel, trace silt, trace clay, wet	N/A
34-36	2	WH-3-6-8	Brown fine and medium sand, fine and medium gravel, trace silt, trace clay, wet	N/A
36-38	2	4-5-7-8	Brown fine and medium sand, fine and medium gravel, trace silt, trace clay, wet	N/A
38-40	2	3-5-5-11	38'-39.75' Brown fine and medium sand, fine and medium gravel, trace silt, trace clay, wet; 39.75'-40' gray fine and medium sand, silt, trace clay, wet	N/A

WELL CONSTRUCTION LOG

WELL NO. CF-15-08

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill South End</u>
Installation Date(s):	<u>11/17/15-11/19/15</u>
Drilling Method:	<u>Hollow stem Auger</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/8/15</u>
Development Method:	<u>Submersible Pump</u>
Field parameters stabilized:	<u>Turbidity = 2.16 NTUs</u>
Volume Purged:	<u>100 gallons</u>
Static Water-Level*:	<u>24.31'</u>
Top of Well Casing Elevation:	<u>462.79'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>443219.57</u>
Easting (X):	<u>562537.29</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Michael Gelles</u>

CONSTRUCTION MATERIALS USED:	
<u>4.5</u>	<u>Bags of Sand</u>
<u>0.5</u>	<u>Bags/Buckets Bentonite Pellets</u>
<u>3</u>	<u>Bags Portland for Grout</u>
<u> </u>	<u>Bags Concrete/Sakrete</u>



*Indicates Depth Below Land Surface

WELL CONSTRUCTION LOG

WELL NO. CF-15-09

Project Number:	<u>2015067</u>
Project Location:	<u>Clifty Creek Plant – Landfill South End</u>
Installation Date(s):	<u>11/24/15-11/25/15</u>
Drilling Method:	<u>Hollow Stem Auger</u>
Drilling Contractor:	<u>Bowser Morner</u>
Development Date(s):	<u>12/16/15</u>
Development Method:	<u>Peristaltic Pump, Bailer</u>
Field parameters stabilized:	<u>Turbidity = 3.21 NTUs</u>
Volume Purged:	<u>6 gallons</u>
Static Water-Level*:	<u>12.18'</u>
Top of Well Casing Elevation:	<u>459.45'</u>
Well Purpose:	<u>Groundwater Monitoring</u>
Northing (Y):	<u>443445.96</u>
Easting (X):	<u>562871.69</u>
Comments/Notes:	<u>2 inch PVC riser and screen</u> <u>5 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh.</u>
Inspector:	<u>Joe Webster</u>

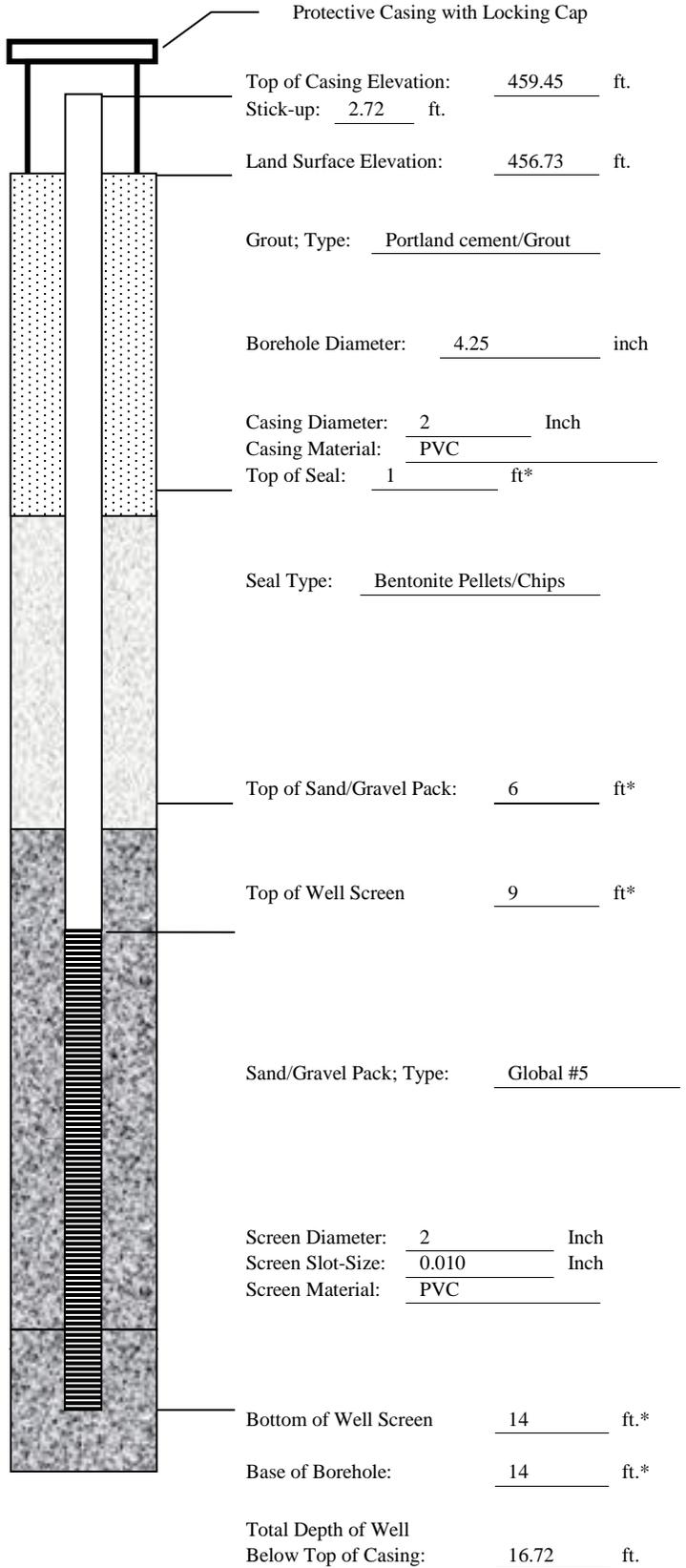
CONSTRUCTION MATERIALS USED:

_____ Bags of Sand

_____ Bags/Buckets Bentonite Pellets

_____ Bags Portland for Grout

_____ Bags Concrete/Sakrete



*Indicates Depth Below Land Surface

APPENDIX C

WELL DEVELOPMENT DATA

TABLE C-1
SUMMARY OF WELL DEVELOPMENT DATA
KYGER CREEK PLANT
GALLIA COUNTY, OHIO

Well/ Piezometer	Dates	Method	Volume (gal)	Final Turbidity (NTU)
Type I Residual Waste Landfill and Landfill Runoff Collection Pond				
CF-15-01	12/08/2015 - 12/14/2015	Pump/Bail	33	3.12
CF-15-02	12/04/2015 - 12/7/2015	Pump/Bail	115	3.69
CF-15-03	12/04/2015 - 12/15/2015	Pump/Bail	32	3.99
CF-15-04	12/9/2015	Pump	65	0.91
CF-15-05	12/09/2015 - 12/16/2015	Pump	46	4.28
CF-15-06	12/09/2015 - 12/18/2016	Pump/Bail	21	9.59
CF-15-07	12/08/2015 - 12/15/2015	Pump/Bail	13	4.42
CF-15-08	12/8/2015	Pump	100	2.16
CF-15-09	12/08/2015 - 12/16/2015	Pump/Bail	6	3.21
West Boiler Slag Pond				
WBSP-15-01	12/03/2015 - 12/17/2015	Pump/Bail	23	70.8
WBSP-15-02	12/03/2015 - 12/15/2015	Pump	31.5	3.48
WBSP-15-03	12/09/2015 - 12/15/2015	Pump/Bail	15	2.42
WBSP-15-04	12/02/2015 - 12/08/2015	Pump	110	1.37
WBSP-15-05	12/02/2015 - 12/03/2015	Pump	130	1.87
WBSP-15-06	12/03/2015 - 12/09/2015	Pump	100	3.44
WBSP-15-07	12/02/2015 - 12/16/2015	Pump/Bail	36	2.86
WBSP-15-08	12/02/2015 - 12/16/2015	Pump	90	4.96
WBSP-15-09	1/08/2016 - 1/19/2016	Pump	59	3.57
WBSP-15-10	1/07/2016 - 1/20/2016	Pump	33	3.59

APPENDIX D

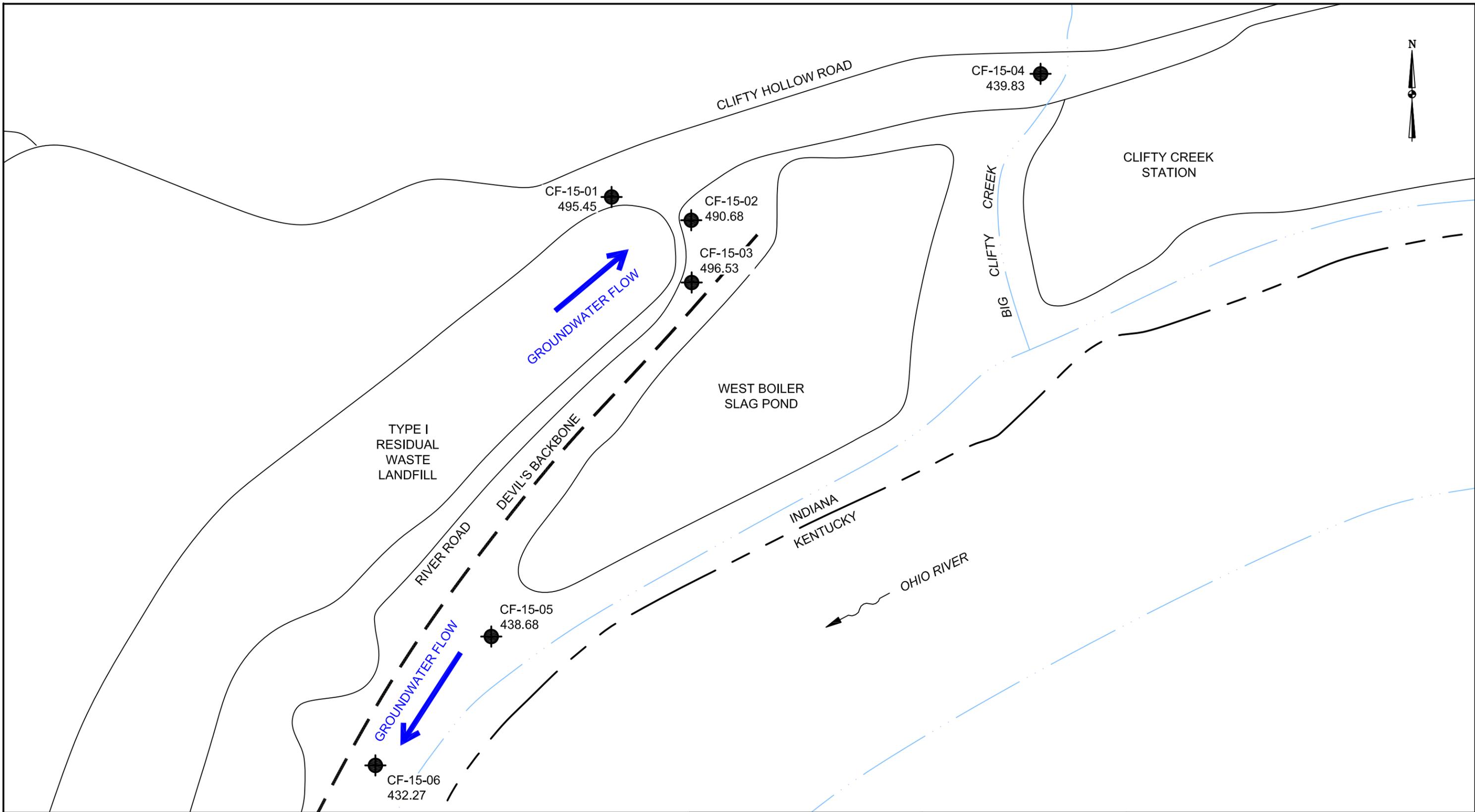
**GROUNDWATER LEVELS
January 2016 through May 2016**

**TABLE D-1
CLIFTY CREEK CREEK PLANT
SUMMARY OF GROUNDWATER ELEVATION DATA
JANUARY 2016 - MAY 2016**

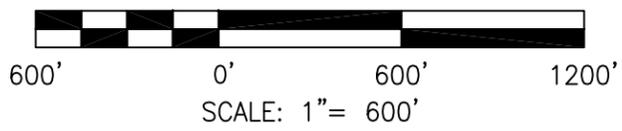
Monitoring Well Designation	Jan-16 Groundwater Elevation (ft)	Mar-16 Groundwater Elevation (ft)	May-16 Groundwater Elevation (ft)
LANDFILL AND LANDFILL RUNOFF COLLECTION POND			
CF-15-01	495.45	496.16	496.35
CF-15-02	490.68	490.95	490.97
CF-15-03	496.53	496.64	496.38
CF-15-04	439.83	441.19	441.27
CF-15-05	438.68	439.86	436.25
CF-15-06	432.27	437.12	429.22
CF-15-07	436.61	438.08	437.48
CF-15-08	439.48	440.54	440.88
CF-15-09	450.77	451.58	450.69
WEST BOILER SLAG POND			
WBSP-15-01	451.72	453.01	453.27
WBSP-15-02	468.31	472.52	471.52
WBSP-15-03	477.03	477.11	477.62
WBSP-15-04	429.22	436.25	424.96
WBSP-15-05	428.95	436.12	424.84
WBSP-15-06	428.82	436.06	424.77
WBSP-15-07	429.72	430.41	430.88
WBSP-15-08	434.03	434.62	434.81
WBSP-15-09	432.17	430.39	432.21
WBSP-15-10	431.41	433.28	432.58

APPENDIX E

GROUNDWATER CONTOUR MAPS
January 2016 through May 2016



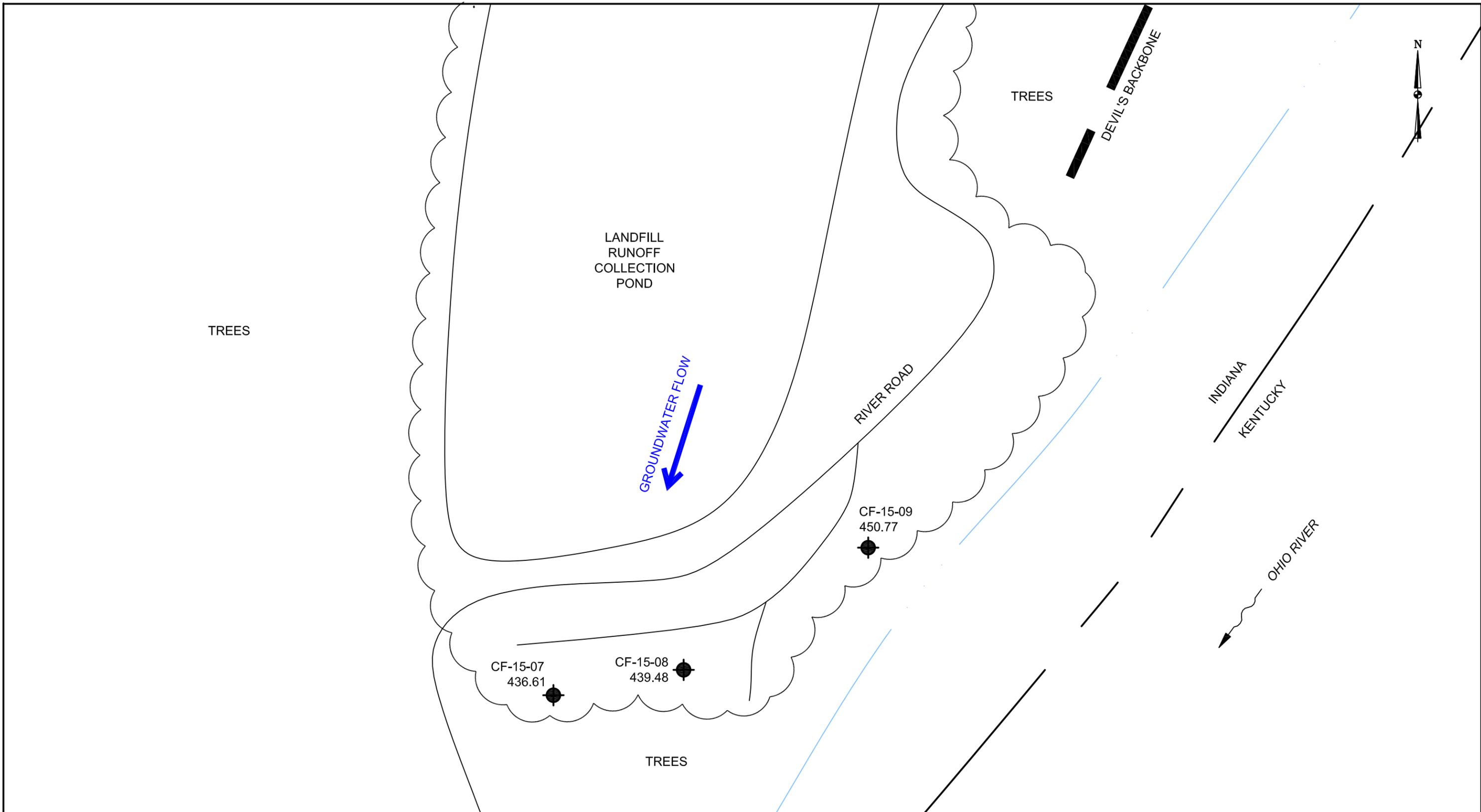
LEGEND:
 MONITORING WELL LOCATION
 GROUNDWATER FLOW DIRECTION



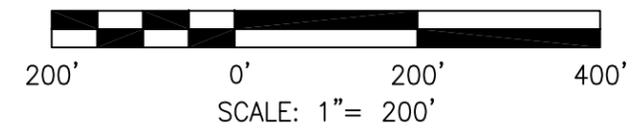
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JOB NO.	2015067-CLI
DWG. FILE	IKEC_Clifty MW Install_Appx E_Jan16 b08.dwg
DRAWING SCALE	AS SHOWN

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INDIANA-KENTUCKY ELECTRIC CORPORATION	
CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND NORTHEAST END & BACKGROUND GROUNDWATER LEVELS & FLOW DIRECTION-JANUARY 2016	
DRAWING NAME	FIGURE E-1
REV.	0



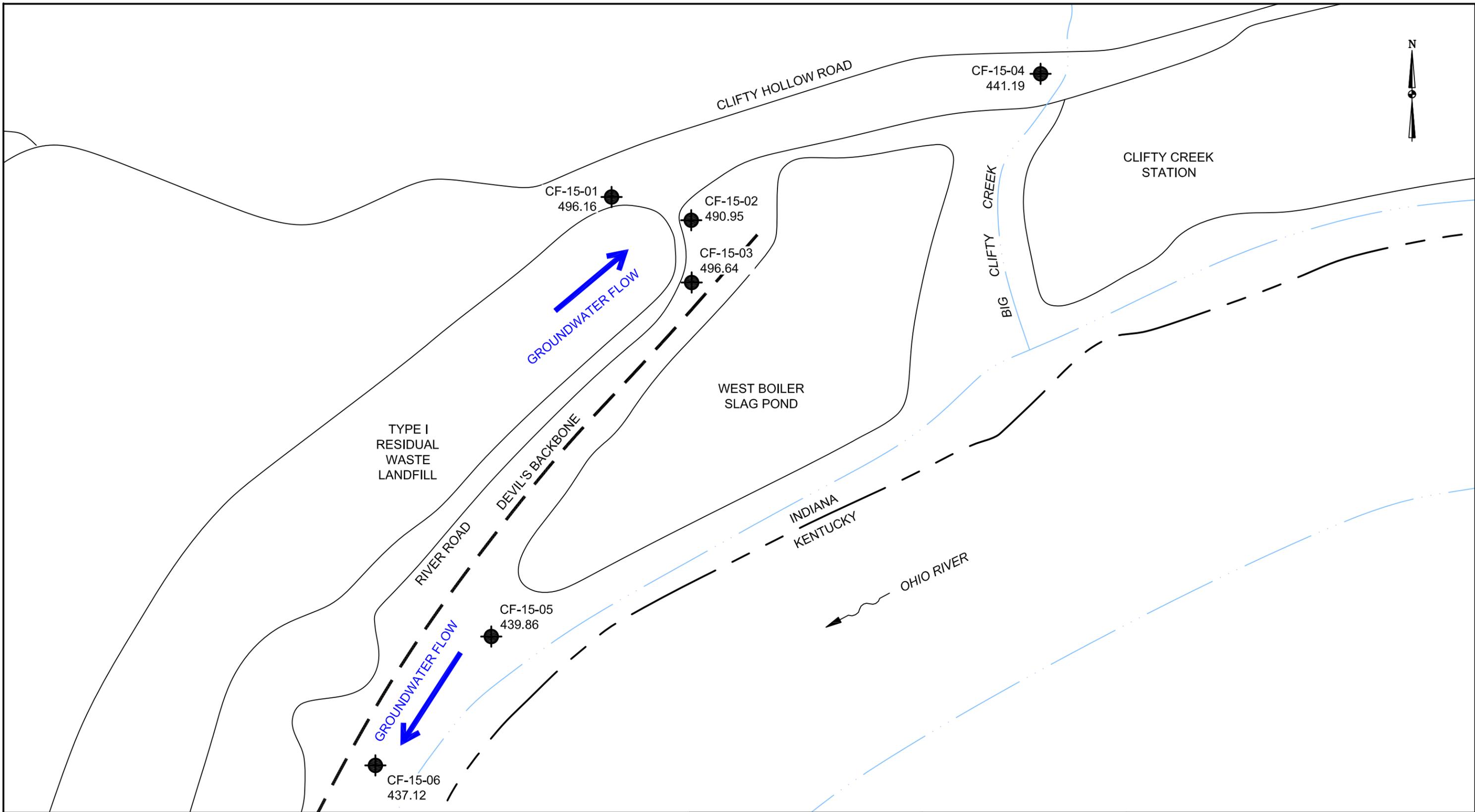
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 GROUNDWATER FLOW DIRECTION



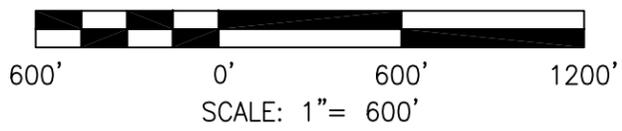
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JOB NO.	2015067-CLI
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DRAWING SCALE	AS SHOWN

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CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND SOUTHWEST END	
GROUNDWATER LEVELS & FLOW DIRECTION-JANUARY 2016	
DRAWING NAME	FIGURE E-2
REV.	0



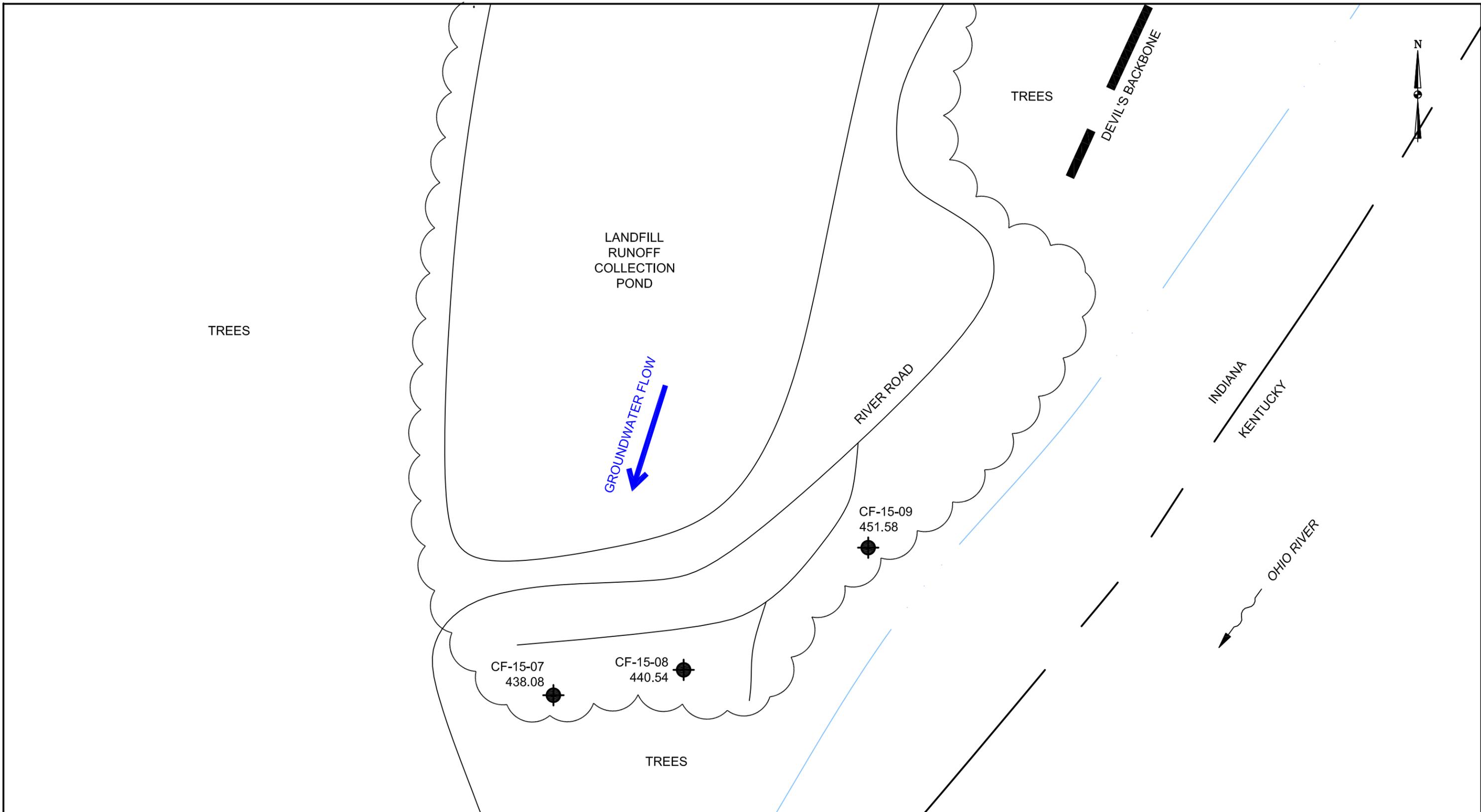
LEGEND:
 MONITORING WELL LOCATION
 GROUNDWATER FLOW DIRECTION



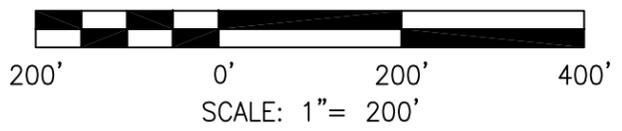
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JOB NO.	2015067-CLI
DWG. FILE	IKEC_Clifty MW Install_Appx E_Mar16 b09.dwg
DRAWING SCALE	AS SHOWN

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CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND NORTHEAST END & BACKGROUND GROUNDWATER LEVELS & FLOW DIRECTION-MARCH 2016	
DRAWING NAME	FIGURE E-4
REV.	0



LEGEND:
 MONITORING WELL LOCATION
 GROUNDWATER FLOW DIRECTION

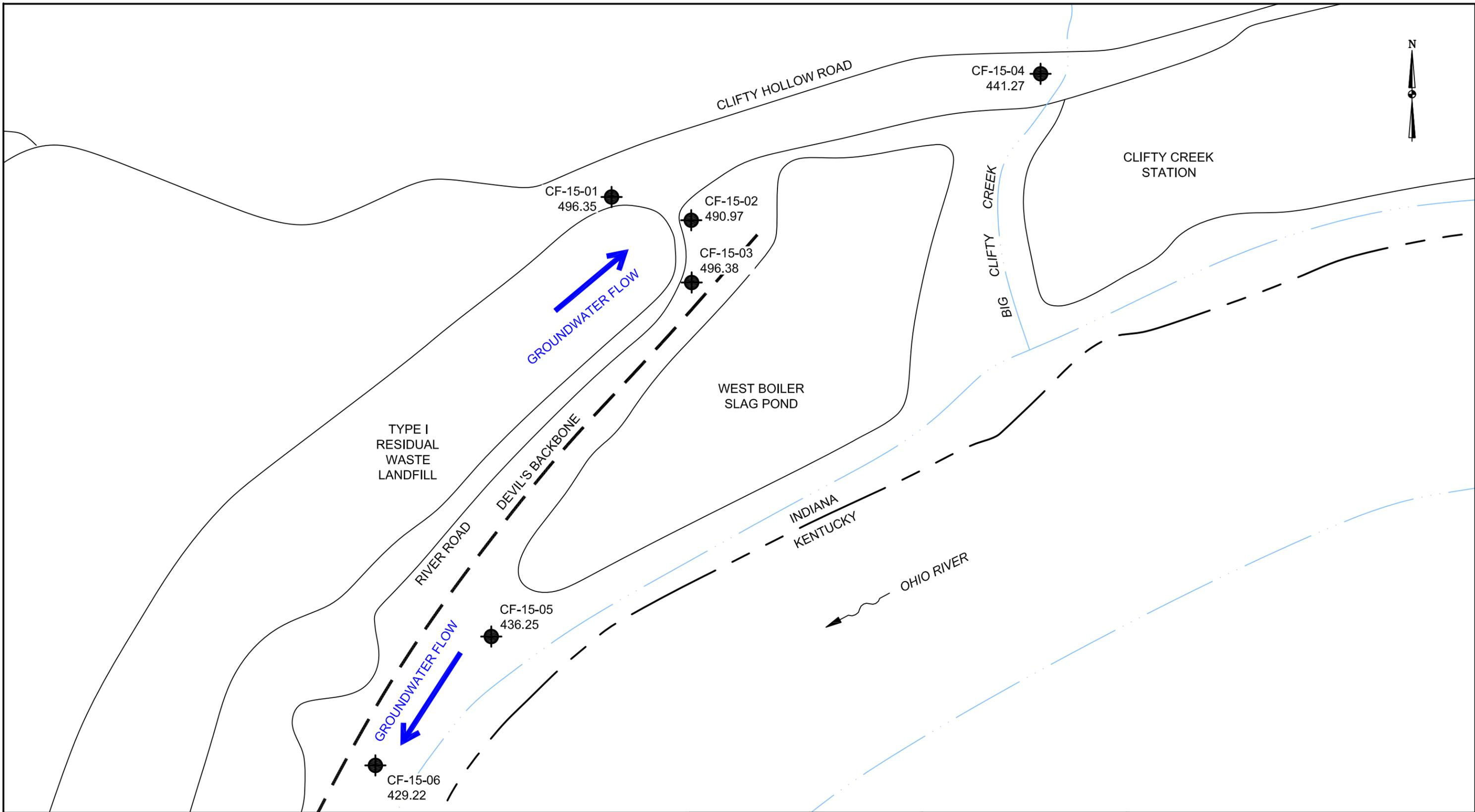


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DRAWING SCALE	AS SHOWN



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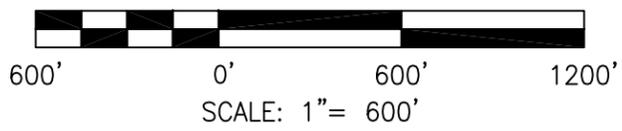
INDIANA-KENTUCKY ELECTRIC CORPORATION	
CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND SOUTHWEST END	
GROUNDWATER LEVELS & FLOW DIRECTION-MARCH 2016	
DRAWING NAME	FIGURE E-5
REV.	0



LEGEND:

● MONITORING WELL LOCATION

← GROUNDWATER FLOW DIRECTION

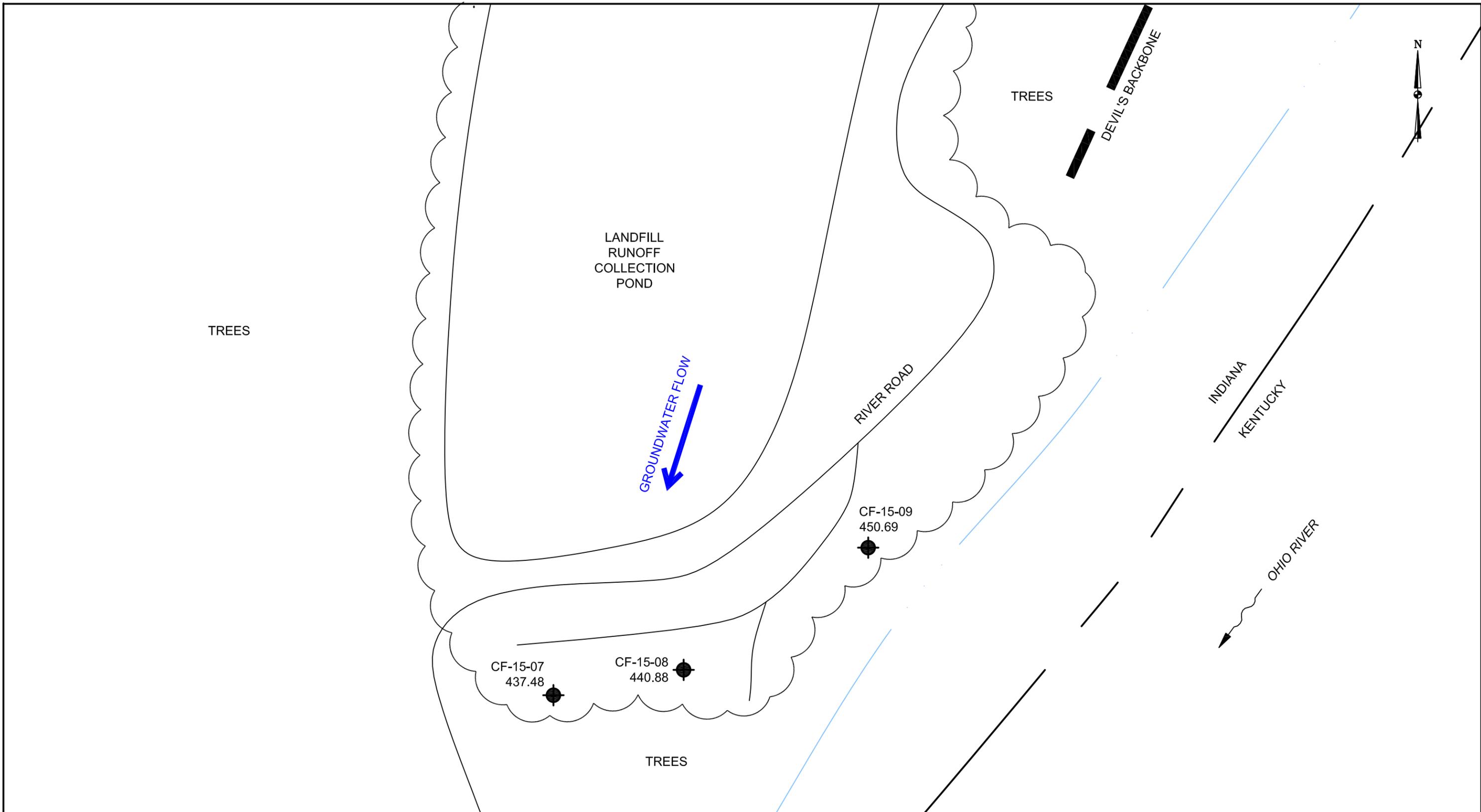


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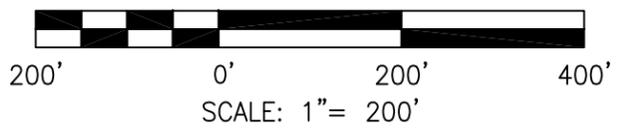
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CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND NORTHEAST END & BACKGROUND GROUNDWATER LEVELS & FLOW DIRECTION-MAY 2016	
DRAWING NAME	FIGURE E-7
REV.	0



LEGEND:
 MONITORING WELL LOCATION
 GROUNDWATER FLOW DIRECTION



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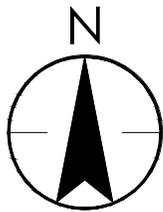


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INDIANA-KENTUCKY ELECTRIC CORPORATION	
CLIFTY CREEK STATION MADISON, INDIANA TYPE I RESIDUAL WASTE LANDFILL AND LANDFILL RUNOFF COLLECTION POND SOUTHWEST END GROUNDWATER LEVELS & FLOW DIRECTION-MAY 2016	
DRAWING NAME	FIGURE E-8
REV.	0

Stantec (2021)

BORINGS AND CPT LOCATED AT THE LANDFILL RUNOFF COLLECTION POND



N 446,000

E 562,000

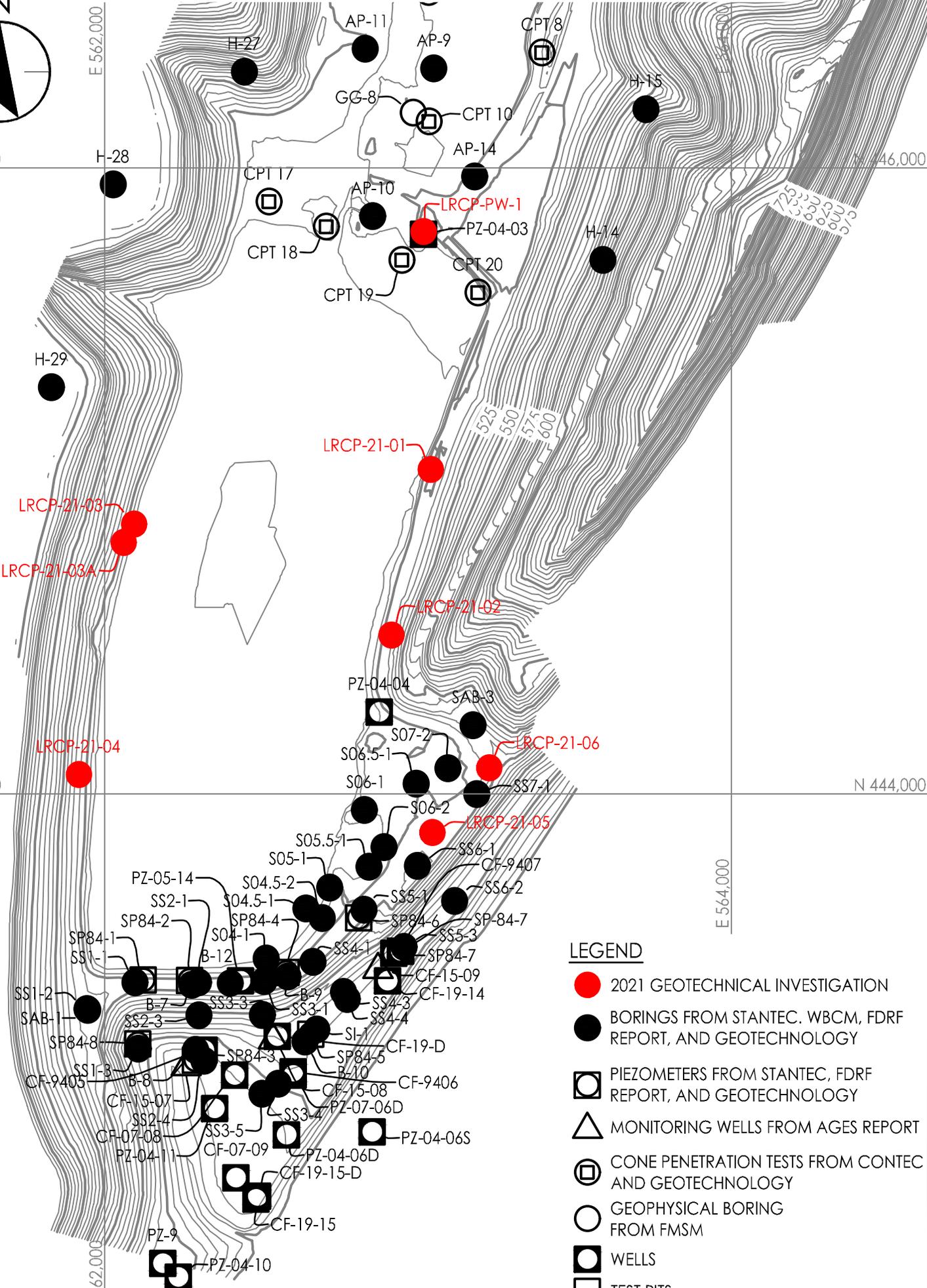
E 564,000

N 446,000

N 444,000

N 444,000

E 564,000



LEGEND

- 2021 GEOTECHNICAL INVESTIGATION
- BORINGS FROM STANTEC, WBCM, FDRF REPORT, AND GEOTECHNOLOGY
- ◻ (with circle) PIEZOMETERS FROM STANTEC, FDRF REPORT, AND GEOTECHNOLOGY
- △ MONITORING WELLS FROM AGES REPORT
- ◻ (with circle) CONE PENETRATION TESTS FROM CONTEC AND GEOTECHNOLOGY
- GEOPHYSICAL BORING FROM FMSM
- (with circle) WELLS
- ◻ (with circle) TEST PITS

Client Borehole Identification <u>LRCP-21-01</u>		Stantec Boring No. LRCP-21-01	
Client <u>IKEC</u>	Boring Location <u>445036.29 N; 563040.013 E</u>		
Project Number <u>175539026</u>	Surface Elevation <u>499.5 ft</u>	Elevation Datum <u>NAVD88</u>	
Project Name <u>Clifty Creek WBSP and LRCP Closure</u>	Date Started <u>3/15/21</u>	Completed <u>3/15/21</u>	
Project Location <u>Clifty Creek Power Plant, Madison, IN</u>	Depth to Water <u>10.0 ft</u>	Date/Time <u>3/15/21</u>	
Logged by <u>B. Herries</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Drilling Contractor <u>Stantec Consulting Services Inc.</u>	Drill Rig Type and ID <u>CME 55 Track Rig #711</u>		
Overburden Drilling and Sampling Tools (Type and Size) <u>4.25" HSA, 2" Split Spoon w/o liners, 3" Shelby Tubes</u>			
Rock Drilling and Sampling Tools (Type and Size) <u>N/A</u>			
Sampler Hammer Type <u>Automatic</u>	Weight <u>140 lb</u>	Drop <u>30 in</u>	Efficiency <u>88 % (Avg.)</u>
Borehole Azimuth <u>N/A (Vertical)</u>	Borehole Inclination (from Vertical) <u>Vertical</u>		

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
499.5	0.0	Top of Hole							
		SAND WITH GRAVEL (SW), CCR, dark brown with black, moist, loose to medium dense, with coal chunks		SPT-1	0.0 - 1.5	1.3	4-8-10	--	
				SPT-2	5.0 - 6.5	1.5	4-3-3	--	
489.5	10.0	SILTY CLAY (CL-ML), dark brownish gray, wet, very soft		SPT-3	10.0 - 11.5	1.5	WH-1-1	--	
484.5	15.0			SPT-4	15.0 - 16.5	1.5	2-2-3	--	
		FAT CLAY (CH), orange brown mottled with tan, moist, medium stiff to very stiff, medium to high plasticity							

TVA RO BORING LOG - CLIFTY - BORINGS_LOGS_GPJ_FMSM-GRAPHIC LOG.GDT 4/12/21

Client Borehole Identification <u>LRCP-21-01</u>				Stantec Boring No. LRCP-21-01					
Client <u>IKEC</u>				Boring Location <u>445036.29 N; 563040.013 E</u>					
Project Number <u>175539026</u>				Surface Elevation <u>499.5 ft</u>		Elevation Datum <u>NAVD88</u>			
Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
		FAT CLAY (CH), orange brown mottled with tan, moist, medium stiff to very stiff, medium to high plasticity <i>(Continued)</i>		SPT-5	20.0 - 21.5	1.4	6-7-8	--	
473.6	25.9	-at 25.0', wet		SPT-6	25.0 - 26.3	1.2	11-11-50/4"	--	
473.2	26.3	SHALE, gray, moist to wet, friable							
		Auger Refusal / Bottom of Hole							
		Borehole was backfilled with a mixture of cement-bentonite grout from the bottom of hole to the ground surface using a tremie pipe.							

TVA RO BORING LOG - CLIFTY - BORINGS_LOSS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-02</u>		Stantec Boring No. LRCP-21-02	
Client <u>IKEC</u>	Boring Location <u>444507.211 N; 562914.551 E</u>		
Project Number <u>175539026</u>	Surface Elevation <u>501.6 ft</u>	Elevation Datum <u>NAVD88</u>	
Project Name <u>Clifty Creek WBSP and LRCP Closure</u>	Date Started <u>3/14/21</u>	Completed <u>3/14/21</u>	
Project Location <u>Clifty Creek Power Plant, Madison, IN</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Logged by <u>B. Herries</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Drilling Contractor <u>Stantec Consulting Services Inc.</u>	Drill Rig Type and ID <u>CME 55 Track Rig #711</u>		
Overburden Drilling and Sampling Tools (Type and Size) <u>4.25" HSA, 2" Split Spoon w/o liners, 3" Shelby Tubes</u>			
Rock Drilling and Sampling Tools (Type and Size) <u>N/A</u>			
Sampler Hammer Type <u>Automatic</u>	Weight <u>140 lb</u>	Drop <u>30 in</u>	Efficiency <u>88 % (Avg.)</u>
Borehole Azimuth <u>N/A (Vertical)</u>	Borehole Inclination (from Vertical) <u>Vertical</u>		

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core						
501.6	0.0	Top of Hole							
501.3	0.3	TOPSOIL							
		SILTY SAND (SM), dark brown with black, moist, medium dense, with coal pieces of about 3/16" to 3/4", possible CCR -clay from 1.3' to 1.5'		SPT-1	0.0 - 1.5	1.5	2-6-7	--	
497.1	4.5	GRAVEL (GP), white to gray, dry, dense, medium coarse to coarse		SPT-2	5.0 - 6.5	1.0	30-12-20	--	
495.1	6.5	LEAN CLAY (CL), light brown and tan, moist, very stiff, some gravel		SPT-3	7.5 - 9.0	1.0	8-15-22	--	
		-at 8.0', cobbles for 6"		SPT-4	10.0 - 11.5	1.4	5-11-16	--	

TVA RO BORING LOG - CLIFTY BORINGS_LOGS.GPJ - FMSM+GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-02</u>				Stantec Boring No. LRCP-21-02					
Client <u>IKEC</u>				Boring Location <u>444507.211 N; 562914.551 E</u>					
Project Number <u>175539026</u>				Surface Elevation <u>501.6 ft</u>		Elevation Datum <u>NAVD88</u>			
Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
481.6	20.0	LEAN CLAY (CL), light brown and tan, moist, very stiff, some gravel <i>(Continued)</i> -at 15.0', medium plasticity		SPT-5	15.0 - 16.5	1.5	7-11-12	--	
		SHALE, light gray to gray, moist, soft rock, horizontal bedding, some fractures		SPT-6	20.0 - 20.3	0.3	50/4"	--	
476.2	25.4			SPT-7	25.0 - 25.4	0.4	50/5"	--	
Auger Refusal / Bottom of Hole									
Borehole was backfilled with a mixture of cement-bentonite grout from the bottom of hole to the ground surface using a tremie pipe.									

TVA RO BORING LOG - CLIFTY - BORINGS_LOSS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-03</u>		Stantec Boring No. LRCP-21-03	
Client <u>IKEC</u>	Boring Location <u>444860.837 N; 562093.44 E</u>		
Project Number <u>175539026</u>	Surface Elevation <u>490.9 ft</u>	Elevation Datum <u>NAVD88</u>	
Project Name <u>Clifty Creek WBSP and LRCP Closure</u>	Date Started <u>3/9/21</u>	Completed <u>3/9/21</u>	
Project Location <u>Clifty Creek Power Plant, Madison, IN</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Logged by <u>B. Herries</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Drilling Contractor <u>Stantec Consulting Services Inc.</u>	Drill Rig Type and ID <u>CME 55 Track Rig #711</u>		
Overburden Drilling and Sampling Tools (Type and Size) <u>4.25" HSA, 2" Split Spoon w/o liners, 3" Shelby Tubes</u>			
Rock Drilling and Sampling Tools (Type and Size) <u>N/A</u>			
Sampler Hammer Type <u>Automatic</u>	Weight <u>140 lb</u>	Drop <u>30 in</u>	Efficiency <u>88 % (Avg.)</u>
Borehole Azimuth <u>N/A (Vertical)</u>		Borehole Inclination (from Vertical) <u>Vertical</u>	

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
490.9	0.0	Top of Hole							
		FAT CLAY (CH), light brown with brown and little green, moist, very soft to very stiff, trace gravel, roots from 0' to 1.5'		SPT-1	0.0 - 1.5	0.5	WH-WH-1	--	
		-at 5.7', roots and organic matter from 5.7' to 5.8'		SPT-2	5.0 - 6.5	1.2	4-4-6	--	
		-at 10.9', gravel and cobbles from 10.9' to 11.5'		SPT-3	10.0 - 11.5	1.5	7-11-13	--	

TVA RO BORING LOG - CLIFTY - BORINGS_LOGS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-03</u>		Stantec Boring No. LRCP-21-03	
Client <u>IKEC</u>		Boring Location <u>444860.837 N; 562093.44 E</u>	
Project Number <u>175539026</u>		Surface Elevation <u>490.9 ft</u> Elevation Datum <u>NAVD88</u>	

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
		FAT CLAY (CH), light brown with brown and little green, moist, very soft to very stiff, trace gravel, roots from 0' to 1.5' <i>(Continued)</i> -at 15.6', gray with brown gravelly clay seam from 15.6' to 16.5'		SPT-4	15.0 - 16.5	1.5	13-22-18	--	
471.7	19.2		Grab-1	18.0 - 19.0		Grab-1-no SPT		--	

Auger Refusal /
Bottom of Hole

Borehole was backfilled with a mixture of cement-bentonite grout from the bottom of hole to the ground surface using a tremie pipe.

T:\A RO BORING LOG - CLIFTY - BORINGS_LOSS.GPJ - FMSM-GRAPHIC LOG.GDT 4/12/21

Client Borehole Identification <u>LRCP-21-03A</u>		Stantec Boring No. LRCP-21-03A	
Client <u>IKEC</u>	Boring Location <u>444802.654 N; 562060.15 E</u>		
Project Number <u>175539026</u>	Surface Elevation <u>497.1 ft</u>	Elevation Datum <u>NAVD88</u>	
Project Name <u>Clifty Creek WBSP and LRCP Closure</u>	Date Started <u>3/10/21</u>	Completed <u>3/10/21</u>	
Project Location <u>Clifty Creek Power Plant, Madison, IN</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Logged by <u>B. Herries</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Drilling Contractor <u>Stantec Consulting Services Inc.</u>	Drill Rig Type and ID <u>CME 55 Track Rig #711</u>		
Overburden Drilling and Sampling Tools (Type and Size) <u>4.25" HSA, 2" Split Spoon w/o liners, 3" Shelby Tubes</u>			
Rock Drilling and Sampling Tools (Type and Size) <u>N/A</u>			
Sampler Hammer Type <u>Automatic</u>	Weight <u>140 lb</u>	Drop <u>30 in</u>	Efficiency <u>88 % (Avg.)</u>
Borehole Azimuth <u>N/A (Vertical)</u>		Borehole Inclination (from Vertical) <u>Vertical</u>	

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
497.1	0.0	Top of Hole							
		FAT CLAY (CH), brown with dark brown, moist, stiff to very stiff, trace to some gravel, some roots from 0' to 1.5'		SPT-1	0.0 - 1.5	1.0	6-7-2	--	
		-at 5.0', light brown and brown		SPT-2	5.0 - 6.5	1.1	2-3-14	--	
				SPT-3	10.0 - 11.5	1.0	8-9-13	--	

TVA RO BORING LOG - CLIFTY - BORINGS_LOGS.GPJ - FMSM+GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-03A</u>		Stantec Boring No. <u>LRCP-21-03A</u>	
Client <u>IKEC</u>		Boring Location <u>444802.654 N; 562060.15 E</u>	
Project Number <u>175539026</u>		Surface Elevation <u>497.1 ft</u> Elevation Datum <u>NAVD88</u>	

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
481.8	15.3	SHALE, light gray with white, dry, highly fractured, horizontal bedding		SPT-4	15.0 - 15.9	0.9	41-50/5"	--	
481.2	15.9								

Auger Refusal /
Bottom of Hole

Borehole was backfilled with a mixture of cement-bentonite grout from the bottom of hole to the ground surface using a tremie pipe.

TVA RO BORING LOG - CLIFTY - BORINGS_LOSS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-04</u>		Stantec Boring No. LRCP-21-04	
Client <u>IKEC</u>	Boring Location <u>444061.327 N; 561916.533 E</u>		
Project Number <u>175539026</u>	Surface Elevation <u>509.0 ft</u>	Elevation Datum <u>NAVD88</u>	
Project Name <u>Clifty Creek WBSP and LRCP Closure</u>	Date Started <u>3/10/21</u>	Completed <u>3/10/21</u>	
Project Location <u>Clifty Creek Power Plant, Madison, IN</u>	Depth to Water <u>15.5 ft</u>	Date/Time <u>3/10/21</u>	
Logged by <u>B. Herries</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Drilling Contractor <u>Stantec Consulting Services Inc.</u>	Drill Rig Type and ID <u>CME 55 Track Rig #711</u>		
Overburden Drilling and Sampling Tools (Type and Size) <u>4.25" HSA, 2" Split Spoon w/o liners, 3" Shelby Tubes</u>			
Rock Drilling and Sampling Tools (Type and Size) <u>N/A</u>			
Sampler Hammer Type <u>Automatic</u>	Weight <u>140 lb</u>	Drop <u>30 in</u>	Efficiency <u>88 % (Avg.)</u>
Borehole Azimuth <u>N/A (Vertical)</u>	Borehole Inclination (from Vertical) <u>Vertical</u>		

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
509.0	0.0	Top of Hole							
508.5	0.5	TOPSOIL							
		FAT CLAY (CH), light brown with tan, moist, stiff to very stiff, trace gravel, angular to subangular		SPT-1	0.0 - 1.5	0.5	2-4-6	--	
		-at 5.9', coarse gravel and cobbles for 4"		SPT-2	5.0 - 6.5	1.5	4-4-8	--	
		-at 10.0', roots for 1"		SPT-3	10.0 - 11.5	1.1	4-8-8	--	
		-at 11.0', coarse gravel and cobbles for 6"							
495.0	14.0	GRAVEL WITH SAND (GP), gray to dark gray, moist to wet, dense to medium dense, angular, poorly graded		SPT-4	14.0 - 15.5	0.6	25-7-10	--	
489.0	20.0								

TVA RO BORING LOG - CLIFTY - BORINGS_LOGS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-04</u>				Stantec Boring No. LRCP-21-04					
Client <u>IKEC</u>			Boring Location <u>444061.327 N; 561916.533 E</u>						
Project Number <u>175539026</u>			Surface Elevation <u>509.0 ft</u>		Elevation Datum <u>NAVD88</u>				
Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
487.0	22.0	FAT CLAY (CH), gray to olive, moist, very stiff, some gravel, trace sand, subangular to angular. Top of Rock at 22.0'		SPT-5	20.0 - 21.5	1.3	12-7-14	--	
483.7	25.3	SHALE, light gray to gray, dry, highly weathered, fractured, horizontal bedding		SPT-6	25.0 - 25.3	0.3	50/4"	--	
<p>Auger Refusal / Bottom of Hole</p> <p>Borehole was backfilled with a mixture of cement-bentonite grout from the bottom of hole to the ground surface using a tremie pipe.</p>									

TVA RO BORING LOG - CLIFTY - BORINGS_LOGS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-05</u>		Stantec Boring No. LRCP-21-05	
Client <u>IKEC</u>	Boring Location <u>443875.965 N; 563044.959 E</u>		
Project Number <u>175539026</u>	Surface Elevation <u>504.8 ft</u>	Elevation Datum <u>NAVD88</u>	
Project Name <u>Clifty Creek WBSP and LRCP Closure</u>	Date Started <u>3/11/21</u>	Completed <u>3/13/21</u>	
Project Location <u>Clifty Creek Power Plant, Madison, IN</u>	Depth to Water <u>13.2 ft</u>	Date/Time <u>3/11/21</u>	
Logged by <u>B. Herries</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Drilling Contractor <u>Stantec Consulting Services Inc.</u>	Drill Rig Type and ID <u>CME 55 Track Rig #711</u>		
Overburden Drilling and Sampling Tools (Type and Size) <u>4.25" HSA, 2" Split Spoon w/o liners, 3" Shelby Tubes</u>			
Rock Drilling and Sampling Tools (Type and Size) <u>N/A</u>			
Sampler Hammer Type <u>Automatic</u>	Weight <u>140 lb</u>	Drop <u>30 in</u>	Efficiency <u>88 % (Avg.)</u>
Borehole Azimuth <u>N/A (Vertical)</u>	Borehole Inclination (from Vertical) <u>Vertical</u>		

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core						
504.8	0.0	Top of Hole							
504.6	0.2	TOPSOIL		SPT-1	0.0 - 1.5	0.7	2-4-4	--	
504.4	0.4	ASPHALT							
502.3	2.5	GRAVELLY CLAY (CL) with sand, light brown with gray, moist, medium stiff		SPT-2	2.5 - 4.0	1.4	6-22-10	--	
		LEAN CLAY (CL), light brown to tan, moist, very stiff to hard, some gravel, subangular		SPT-3	5.0 - 6.5	1.5	13-16-26	--	
		-at 3.6', coarse gravel for 2"							
		-at 5.0', some gravel		SPT-4	7.5 - 9.0	1.5	13-27-35	--	
		-at 8.0', clay is gray for 1.8'							
				SPT-5	10.0 - 11.5	1.2	7-9-13	--	
				SPT-6	12.5 - 14.0	1.5	11-14-24	--	
		-at 13.2, wet							
489.8	15.0	SHALE, gray to dark gray, moist, hard, residual soil, weathered		SPT-7	15.0 - 15.6	0.6	43-50/1"	--	

TVA RO BORING LOG - CLIFTY - BORINGS_LOGS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-05</u>		Stantec Boring No. LRCP-21-05	
Client <u>IKEC</u>		Boring Location <u>443875.965 N; 563044.959 E</u>	
Project Number <u>175539026</u>		Surface Elevation <u>504.8 ft</u> Elevation Datum <u>NAVD88</u>	

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
479.5	25.3	SHALE, gray to dark gray, moist, hard, residual soil, weathered <i>(Continued)</i> -at 20.0', horizontal bedding, less weathered		SPT-8	20.0 - 20.4	0.4	50/5"	--	
				SPT-9	22.5 - 22.9	0.4	50/5"	--	
				SPT-10	25.0 - 25.3	0.3	50/4"	--	

Auger Refusal /
Bottom of Hole

Borehole was backfilled with a mixture of cement-bentonite grout from the bottom of hole to the ground surface using a tremie pipe.

TVA RO BORING LOG - CLIFTY - BORINGS_LOSS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Client Borehole Identification <u>LRCP-21-06</u>		Stantec Boring No. LRCP-21-06	
Client <u>IKEC</u>	Boring Location <u>444082.85 N; 563226.249 E</u>		
Project Number <u>175539026</u>	Surface Elevation <u>506.2 ft</u>	Elevation Datum <u>NAVD88</u>	
Project Name <u>Clifty Creek WBSP and LRCP Closure</u>	Date Started <u>3/14/21</u>	Completed <u>3/14/21</u>	
Project Location <u>Clifty Creek Power Plant, Madison, IN</u>	Depth to Water <u>22.5 ft</u>	Date/Time <u>3/14/21</u>	
Logged by <u>B. Herries</u>	Depth to Water <u>N/A</u>	Date/Time <u>N/A</u>	
Drilling Contractor <u>Stantec Consulting Services Inc.</u>	Drill Rig Type and ID <u>CME 55 Track Rig #711</u>		
Overburden Drilling and Sampling Tools (Type and Size) <u>4.25" HSA, 2" Split Spoon w/o liners, 3" Shelby Tubes</u>			
Rock Drilling and Sampling Tools (Type and Size) <u>N/A</u>			
Sampler Hammer Type <u>Automatic</u>	Weight <u>140 lb</u>	Drop <u>30 in</u>	Efficiency <u>88 % (Avg.)</u>
Borehole Azimuth <u>N/A (Vertical)</u>		Borehole Inclination (from Vertical) <u>Vertical</u>	

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core						
506.2	0.0	Top of Hole							
505.6	0.6	CRUSHED STONE AND ASPHALT							
		LEAN CLAY (CL) with gravel, tan with light brown, moist, stiff, angular to subangular		SPT-1	2.5 - 4.0	1.3	3-6-6	--	
		-at 5.9', gravel for 2"		SPT-2	5.0 - 6.5	1.2	3-7-5	--	
498.7	7.5	FAT CLAY (CH), light brown with gray to blueish gray, moist, stiff to hard, with gravel and sand		SPT-3	7.5 - 9.0	1.3	2-7-14	--	
		-at 10.7', olive gray with blue gray		SPT-4	10.0 - 11.5	1.2	6-5-8	--	
				SPT-5	12.5 - 14.0	1.5	2-3-4	--	

TVA RO BORING LOG - CLIFTY BORINGS_LOGS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
486.2	20.0	FAT CLAY (CH), light brown with gray to blueish gray, moist, stiff to hard, with gravel and sand <i>(Continued)</i>		SPT-6	15.0 - 16.5	0.0	6-7-8	--	
		-at 17.5', olive to olive gray for 1.5'		SPT-7	17.5 - 19.0	0.9	7-9-13	--	
481.2	25.0	LEAN CLAY (CL), light brown to brown, some mottling with gray, moist, stiff, low to medium plasticity		SPT-8	20.0 - 21.5	1.5	3-4-8	--	
		-at 22.5', wet, soft		SPT-9	22.5 - 24.0	1.5	3-2-3	--	
		FAT CLAY (CH), brownish orange with with light brown and gray, moist, stiff		SPT-10	25.0 - 26.5	1.5	3-4-12	--	
		-at 30.2', coarse gravel for 1" -at 31.3', coarse gravel for 2"		SPT-11	30.0 - 31.6	1.5	8-9-11	--	

TVA RO BORING LOG - CLIFTY_BORINGS_LOSS.GPJ_FMSM+GRAPHIC.LOG.GDT_4/12/21

Client Borehole Identification <u>LRCP-21-06</u>		Stantec Boring No. LRCP-21-06	
Client <u>IKEC</u>		Boring Location <u>444082.85 N; 563226.249 E</u>	
Project Number <u>175539026</u>		Surface Elevation <u>506.2 ft</u> Elevation Datum <u>NAVD88</u>	

Lithology		Description	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	Remarks
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	
		FAT CLAY (CH), brownish orange with with light brown and gray, moist, stiff <i>(Continued)</i> -at 36.0', coarse gravel for 6"		SPT-12	35.0 - 36.5	1.0	12-10-15	--	
466.2	40.0								
466.0	40.2	SHALE, gray, dry							
		Auger Refusal / Bottom of Hole							
		Borehole was backfilled with a mixture of cement-bentonite grout from the bottom of hole to the ground surface using a tremie pipe.							

TVA RO BORING LOG - CLIFTY - BORINGS_LOSS.GPJ - FMSM-GRAPHIC LOG.GDT - 4/12/21

APPENDIX F

Slope Stability Analysis



**Landfill Runoff Collection Pond
Dam Stability**

Closure Plan
Landfill Runoff Collection Pond
Clifty Creek Station
Madison, Jefferson County, Indiana

April 16, 2021

Prepared for:

Indiana-Kentucky Electric Corporation
(IKEC)

Prepared by:

Stantec Consulting Services Inc.

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LIST OF ATTACHMENTS

ATTACHMENT A SLOPE STABILITY ANALYSES OUTPUTS



LANDFILL RUNOFF COLLECTION POND DAM STABILITY

INTRODUCTION

1.0 INTRODUCTION

The Clifty Creek Generating Station's Landfill Runoff Collection Pond (LRCP), owned and operated by the Indiana Kentucky Electric Corporation (IKEC), is located in Jefferson County, Indiana. The facility is bordered in the south by the Ohio River, on the east a bedrock outcrop known as the Devil's Backbone and on the west by high ground. The LRCP served as a settling facility for sluiced fly ash produced by the generating plant. The LRCP will no longer receive fly ash and closure documentation is being prepared.

Stantec Consulting Services, Inc. (Stantec) was contracted by the IKEC to prepare construction design documents to support the closure of the LRCP. The purpose of this report is to present the results of slope stability analyses of the LRCP dam with the proposed closure configuration of closure. Closure will be executed by excavating, filling, and grading existing fly ash in the pond to create positive drainage and will include the installation of a cover system. The existing dam will be degraded as stormwater run-on will be captured by a diversion ditch and routed downstream. The cover system, from top to bottom, will consist of 6 inches of topsoil, 2.5 feet of cover soil, a geocomposite drainage layer, and a 40-mil thickness linear low-density polyethylene (LLDPE) flexible membrane liner (FML) overlying the in-place fly ash coal combustion residual (CCR) material.

This report documents the information reviewed from previous field explorations and laboratory testing and geotechnical engineering analyses performed by Stantec to support the closure requirements. The scope of work includes the following:

- Existing documentation review
- Slope stability analyses of the modified LRCP dam under long-term, 1/2 Probable Maximum Flood (PMF), and seismic loading conditions

2.0 GLOBAL SLOPE STABILITY ANALYSES

The global slope stability was evaluated using conventional, limit equilibrium, method of slices analysis as implemented in the SLOPE/W module of GeoStudio 2021. Spencer's method was selected for the analyses as this method includes all interslice forces and satisfies both moment and force equilibrium. A slope stability analysis includes a search for the most critical slip surface, corresponding to the lowest factor of safety. Several options are available in SLOPE/W to facilitate the search for the critical failure surface. In the analyses presented in this report, potential circular failure surfaces were generated using the "entry and exit" method. Considering shallow, surficial failures pose little risk to the overall stability of the slopes, and are usually considered as a potential maintenance issue, a minimum slip surface depth of 3 feet was specified in the analyses to force the evaluation on the deeper potential failure surfaces.

When a soil at the entry of a potential failure surface is assigned with a cohesion value ($c > 0$) in a slope stability analysis, tensile stresses are often computed between the slices in this area. In the field, tensile



LANDFILL RUNOFF COLLECTION POND DAM STABILITY

GLOBAL SLOPE STABILITY ANALYSES

stresses result in the opening of a tension crack, reducing the lateral stresses to zero. Because tension results in a stabilizing force at the head of the sliding mass, it is unconservative to have tensile stresses between the slices in a slope stability analysis. A tension crack line was used in the analysis to eliminate the tensile inter-slice forces. Multiple iterations were performed to define the appropriate tension crack line.

2.1 LOAD CASES, GROUNDWATER CONDITIONS, AND DESIGN CRITERIA

Global slope stability analyses were performed on the final grading of the LRCP closure. The evaluated load cases, along with the target factors of safety, are summarized in Table 2.1. Target factors of safety were selected per criteria presented in the Indiana Administrative Code (Table 1 of 329 IAC 10-15-8). If the slope were to fail, it would cause major environmental impact due to material being released into the Ohio River. There is small uncertainty of soil strengths with the laboratory testing performed (discussed in Stantec 2016). Therefore, the minimum factors of safety used for these analyses were 1.5 for static conditions and 1.3 for seismic, post-earthquake conditions.

Table 2.1: Evaluated Load Cases

Slope	Load Case	Analysis	Target Factor of Safety
LRCP Dam	Long-term	Drained, effective stress	1.5
	1/2 PMF	Drained, effective stress	1.5
	Seismic (Pseudo-static)	Undrained, total stress	1.0
	Seismic (Post-Earthquake)	Residual strengths for liquefied CCR material Undrained, total stress reduced 20% for non-liquefied soils	1.3

2.1.1 Long-Term

This analysis was performed to evaluate the stability of the final grading under the long-term, drained condition after excess pore pressures have dissipated. Drained, effective stress strength parameters were used in the analysis. A long-term piezometric line was applied to the slope stability models assumed to be five feet below the lowest point of the proposed closure liner.

2.1.2 1/2 Probable Maximum Flood

This analysis was performed to evaluate the stability of the LRCP dam under a design storm event equivalent to 1/2 of the site's PMF event. Drained, effective strength parameters were used in the



LANDFILL RUNOFF COLLECTION POND DAM STABILITY

GLOBAL SLOPE STABILITY ANALYSES

analysis. The high water piezometric line was assigned to elevation 487 feet based on an anticipated backwater in the LRCP dam during this flood event.

2.1.3 Seismic (Pseudo-static)

This analysis was performed to evaluate the stability of the embankment dike and final grading under seismic loading from a design earthquake event. Undrained, pseudo-static strength parameters were assigned to low permeability materials in the analysis. Groundwater was assumed to be at the long-term water level.

The US Geological Survey (USGS) Unified Hazard Tool was used to determine the site's peak ground acceleration (PGA) corresponding to a seismic event with a return period of 2,475 years and a peak rock acceleration at the site of 0.0882 g was obtained.

Haynes-Griffin and Franklin (1984) recommended that half of the peak rock acceleration be used as the seismic coefficient in the pseudo-static slope stability analysis. Based on that, a seismic coefficient of 0.0441 ($=0.5 \times 0.0882$) was selected. The selected seismic coefficient was rounded and a horizontal seismic coefficient of 0.045 was used in SLOPE/W. The program calculates seismic force ($=\text{column weight} \times \text{seismic coefficient}$) and applies to each column in the sliding mass.

Note that the pseudo-static stability analysis should only be considered as an index of the seismic resistance available in a structure not subject to build-up of pore pressure from shaking. A pseudo-static factor of safety greater than 1.0 is very strong evidence that there would be little or no damage to the dam from an earthquake (FEMA 2005).

Groundwater was assumed to be at the long-term water level which was assumed to be 5 feet below the proposed closure liner.

2.1.4 Post-Earthquake

This analysis was performed to evaluate the stability of the LRCP dam assuming liquefaction of the LRCP fly ash to evaluate the potential for a release of CCR offsite. The assumption for performing this analysis is that the saturated fly ash (CCR) deposits will undergo pore pressure build-up and liquefy during the design seismic event. A post-earthquake, residual strength (in the form of Residual Shear Strength ratio as discussed in Section 2.3) was applied to the saturated fly ash (CCR) layers. Undrained, total stress strength parameters were reduced 20% and were applied to non-liquifiable soils to account for potential strength loss due to earthquake shaking.

Groundwater was assumed to be at the long-term water level which was assumed to be 5 feet below the proposed closure liner.



2.2 SECTION GEOMETRY

The section of maximum dam height was selected as the critical section for analyses. Locations of the selected cross sections are shown in the drawings. The subsurface profiles modeled in the analyses for each cross section were selected based on historical data from previous explorations. Soil borings selected for definition of the subsurface lithology for each cross section are shown in Table 2.2 below.

Table 2.2: Soil Borings used for Stratigraphy Definition

Cross Section	Soil Boring
LRCP Dam	Stantec B-7, Stantec B-8, Stantec B-9, and Stantec B-10

2.3 MATERIAL PROPERTIES

Material strength parameters used in the stability analyses of this study are summarized in Table 2.3 below. These parameters are the same parameters that were used and selected in the Stantec 2016 Geotechnical Report, with the exception of the Cover Soil which is based on the field exploration results, reviewed historical geotechnical information, and engineering judgement.

Table 2.3: Material Parameters for Slope Stability Analysis

Material	Unit Weight	Drained Shear Strengths		Undrained Shear Strengths	Residual Strength
		Effective Friction Angle ϕ'	Effective Cohesion c'	S_u	S_r
	(pcf)	(deg.)	(psf)	(psf)	(psf)
Embankment	129	27.5	198	1400 ¹	-
Lean Clay with Sand	127	28	206	1200 ¹	-
Clayey Gravel with Sand	130	35	0	-	-
Fly Ash (CCR)	115	25	0	-	$S_r / \sigma_{vc}' = 0.04^6$
Sandy Silt	125	30	0	-	-
Silty Sand	94	30	0	-	-
Cover Soil	125	27	300	1000 ¹	-

1. For Seismic (Post Earthquake Analyses) use 80% of Undrained Shear Strength.



LANDFILL RUNOFF COLLECTION POND DAM STABILITY

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Soil strength parameters, with the exception of the Cover Soil, were derived in the Stantec February 2016 CCR Rule Stability Analyses report. The 2016 parameters were developed using CU triaxial test data for the Embankment Fill and Lean Clay with Sand soils. Strength parameters for the proposed Cover Soil were taken to be identical to the parameters of the Lean Clay with Sand soil and its unit weight was selected based on preliminary standard Proctor testing data of a potential borrow source soil. Undrained shear strength parameters for coarse-grained (or cohesionless) materials were taken to be identical to the drained shear strength parameters.

The residual strength of liquefied fly ash (CCR) layers for post-earthquake evaluations was estimated by applying a conservative Residual Shear Strength ratio, τ/σ (Shear Strength/Effective Overburden Stress Ratio) of 0.04 using correlations presented by Idriss and Boulanger between corrected SPT blow counts and the Residual Shear Strength Ratio. SPT blowcounts were obtained from existing site data boring logs and a representative SPT N value of 3 was selected to obtain the selected Residual Shear Strength ratio of 0.04 for the fly ash materials.

2.4 ANALYSIS RESULTS

The computed factors of safety of slope stability for the LRCP closure grading under the evaluated loading conditions are summarized in Table 2.4, along with the target factors of safety according to the established design criteria for the project. The computed output plots, depicting the predicted critical failure surfaces, are presented in Attachment A.

Table 2.4: Computed Slope Stability Factors of Safety

Section	Location	Loading Condition	Computed Slope Stability Factor of Safety	Target Factor of Safety
LRCP Dam	Dam – Downstream Slope	Long Term	2.15	1.5
	Dam – Upstream Slope	Long Term	4.12	1.5
	Dam – Downstream Slope	½ PMF Pool	1.93	1.5
	Dam – Upstream Slope	½ PMF Pool	4.49	1.5
	Dam – Downstream Slope	Seismic (Pseudo-Static)	1.50	1.0
	Dam – Upstream Slope	Seismic (Pseudo-Static)	3.05	1.0
	Dam – Downstream Slope	Seismic (Post-earthquake)	1.66	1.3
	Dam – Upstream Slope	Seismic (Post-earthquake)	4.15	1.3



CONCLUSION

3.0 CONCLUSION

The calculated factors of safety for the analyzed cases meet the minimum factors of safety required by 329 IAC 10-15-8. This analysis is based on the information discussed in this report and the interpretation of the subsurface conditions encountered at the site. No warranties can be made regarding the continuity of conditions. If future design changes are made, Stantec should be notified so that such changes can be reviewed, and the analysis amended as necessary.

4.0 LIMITATIONS

This report was prepared by Stantec Consulting Services, Inc. (Stantec) for IKEC. Stantec's professional services have been performed using a degree of care and skill ordinarily exercised, under similar circumstances, by reputable geotechnical consultants practicing in this or similar localities. No other warranty, express or implied, is made as to the professional advice included in this report. This geotechnical report has been prepared for IKEC and is to be used solely for the design of the proposed Closure for the Landfill Runoff Collection Pond at the Clifty Creek Station in Jefferson County, Indiana and may not contain sufficient information for use by other parties.

The recommendations provided in this geotechnical report are based upon our understanding of the described project information and our interpretation of available published information and previous field and laboratory investigations. We have made our recommendations based upon experience with similar subsurface conditions. The recommendations apply to the specific project discussed in this report; therefore, any change in the configuration of the proposed design or any change to the site grades should be provided to us so that we can review our conclusions and recommendations and make any necessary modifications.

The recommendations provided in this report are based upon the assumption that the necessary geotechnical observations and testing during construction will be performed by our firm during the entire duration of the construction. The field observation services are considered a continuation of the geotechnical investigation and are essential to verify that the actual soil conditions are as expected. This also provides for the procedure whereby IKEC may be advised of unexpected or changed conditions that would require modifications of our original recommendations.



5.0 REFERENCES

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ATTACHMENT A
SLOPE STABILITY ANALYSES OUTPUTS

Indiana Kentucky Electric Corporation (IKEC) Clifty Creek Generating Station Landfill Runoff Collection Pond Dam - Station 405+70 Madison, Indiana

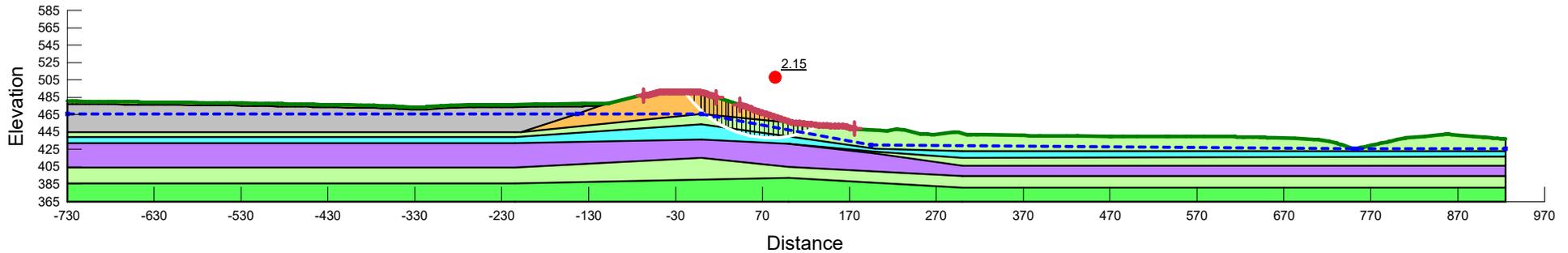
Factor of Safety: 2.15

Long Term, Downstream Slope

Method: Spencer
Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling. No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Green	Clayey Gravel with Sand (Drained)	Mohr-Coulomb	130	0	35
Yellow	Cover Soil (Drained)	Mohr-Coulomb	125	300	27
Orange	Embankment (Drained)	Mohr-Coulomb	129	198	27.5
Grey	Fly Ash (Drained)	Mohr-Coulomb	115	0	25
Light Green	Lean Clay with Sand (Drained)	Mohr-Coulomb	127	206	28
Cyan	Sandy Silt (Drained)	Mohr-Coulomb	125	0	30
Purple	Silty Sand (Seismic Undrained)	Mohr-Coulomb	94	0	30



Indiana Kentucky Electric Corporation (IKEC) Clifty Creek Generating Station Landfill Runoff Collection Pond Dam - Station 405+70 Madison, Indiana

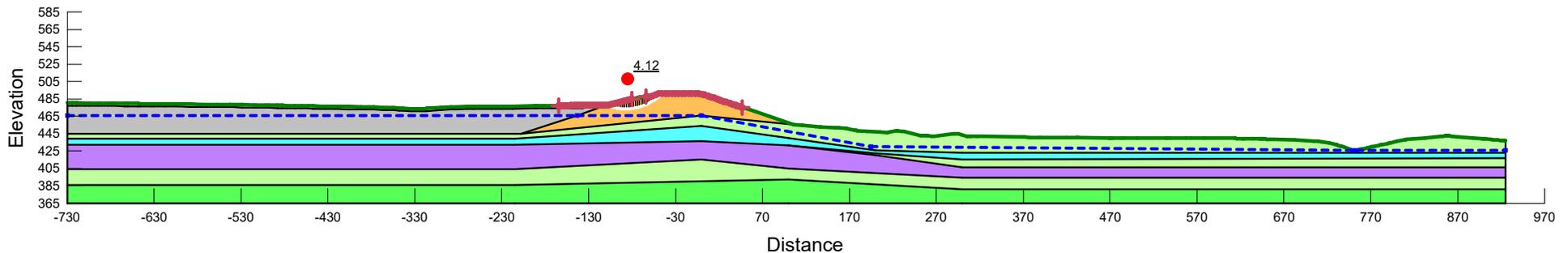
Factor of Safety: 4.12

Long Term, Upstream Slope

Method: Spencer
Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling. No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Green	Clayey Gravel with Sand (Drained)	Mohr-Coulomb	130	0	35
Yellow	Cover Soil (Drained)	Mohr-Coulomb	125	300	27
Orange	Embankment (Drained)	Mohr-Coulomb	129	198	27.5
Grey	Fly Ash (Drained)	Mohr-Coulomb	115	0	25
Light Green	Lean Clay with Sand (Drained)	Mohr-Coulomb	127	206	28
Cyan	Sandy Silt (Drained)	Mohr-Coulomb	125	0	30
Purple	Silty Sand (Seismic Undrained)	Mohr-Coulomb	94	0	30



Indiana Kentucky Electric Corporation (IKEC) Clifty Creek Generating Station Landfill Runoff Collection Pond Dam - Station 405+70 Madison, Indiana

Factor of Safety: 1.93

50% PMF Pool, Downstream Slope

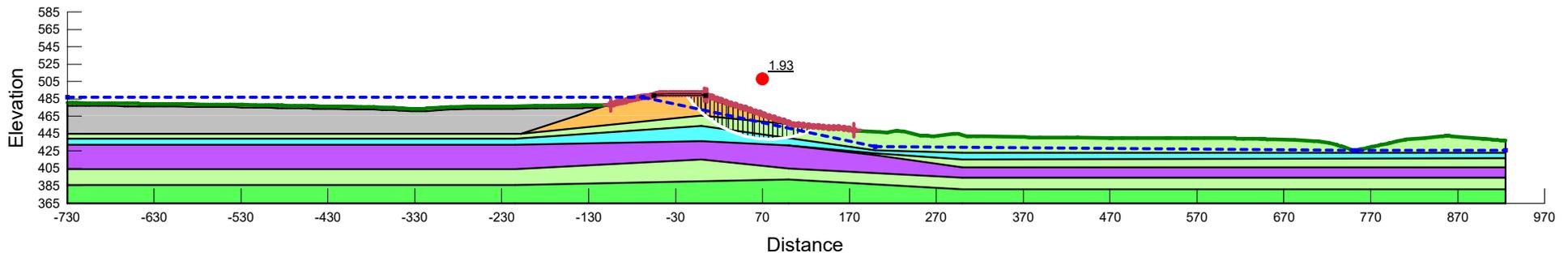
50% PMF Pool Elevation: 487 feet

Method: Spencer

Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling. No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Green	Clayey Gravel with Sand (Drained)	Mohr-Coulomb	130	0	35
Yellow	Cover Soil (Drained)	Mohr-Coulomb	125	300	27
Orange	Embankment (Drained)	Mohr-Coulomb	129	198	27.5
Grey	Fly Ash (Drained)	Mohr-Coulomb	115	0	25
Light Green	Lean Clay with Sand (Drained)	Mohr-Coulomb	127	206	28
Cyan	Sandy Silt (Drained)	Mohr-Coulomb	125	0	30
Purple	Silty Sand (Drained)	Mohr-Coulomb	94	0	30



Indiana Kentucky Electric Corporation (IKEC) Clifty Creek Generating Station Landfill Runoff Collection Pond Dam - Station 405+70 Madison, Indiana

Factor of Safety: 4.49

50% PMF Pool, Upstream Slope

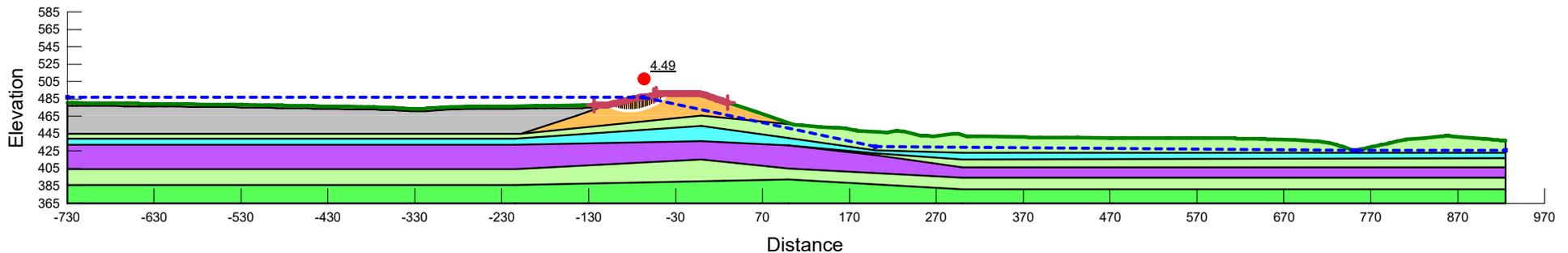
50% PMF Pool Elevation: 487 feet

Method: Spencer

Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling. No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Green	Clayey Gravel with Sand (Drained)	Mohr-Coulomb	130	0	35
Yellow	Cover Soil (Drained)	Mohr-Coulomb	125	300	27
Orange	Embankment (Drained)	Mohr-Coulomb	129	198	27.5
Grey	Fly Ash (Drained)	Mohr-Coulomb	115	0	25
Light Green	Lean Clay with Sand (Drained)	Mohr-Coulomb	127	206	28
Cyan	Sandy Silt (Drained)	Mohr-Coulomb	125	0	30
Purple	Silty Sand (Drained)	Mohr-Coulomb	94	0	30



Indiana Kentucky Electric Corporation (IKEC) Clifty Creek Generating Station Landfill Runoff Collection Pond Dam - Station 405+70 Madison, Indiana

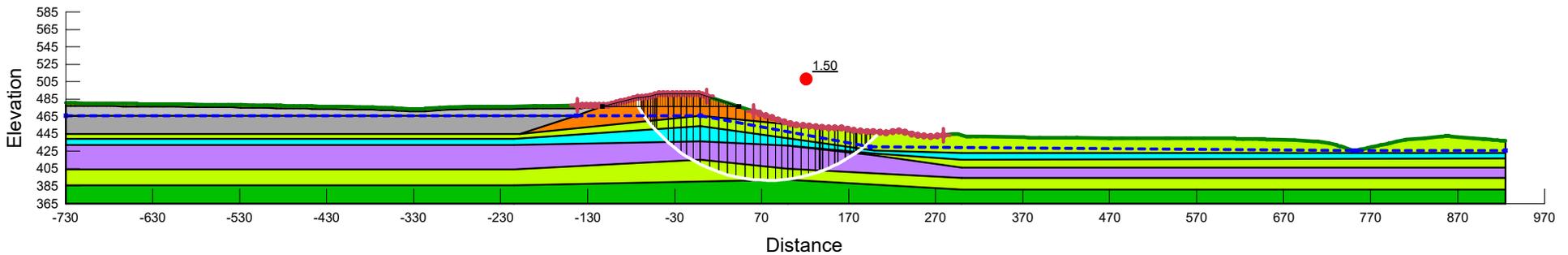
Factor of Safety: 1.50

Seismic (Pseudo-static), Downstream Slope

Method: Spencer
Slip Surface: Entry and Exit
Horz Seismic Coef.: 0.045

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling. No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Cohesion (psf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Green	Clayey Gravel with Sand (Seismic Undrained)	Mohr-Coulomb	130		0	35
Yellow	Cover Soil (Seismic Undrained)	Undrained (Phi=0)	125	1,000		
Orange	Embankment (Seismic Undrained)	Undrained (Phi=0)	129	1,400		
Light Gray	Fly Ash (Drained)	Mohr-Coulomb	115		0	25
Dark Gray	Fly Ash (Seismic Undrained)	Mohr-Coulomb	115		0	25
Light Green	Lean Clay with Sand (Seismic Undrained)	Undrained (Phi=0)	127	1,200		
Cyan	Sandy Silt (Seismic Undrained)	Mohr-Coulomb	125		0	30
Purple	Silty Sand (Seismic Undrained)	Mohr-Coulomb	94		0	30



Indiana Kentucky Electric Corporation (IKEC) Clifty Creek Generating Station Landfill Runoff Collection Pond Dam - Station 405+70 Madison, Indiana

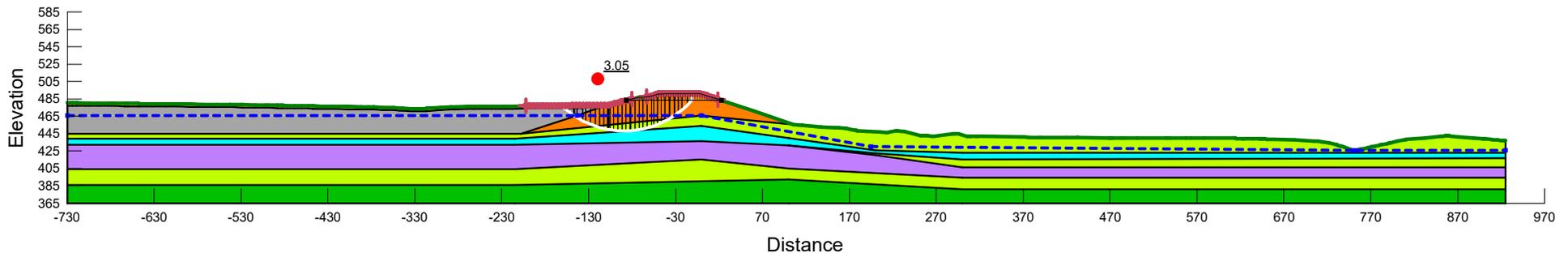
Factor of Safety: 3.05

Seismic (Pseudo-static), Upstream Slope

Method: Spencer
Slip Surface: Entry and Exit
Horz Seismic Coef.: 0.045

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling. No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Cohesion (psf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Green	Clayey Gravel with Sand (Seismic Undrained)	Mohr-Coulomb	130		0	35
Yellow	Cover Soil (Seismic Undrained)	Undrained (Phi=0)	125	1,000		
Orange	Embankment (Seismic Undrained)	Undrained (Phi=0)	129	1,400		
Grey	Fly Ash (Seismic Undrained)	Mohr-Coulomb	115		0	25
Light Green	Lean Clay with Sand (Seismic Undrained)	Undrained (Phi=0)	127	1,200		
Cyan	Sandy Silt (Seismic Undrained)	Mohr-Coulomb	125		0	30
Purple	Silty Sand (Seismic Undrained)	Mohr-Coulomb	94		0	30



Indiana Kentucky Electric Corporation (IKEC) Clifty Creek Generating Station Landfill Runoff Collection Pond Dam - Station 405+70 Madison, Indiana

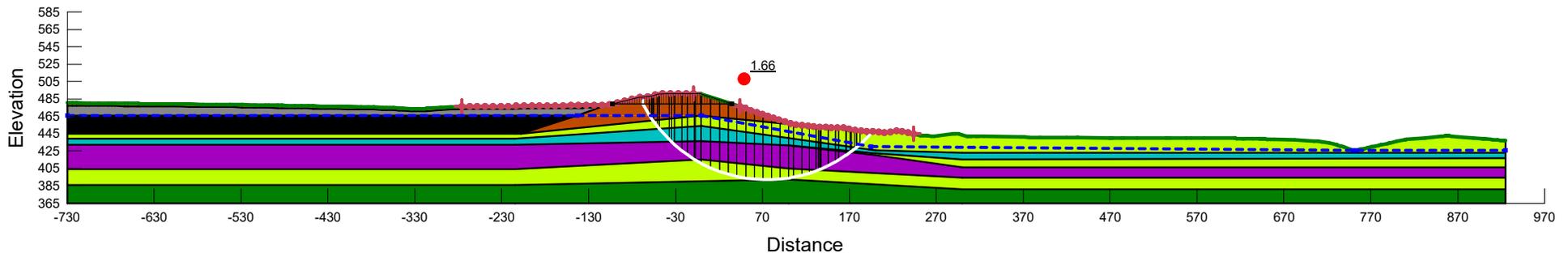
Factor of Safety: 1.66

Seismic (Post Earthquake), Downstream Slope

Method: Spencer
Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling. No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Tau/Sigma Ratio	Cohesion (psf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Green	Clayey Gravel with Sand (Post Earthquake)	Mohr-Coulomb	130			0	35
Yellow-Green	Cover Soil (Post Earthquake)	Undrained (Phi=0)	125		800		
Orange	Embankment (Post Earthquake)	Mohr-Coulomb	129			1,120	27.5
Grey	Fly Ash (Post Earthquake)	Mohr-Coulomb	115			0	25
Black	Fly Ash (Residual Strength)	SHANSEP	115	0.04			
Light Green	Lean Clay with Sand (Post Earthquake)	Undrained (Phi=0)	127		960		
Cyan	Sandy Silt (Post Earthquake)	Mohr-Coulomb	125			0	30
Purple	Silty Sand (Post Earthquake)	Mohr-Coulomb	94			0	30



Indiana Kentucky Electric Corporation (IKEC) Clifty Creek Generating Station Landfill Runoff Collection Pond Dam - Station 405+70 Madison, Indiana

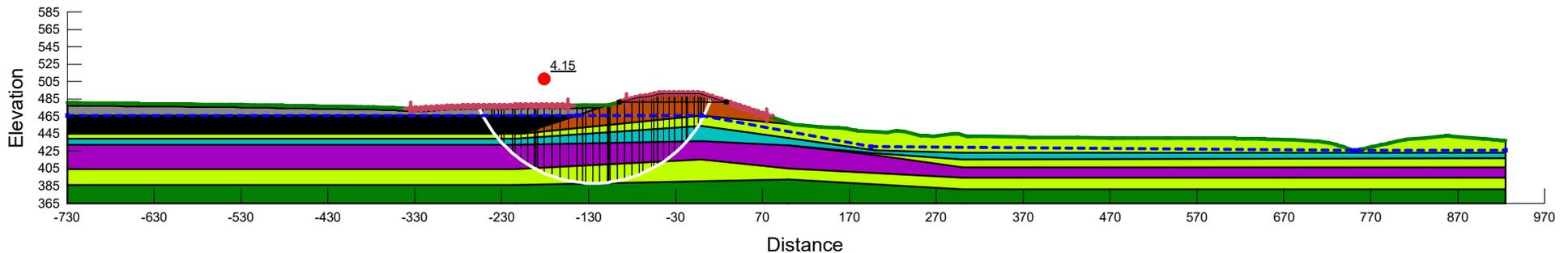
Factor of Safety: 4.15

Seismic (Post Earthquake), Upstream Slope

Method: Spencer
Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling. No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Tau/Sigma Ratio	Cohesion (psf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Green	Clayey Gravel with Sand (Post Earthquake)	Mohr-Coulomb	130			0	35
Yellow	Cover Soil (Post Earthquake)	Undrained (Phi=0)	125		800		
Orange	Embankment (Post Earthquake)	Mohr-Coulomb	129			1,120	27.5
Grey	Fly Ash (Post Earthquake)	Mohr-Coulomb	115			0	25
Black	Fly Ash (Residual Strength)	SHANSEP	115	0.04			
Light Green	Lean Clay with Sand (Post Earthquake)	Undrained (Phi=0)	127		960		
Cyan	Sandy Silt (Post Earthquake)	Mohr-Coulomb	125			0	30
Purple	Silty Sand (Post Earthquake)	Mohr-Coulomb	94			0	30



APPENDIX G

LRCP Permit Drawings

APPENDIX H

Ditch Sizing Calculations

Ohio Valley Electric Corporation / Indiana-Kentucky Electric Corporation Madison Township, Indiana	Clifty Creek Station Landfill Runoff Collection Pond												
 Stantec	Hydrologic and Hydraulic Analysis Ditch Sizing Calculations												
<p><u>Purpose:</u></p> <ul style="list-style-type: none"> Calculations to determine sizing of drainage ditches on the final grade closure plan 													
<p><u>Methods:</u></p> <p>PCSWMM was used to size and model the stormwater ditch network and simulate the peak discharges in the ditches. The model was simulated using the Curve Number Infiltration method and dynamic wave flow routing.</p>													
<p>Table 1 - Design Standards and References</p>													
<table border="1"> <thead> <tr> <th>Parameter</th> <th>Design Standard/Method/Source</th> </tr> </thead> <tbody> <tr> <td>Design Storm</td> <td>25-Year, 24-Hour Recurrence Interval</td> </tr> <tr> <td>Curve Number</td> <td>TR-55, SCS CN methodology</td> </tr> <tr> <td>Rainfall Temporal Distribution</td> <td>Soil Conservation Service (SCS) Type II (USDA, 1986)</td> </tr> <tr> <td>Rainfall Intensity</td> <td>NOAA Atlas 14 Precipitation Frequency Data Server</td> </tr> <tr> <td>Subbasin Area</td> <td>Delineation based on Permit Drawings Dated 04.16.2021</td> </tr> </tbody> </table>		Parameter	Design Standard/Method/Source	Design Storm	25-Year, 24-Hour Recurrence Interval	Curve Number	TR-55, SCS CN methodology	Rainfall Temporal Distribution	Soil Conservation Service (SCS) Type II (USDA, 1986)	Rainfall Intensity	NOAA Atlas 14 Precipitation Frequency Data Server	Subbasin Area	Delineation based on Permit Drawings Dated 04.16.2021
Parameter	Design Standard/Method/Source												
Design Storm	25-Year, 24-Hour Recurrence Interval												
Curve Number	TR-55, SCS CN methodology												
Rainfall Temporal Distribution	Soil Conservation Service (SCS) Type II (USDA, 1986)												
Rainfall Intensity	NOAA Atlas 14 Precipitation Frequency Data Server												
Subbasin Area	Delineation based on Permit Drawings Dated 04.16.2021												
<p><u>Parameters:</u></p> <p>Climatological Data:</p> <p>The 25-year, 24-hour storm was used to determine ditch capacity. Peak rainfall depths for the design storm were taken from NOAA Atlas 14 Precipitation Frequency Data Server specific to the geographic location of the Clifty Creek Plant. The selected Point Precipitation depth is shown in Table 1. An SCS Type II temporal distribution was used to model the rainfall hyetograph over the 24-hour duration.</p>													
<p style="text-align: center;">Table 1 - NOAA Atlas 14 PFDS Rainfall Depths</p> <table border="1" style="margin-left: auto; margin-right: auto;"> <thead> <tr> <th>Storm Return Interval and Duration</th> <th>Rainfall depth (inches)</th> </tr> </thead> <tbody> <tr> <td>25-year, 24-hour</td> <td>5.30</td> </tr> </tbody> </table>		Storm Return Interval and Duration	Rainfall depth (inches)	25-year, 24-hour	5.30								
Storm Return Interval and Duration	Rainfall depth (inches)												
25-year, 24-hour	5.30												

Watershed Delineation:

Subcatchment delineations were completed in PCSWMM based on the permit design final grade surface. Table 2 provides a breakdown of the subcatchment areas. Subcatchment delineations are shown in Appendix A

Curve Numbers

The NRCS curve number method was used to estimate infiltration during the design storm event. A composite curve number was generated for the watershed using SCS hydrologic soil group data and land use data determined from aerial imagery. Curve number values for each land use and soil type combination were assigned based on the values published in Tables 2-2a through 2-2d in TR-55 (NRCS, 1986). To model the final closed conditions, land uses of "Meadow" (CN = 78) was assumed for the cap liner system and land use of "Open spaces – Fair" (CN = 84) was assumed for regraded areas around the cap. A summary of curve numbers used in this analysis is provided in Table 2.

Table 2 – Subbasin Drainage Areas and Curve Numbers

Subbasin	Area (acres)	Composite Curve Number
Ash_Pond	46.46	98
R1_5	14.29	93
R1_6	22.17	79
R1_7	14.16	78
R5	3.44	92
R6	4.26	80
R7	3.54	94
R8	3.41	77
S2	0.32	94
S3	7.99	78
S4	1.69	83
S5	0.99	67
SB10	37.16	76
SB11	35.73	75
SB16	5.91	94
SB18	6.21	94
SB6	31.55	76
SB7	84.24	80
SB8	71.00	79
SB9	19.75	75
SB-A	20.89	81
SB-F	12.14	77
SB-G_1	9.41	93
SB-G_2	6.37	89
SB-G_3	5.03	94
SB-H_1	6.66	92
SB-H_2	10.66	93

Calculations/Results:

Peak Discharges were calculated in the dynamic PCSWMM model. Peak discharges for each ditch type are shown in the table below

Table 3 – Peak Discharge Calculations

Ditch	Peak 25-year Q (cfs)
Diversion Ditch Upstream	227.1
Diversion Ditch Downstream	380.05

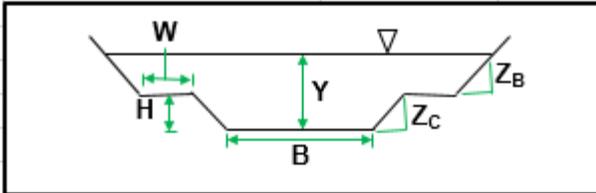
Peak discharges were used to calculate normal depth in the final cover ditches. Ditches were sized to convey the peak discharge accordingly.

Methods:

Hydraulic calculations were performed to determine minimum ditch size required to convey the peak discharges. Manning's equation was used to estimate flow depth in each ditch. The peak flow calculated was used to size all ditches.

Diversion Ditch Upstream Calculations – 25-year, 24-hour storm

Normal Depth - Benched Trapezoidal Channel



User Input		
Discharge (cfs)	Q	227.1
Bottom Width (ft)	B	10
Side Slope 1 (ft/ft)	Z _{c1}	4
Side Slope 2 (ft/ft)	Z _{c2}	4
Bench Width Total (ft)	W ₁ +W ₂	0
Bench Stage (ft)	H	10
Bench Side Slope 1 (ft/ft)	Z _{B1}	4
Bench Side Slope 2 (ft/ft)	Z _{B2}	4
Channel Slope (ft/ft)	S _o	0.006
Manning's Roughness	n	0.017

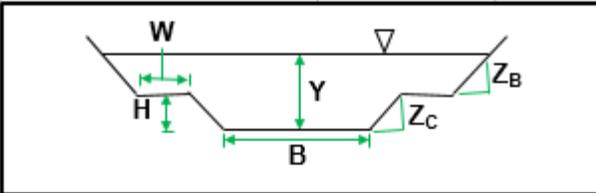
Evaluate Normal Depth

Normal Depth Output		
Normal Depth (ft)	Y	1.74
Calculated Flow (cfs)	Q _c	227.1
Flow Area (ft ²)	A	29.4
Wetted Perimeter (ft)	P _w	24.3
Hydraulic Radius (ft)	R _H	1.21
Top Width (ft)	TW	24
Average Velocity (ft/s)	V _{avg}	7.71
Specific Energy (ft)	E	2.66
Froude Number	FN	1.22

The maximum estimated depth in the channel is 1.74 feet and is less than the design depth of 4 feet.

Diversion Ditch Downstream Calculations – 25-year, 24-hour storm

Normal Depth - Benched Trapezoidal Channel



User Input		
Discharge (cfs)	Q	380.05
Bottom Width (ft)	B	12
Side Slope 1 (ft/ft)	Z _{c1}	4
Side Slope 2 (ft/ft)	Z _{c2}	4
Bench Width Total (ft)	W ₁ , W ₂	0
Bench Stage (ft)	H	10
Bench Side Slope 1 (ft/ft)	Z _{B1}	4
Bench Side Slope 2 (ft/ft)	Z _{B2}	4
Channel Slope (ft/ft)	S _o	0.005
Manning's Roughness	n	0.017

Evaluate Normal Depth

Normal Depth Output		
Normal Depth (ft)	Y	2.22
Calculated Flow (cfs)	Q _c	380.1
Flow Area (ft ²)	A	46.2
Wetted Perimeter (ft)	P _w	30.3
Hydraulic Radius (ft)	R _H	1.53
Top Width (ft)	TW	30
Average Velocity (ft/s)	V _{avg}	8.22
Specific Energy (ft)	E	3.27
Froude Number	FN	1.16

The maximum estimated depth in the channel is 2.22 feet and is less than the design depth of 4 feet.

References:

NOAA. (2013). NOAA Atlas 14 Point Precipitation Frequency Estimates. Retrieved July, 2015, from <http://dipper.nws.noaa.gov/hdsc/pfds/>

USDA. (1986). Urban Hydrology for Small Watersheds, TR-55. United States Department of Agriculture.

Indiana Department of Transportation – 2013 Design Manual, Chapter 202 Hydrology, Revision Date Feb. 2014

Attachments:

Attachment A: Final Grade Subbasin Boundaries

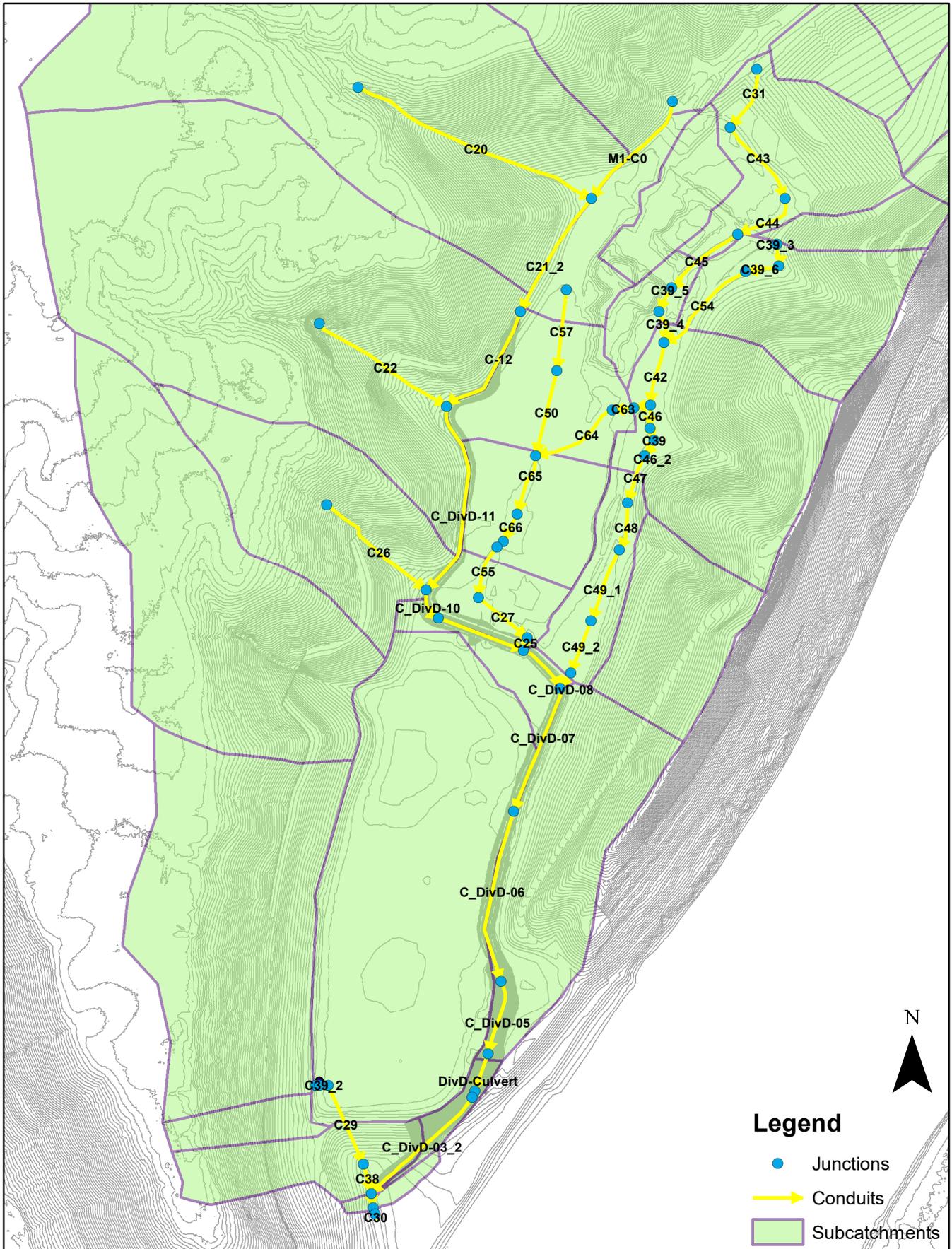
Calculation Performed by: Stantec Consulting Services Inc.

Prepared by: Brenton Newswanger

Reviewed by: Nick Mueller

Revisions:R0

Appendix A - Subbasin Boundaries



APPENDIX I

Final Cover Soil Loss

The Universal Soil Loss Equation (USLE) is:

$$A = (R)(LS)(P)(K)(C)$$

where: A = soil loss in tons/acre/year,
 R = rainfall erosion index,
 LS = slope length and steepness factor,
 P = erosion control practice factor,
 K = soil erodibility factor, and
 C = vegetative cover factor.

Under 329 IAC 10-30-2, the final cover must have a maximum erosion rate of five tons per acre per year. Therefore, $A \leq 5.0$ tons/acre/year.

R = 180 (southeastern Indiana in Figure 1)

LS = 2.6
 for a 1,000-foot distance at 7.0% slope (such as a drainage channel).
 Assuming bare earth prior to riprap/geotextile placement.
 (USDA, AH537, 1981 - attached)

P = 1.00

K = 0.43

C = 0.01

(Indiana State Board of Health, 1986 – memorandum attached)

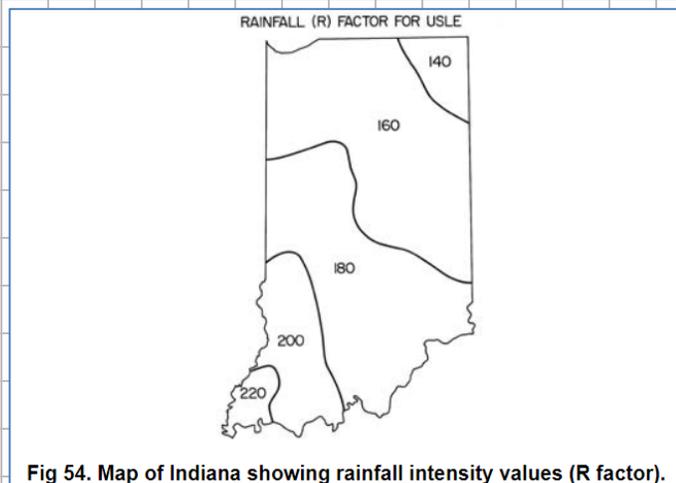


Fig 54. Map of Indiana showing rainfall intensity values (R factor).

1 - https://www.agry.purdue.edu/soils_judging/new_manual/ch6-water.html

$$A = (180)(2.6)(1.00)(0.43)(0.01) = 2.01 \text{ tons/acre/year for a slope length of 1,000 feet at 7.0\%}.$$

For LRCP, the steepest slope (excluding ditch sideslopes) is 7.0% with a maximum flow path of approximately 1,000 feet (less than the above calculation). The longest flow path on the surface is roughly 2,500 feet with a slope of 0.5%. However, these flow paths will be covered with 6" Unimat lining upon construction, limiting soil loss potential. The steepest slope (excluding ditch sideslopes) which will be permanently exposed is 2.0% with a maximum flow path of approximately 50 feet (less than the above calculation). The longest flow path on such a surface is roughly 2,000 feet with a slope of 0.8%.

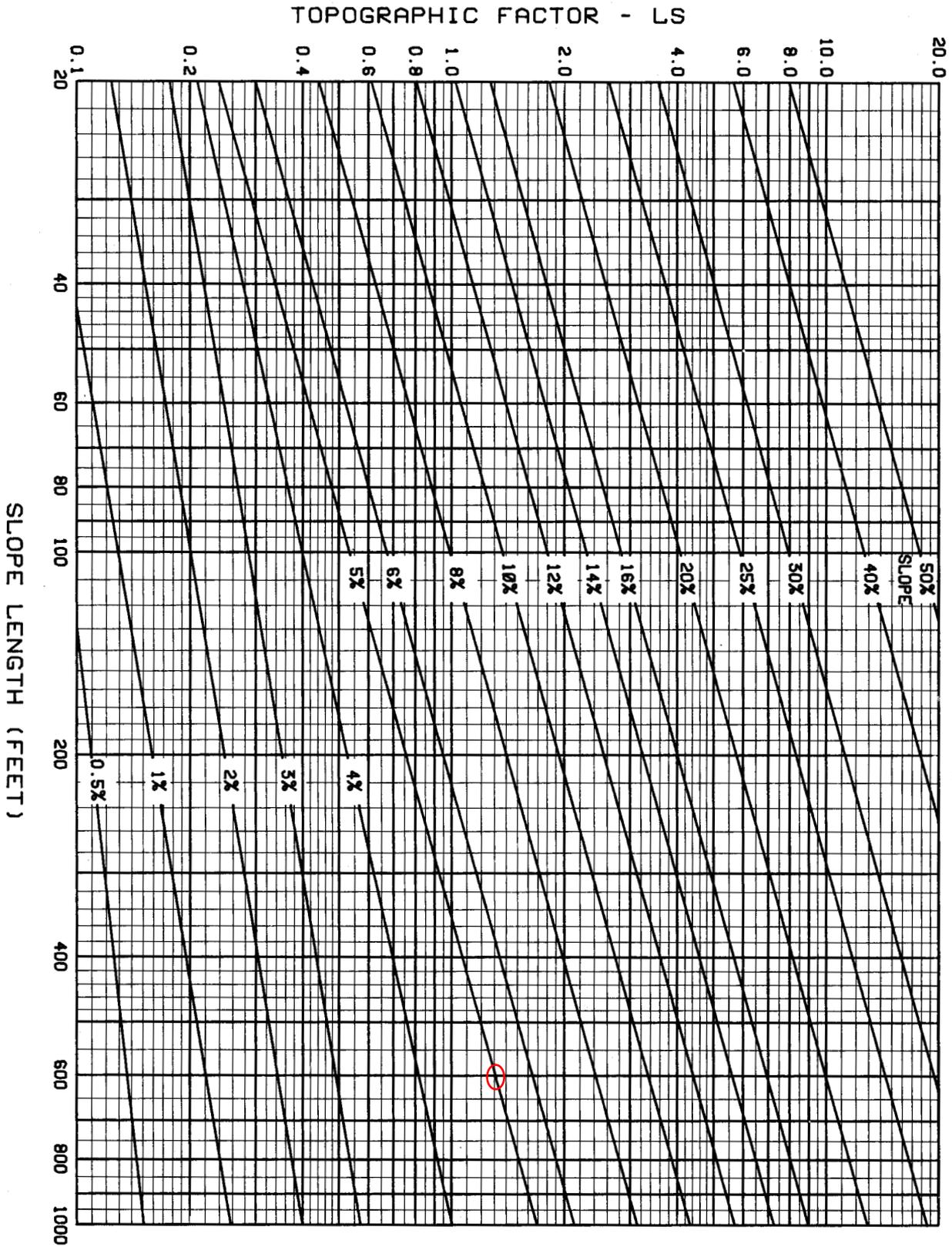


FIGURE 4.—Slope-effect chart (topographic factor, LS). $LS = \frac{(\lambda/72.6)^m}{(65.41 \sin^2 \theta + 4.56 \sin \theta + 0.065)}$ where λ = slope length in feet; θ = angle of slope; and $m = 0.2$ for gradients < 1 percent, 0.3 for 1 to 3 percent slopes, 0.4 for 3.5 to 4.5 percent slopes, and 0.5 for slopes of 5 percent or steeper.

STATE BOARD OF HEALTH

INDIANAPOLIS

OFFICE MEMORANDUM

DATE: January 3, 1986

TO: James E. Traylor
Technical Support Branch

THRU: Bruce Palin

FROM: Duane Leith
Engineering SectionSUBJECT: Guideline for the Evaluation of the
Erosion Potential of Landfill Covers

Sanitary landfill covers should be designed for erosion control in order to avoid later exposure of the refuse and infiltration into the refuse. The design standards which are used can be controlling factors for the size and steepness of a landfill and are therefore often the basis for deciding when a landfill will have to close. It is therefore important to have reasonable and defensible standards. The most recent guidance available to staff in this regard is a memo by Mr. Larry Dunbar dated October 17, 1984, entitled "Evaluation of Design of Final Cover for Landfills." Certain publications recently available to staff and conversations with staff of the Soil Conservation Service have led to the conclusion that the guidance contained herein would be more appropriate than the previously-mentioned memo.

The Governor's Soil Resource Study Commission has proposed goals for erosion and sediment reduction as set forth in their report "Indiana's Erosion and Sedimentation Situation." Briefly, these goals are to reduce erosion on all land to an average annual rate denoted as "T" or the tolerable limit, which prevents depletion of the soil resource and to control all off-site sedimentation by application of best available technology. "T" is the rate at which the soil replaces itself. It is based on factors such as soil depth, texture, and permeability; its value is a matter of judgment rather than being quantifiable. The value of "T" is reported to range between two and five plus tons/acre/year. A value of five tons/acre/year is recommended as a standard under this guideline with an exception as noted.

The currently accepted method for determining the erosion potential of landfill covers is found in Agriculture Handbook 537 "Predicting Rainfall Erosion Losses," which is available from SCS offices. Application of the Universal Soil Loss Equation (USLE) from Agriculture Handbook 537 is conducted as follows:

A = RKLSCP, where:

A is the average soil loss in tons per acre calculated on an annual basis for landfill design.

Mkong
12

R is the rainfall and runoff factor as obtained from the map Figure 1, inserted between pages 6 and 7 of the handbook, copy is attached to this document. This value ranges from 130 to 225 depending on location.

K is the soil erodibility factor as obtained from Figure 3 on page 11 of the handbook (copy attached), for the composition, structure and permeability of the surface soil as replaced over the landfill. This generally ranges from .30 to .50. If the soil source or characteristics are unknown, it has been recommended that a value of 0.43 be used for landfills.¹

LS is the combined topographic factor for the length and slope which can be found from Table 3 on page 12, or the chart on page 13 of the handbook, copies of which are attached. Most landfills will have irregular convex slopes. The LS value for irregular slopes is determined by the procedures specified on page 16 of the handbook. The slope is divided into successive equal length segments of uniform slope. The LS factor is obtained for each segment at its respective slope and at a length equal to the total slope length. This LS factor is then adjusted by the "fraction of soil loss" figures shown under the column M = 0.5 in Table 4 on page 15 of the handbook, copies attached. Alternatively, this figure can be obtained from the equation

$$\text{Soil loss fraction} = \frac{i^{m+1} - (i-1)^{m+1}}{N^{m+1}}$$

where: i = segment sequence number, m = slope length exponent of 0.5 for slopes greater than or equal to five percent. N = the number of equal length segments into which the slope was divided. The sum of the adjusted LS values is the LS value to be used for the entire slope.

C is the cover and management factor which can be found from Table 10 on page 32 of the handbook, or the copy attached. For properly prepared and seeded landfill covers which will be maintained in sod, use a value of 0.01. If a different vegetative cover is planned, adjust the value to reflect the anticipated conditions using Tables 5 through 12 of the handbook. A value of C lower than 0.01 should be allowed only with very intensive specifications regarding cover preparation, fertilization, seeding, and management. In order to establish high productivity, staff of the Soil Conservation Service discussed fertilization rates of 1,000 lbs/acre and contractor prices for fertilization, mulching, and seeding of \$700 to \$1,000 per acre.

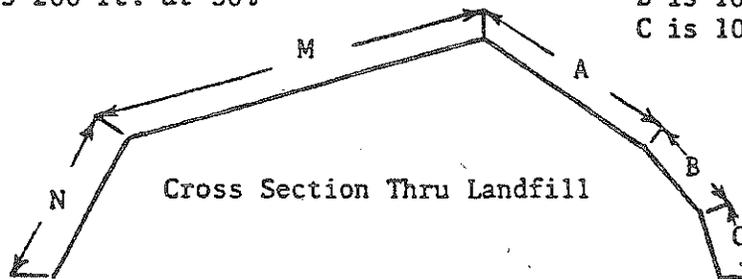
¹ Verbal communication with Mr. Raymond Sinclair of the Soil Conservation Service on October 7, 1985.

P is the supporting practices factor. For landfills, the value of P is 1.00, unless the site is to be used as cropland. Use the figures from Table 13 on page 35 and Table 14 on page 36 of the handbook as required for landfills used for cropland.

Sample calculation:

M is 800 ft. at 6%
N is 200 ft. at 30%

A is 300 ft. at 4%
B is 100 ft. at 10%
C is 100 ft. at 25%



To determine A values for slopes A, B, C, and M, N:

For slopes A, B, and C

1. R = 175, for Marion County location on the rainfall and runoff map.
2. K = 0.43, assumed typical value since actual soil samples have not been tested.
3. LS determination:

Total Slope Length	Segment	Slope	LS Value from Figure 4	Adjustment Factor from Table 4	Revised LS Value
500 ft (5 segments of 100 ft each)	A1	4%	.76	.09	.07
	A2	4%	.76	.16	.12
	A3	4%	.76	.21	.16
	B4	10%	3.06	.25	.77
	C5	25%	13.20	.28	3.70
<u>Total</u>					4.82

4. C = 0.01, from grass sod, well maintained.
5. P = 1.0, since it is not tilled cropland.

Following the USLE: $A = RKLSCP$

$A = 175 \times .43 \times 4.82 \times .01 \times 1.00$, for slope A, B, C

$A = 3.63$ tons per acre

Since A is less than or equal to five tons per acre, this slope is acceptable.

For slopes M and N

1000 ft	M1	6%	2.13	.09	.19
(5	M2	6%	2.13	.16	.34
segments	M3	6%	2.13	.21	.45
of 200 ft	M4	6%	2.13	.25	.53
each)	M5	30%	25.57	.28	7.16
					Total 8.67

A = 175 x .43 x 8.67 x .01 x 1.00, for slope MN

A = 6.52 tons per acre

Since A is greater than five tons per acre, this slope is not acceptable.

It has been suggested that a possible way for a facility to have a cover with an A value higher than five would be to increase the cover depth on the lower slopes. The increased depth can allow for the formation of gullies which can then be stone-lined or similarly stabilized. Whether the increased A value should be allowed and the calculation of the necessary depth increase is not within the scope of this guidance. Erosion control is not the only factor to be considered in cover design. Other factors, such as prevention of ponding, slope stability, drainage, and feasibility of maintenance, will need to be considered.

In conclusion, it is recommended that sanitary landfill covers be designed for an A value not greater than five tons/acre/year, as determined by the Universal Soil Loss Equation from Agriculture Handbook 537. It is further recommended that a K value of 0.43 be used in calculating the A value for typical landfill soils.

References and documents.

1. Predicting Rainfall Erosion Losses, 1978, Agricultural Handbook 537, U.S.D.A.
2. Indiana's Erosion and Sediment Situation, 1984, Governor's Soil Resources Study Commission.
3. Dunbar, Larry, Office Memo to Engineering Staff, October 17, 1984.
4. Design and Construction of Covers for Solid Waste Landfills, 1979, EPA 600/2-79-165, U.S. EPA.

*OK
By [Signature]
Creek Station
5/1/84
100m*

Guideline No. _____

Comment period ends _____.

APPENDIX J

LRCP Quality
Management Plan
(QMP)



**Construction Quality
Management Plan (QMP)**

Landfill Runoff Collection Pond Closure

Clifty Creek Plant
Jefferson County, Madison, Indiana

Issued for Bid – 60% Design

Prepared for:

Indiana-Kentucky Electric Corporation

Prepared by:

Stantec Consulting Services Inc.

May 21, 2021

Abbreviations

CCR	Coal Combustion Residuals
CM	Construction Manager
CQA	Construction Quality Assurance
CQC	Construction Quality Control
EDC	Engineering during Construction
EOR	Engineer of Record
EM	Engineering Manager
FR	Field Representative
FTP	File Transfer Protocol
IKEC	Indiana-Kentucky Electric Corporation
LRCP	Landfill Runoff Collection Pond
LVWTS	Low Volume Waste Treatment System
NCR	Nonconformance Reports
OD	Observed Deficiency
POD	Plan of the Day
QMP	Quality Management Plan
RFI	Request for Information
WBSP	West Boiler Slag Pond

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CONSTRUCTION QUALITY MANAGEMENT PLAN (QMP)

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CONSTRUCTION QUALITY MANAGEMENT PLAN (QMP)

Introduction

1.0 INTRODUCTION

1.1 PURPOSE

The purpose of the Construction Quality Management Plan (QMP) is to promote quality of the constructed work. It consists of three main components, namely, Construction Quality Control (CQC), Construction Quality Assurance (CQA), and Engineering during Construction (EDC). The CQC activities are the Subcontractor's responsibility. The CQA activities are an audit process, performed by the CQA Team, to make sure that the Subcontractor's CQC plan is implemented and on track. EDC activities consist primarily of reviewing and responding to Subcontractor submittals and requests for information (RFIs), and general design support throughout construction.

This Construction QMP provides guidance to the project team and establishes assessment, reporting, and documentation procedures to be implemented throughout the project. Where conflict arises between the requirements of this QMP and the contract documents, the most stringent requirements shall govern.

This QMP describes the CQC and CQA management structure, personnel requirements, and minimum project requirements. This QMP also serves as an outline to develop site-specific protocols based on conditions encountered during the work.

1.2 SCOPE OF WORK

This Construction QMP has been prepared for the Landfill Runoff Collection Pond (LRCP) Pond Closure project.

This project generally consists of the following activities:

- Lowering the pond water surface.
- Constructing a diversion ditch around the perimeter of the pond.
- Grading coal combustion residuals (CCR) to proposed subgrade elevations.
- Installing final cover system over the CCR material.
- Modifying the existing spillway structure.

1.3 CONSTRUCTION QUALITY CONTROL AND QUALITY ASSURANCE

The QMP establishes the requirements for CQC and CQA. It outlines roles, responsibilities, CQC and CQA activities, and establishes project processes and procedures. The CQC/CQA Program structure is shown below as Figure 1-1.

CONSTRUCTION QUALITY MANAGEMENT PLAN (QMP)

Introduction

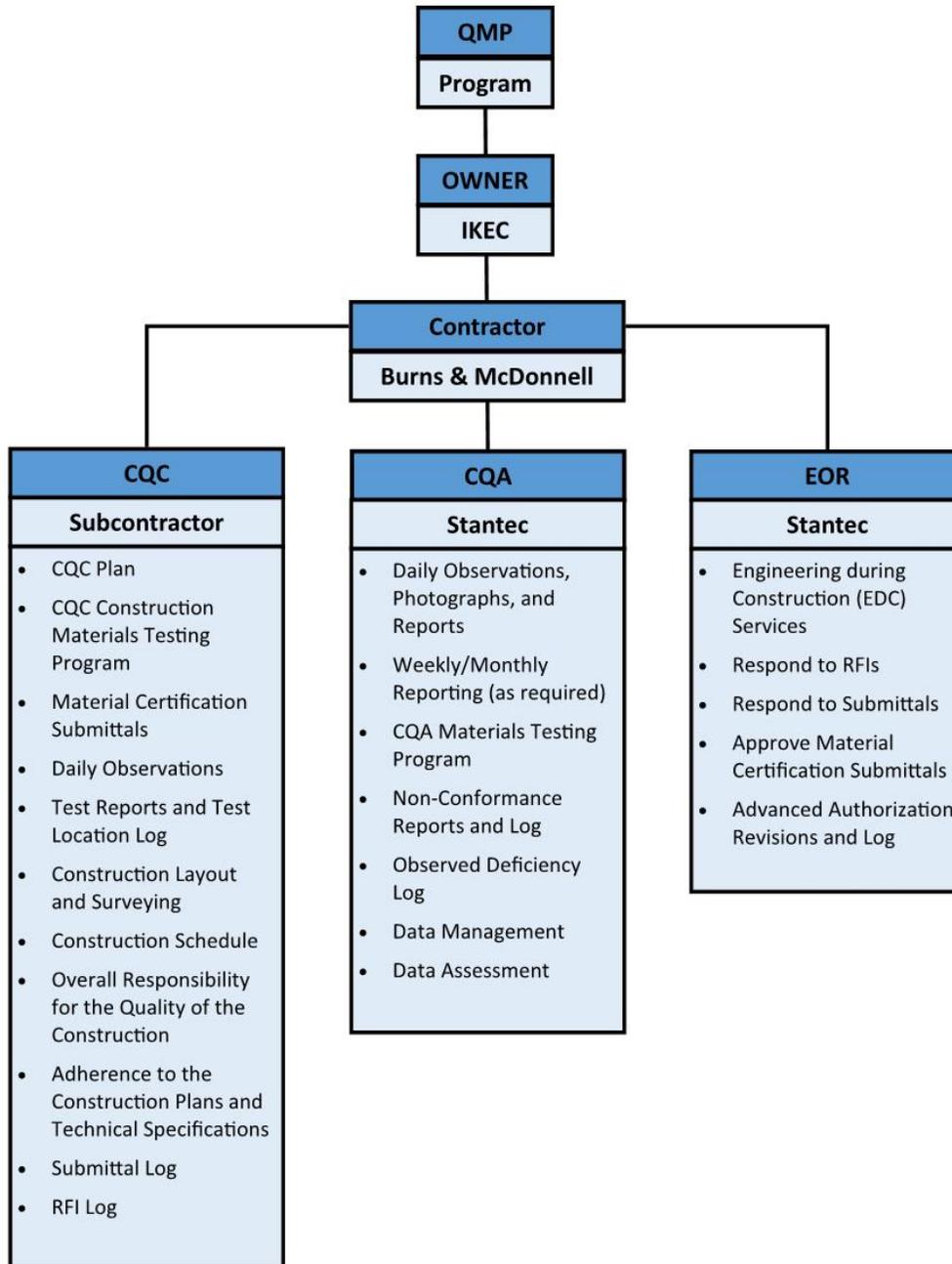


Figure 1-1. General Program Structure for CQC/CQA Program

CONSTRUCTION QUALITY MANAGEMENT PLAN (QMP)

Introduction

1.4 SURVEY REQUIREMENTS

The Subcontractor shall provide construction layout services to execute the work according to the contract documents. Final as-built surveys and CQC surveys will be conducted by the Subcontractor and may be supplemented with surveys conducted by the Owner, Contractor, CQA Manager, or identified representative. As-built surveys shall be performed under the supervision of a professional surveyor licensed in the state of Indiana.

1.5 LIMITATIONS

The QMP does not include any facility elements outside the limits of construction designated on the Plans for Construction.

1.6 WORKING ON ASH

The Subcontractor shall be aware that there are inherent risks associated with working on CCR, including but not limited to, soft bearing conditions and unstable slopes. failures and instabilities may occur on apparently firm surfaces when loaded, or during active drawdown and dewatering. It is the Subcontractor's responsibility to actively manage their equipment and personnel to safely execute the work. the Subcontractor shall provide an access and excavation work plan that outlines their means and methods for working on CCR surfaces. The plan shall include sequencing of work and note the equipment and materials used.

Subcontractor is responsible for maintaining the stability of the CCR surfaces and perimeter dikes during construction. Subcontractor shall develop a monitoring plan detailing the type of monitoring to be performed (visual, installation of geotechnical instrumentation, or other methods). The plan shall also include, at a minimum, the potential for sloughing or instability of the perimeter dikes due to groundwater levels, how the risks will be mitigated, and how progressive failures along the perimeter dikes will be reduced and mitigated. the monitoring plan shall be submitted to the owner and their representative for review and approval.

The Subcontractor is responsible for all site safety and near surface ash stability at the site and shall follow all OSHA, Contractor, and owner safety requirements including but not limited to man on the ground exclusion zones. Furthermore, the Subcontractor shall provide to the owner a slope stability analysis prepared by a Registered Engineer in the State of Indiana for all excavations 10-feet or greater located outside of the exclusion zones or as directed by the Contractor. All evaluated excavations will require a minimum factor of safety of 1.3. The Subcontractor shall also prepare and provide to the owner for review a plan outlining all exclusion zones to include, but not limited to, barriers, setbacks, signage, and guidelines for entry.

CONSTRUCTION QUALITY MANAGEMENT PLAN (QMP)

Organization and Responsibilities

2.0 ORGANIZATION AND RESPONSIBILITIES

2.1 PROGRAM ROLES AND RESPONSIBILITIES

The major participants in the project are listed below along with a description of their roles. An organizational chart is provided in Figure 1-1.

2.1.1 Owner

The plant and its ancillary functions are owned and operated by the Indiana-Kentucky Electric Corporation (IKEC). The Owner has contracted with the Contractor to serve as the Construction Manager (CM) who shall approve any design and/or QMP revisions. The Owner will administer communication with any regulatory agencies, including any related permit modifications.

2.1.1.1 Construction Manager (CM)

The Construction Manager (CM) is responsible for coordinating with the Subcontractor regarding contractual issues, including scope, budget, and schedule. In addition, the CM will provide daily oversight of the construction activities. The CM has the authority to stop work that is not in conformance with the Plans for Construction and Technical Specifications. The CM is responsible for tracking submittals and RFIs. The CM will then distribute the submittals and RFIs, as needed, to the appropriate party.

In addition, the CM or qualified representative shall perform at least monthly inspections of the LRCP facility. These inspections shall include observations of all outcrops for indications of slope instability including tension cracks, sloughs, and excessive seepage. These inspections shall be documented and retained within the project records. Any suspect site conditions shall be promptly reported to the Owner.

2.1.2 Engineer of Record (EOR)

The Engineer of Record (EOR) is responsible for development of the technical components of the project. During field implementation, the Engineer will provide support to the CQA Team for construction observation and CQA services.

Specific examples of Engineer responsibilities include responding to technical RFIs and submittals, supporting the CQA team, and review of field-testing data.

2.1.3 CQA Manager

The Construction Quality Assurance (CQA) Manager shall be responsible for the execution of the CQA program and related documentation as outlined in the QMP for all work performed. The CQA Manager shall be a Professional Engineer licensed in the state of Indiana.

2.1.4 Contractor

The Prime Contractor (Contractor) holds the Engineering, Procurement, and Construction contract directly with the Owner. The Contractor is responsible for coordinating the design, procurement, and construction work to complete the project. The Subcontractor (CQC Team), CQA Team and Engineer are contracted with the Prime Contractor for execution of the project.

CONSTRUCTION QUALITY MANAGEMENT PLAN (QMP)

Quality Assurance

2.1.5 Subcontractor

The Subcontractor is responsible for execution of the work in accordance with the contract documents. The Subcontractor is solely responsible for field implementation activities, including instrumentation and monitoring. The Subcontractor will collaborate with the CM to propose adjustments (if needed) to the scope of work depending on encountered conditions. Review and approval of scope of work adjustments shall be made by the Engineer.

Specific examples of Subcontractor responsibilities include, but are not limited to, data collection and providing that data to the Owner; development and implementation of the CQC Plan, including development of applicable procedures, processes, and work plans; reporting in accordance with the Technical Specifications; CQC materials testing; quantity tracking; conformance to the Technical Specifications; and structure and ground movement monitoring data collection and reporting.

2.1.5.1 Construction Quality Control Manager

The Subcontractor's CQC Manager shall have a minimum of five years of construction experience and be a full-time on-site employee of the Subcontractor. The Subcontractor CQC Manager is responsible for the Subcontractor's tests, inspections, processes, and related actions during and after construction execution to evaluate that both the actual products used, and the completed construction comply with the requirements of the Plans for Construction and Technical Specifications. The CQC Manager shall report to the Subcontractor's principal officers and the CM.

2.2 STOP WORK AUTHORITY

The EOR, CQA Manager, CM, and CQC Manager may exercise stop work authority when concerns related to quality are identified. In situations where personnel safety is concerned, any project personnel may stop work at any time.

3.0 QUALITY ASSURANCE

Construction Quality Assurance (CQA) is the responsibility of the Contractor, Engineer, and CQA Team. CQA includes assessments, observations, and reporting to document that the implementation of the work performed by the Subcontractor meets the requirements of the Plans for Construction and Technical Specifications. The CQA Team functions as the field representative for the Engineer through performance of assessments, verifications, and observations. Specific CQA responsibilities are listed as follows:

- Daily field report of construction activities including photographs.
- Noting observed deficiencies (ODs) during construction that require correction.
- Reviewing applicable Subcontractor submittals and Requests for Information (RFIs) related to quality for adherence to project requirements.
- Reviewing test data for compliance with project requirements and specifications.
- Performing assessments of the Subcontractor CQC Plan to ensure adherence to the QMP.
- Reviewing the Subcontractor documentation deliverables for conformance with project requirements.

CONSTRUCTION QUALITY MANAGEMENT PLAN (QMP)

Quality Assurance

- Periodic material testing to audit CQC test results.
- Reporting of test results.
- Special testing requested by the Owner.

3.1 QUALITY ASSURANCE TEAM

3.1.1 Roles and Responsibilities

The CQA Team serves as observers of field implementation of the construction documents and to provide CQA documentation. The CQA Team is responsible for the execution of the CQA Plan and related documentation as outlined herein. Individual roles and responsibilities for the CQA Team members are defined below. Personnel assignments are subject to change, if qualification requirements are met and approved by IKEC.

3.1.1.1 Construction Quality Assurance (CQA) Manager

A professional engineer licensed in the State of Indiana shall be designated as the CQA Manager. The CQA Manager shall be responsible for administering the CQA program and advising the CQA Team.

The CQA Manager will be responsible for the following tasks:

- Observing conformance with the QMP by reviewing and documenting project records and activities;
- Managing overall implementation of the CQA program;
- Evaluating the testing results of the CQC program;
- Evaluating work for conformance with the project plans and specifications and notifying the CM if work is non-compliant with the contract documents;
- Managing the documentation of all CQA activities;
- Reviewing progress of the work and reports prepared by the Subcontractor as part of the CQC Plan;
- Verify the appropriate test standards are used for the methods to conduct assessments and field and laboratory testing for CQA testing;
- Evaluating and auditing the results of CQC and CQA assessments and testing; and
- Review daily field reports prior to submittal to the CM and EOR.

3.1.1.2 Construction Quality Assurance (CQA) Supervisor

The Construction Quality Assurance (CQA) Supervisor provides oversight for the CQA Field Representative(s) on site. The CQA Supervisor is familiar with the materials to be used, the observations and testing to be done, and the functional intent of the QMP. The CQA Supervisor has responsibility for:

CONSTRUCTION QUALITY MANAGEMENT PLAN (QMP)

Quality Assurance

- Coordination of the periodic CQA construction testing in the field;
- Coordination of other testing with a commercial laboratory (as needed);
- Provides support to field CQA staff;
- Plans and directs the activities of CQA field representative(s); and
- Reviews Daily Field Reports.

In conjunction with their staff, the CQA Supervisor reviews daily field reports and directives, and reports to the CQA Manager any situation where the Plans for Construction and Technical Specifications do not appear to be appropriate for the conditions encountered.

The CQA Supervisor reviews Nonconformance Reports and has authority to stop work due to adverse quality conditions or potentially unsafe work practices. The CQA Supervisor reports to the CQA Manager. The CQA Supervisor will be on-site an average of one day per month for the duration of construction, or as necessary to observe key construction activities to support the CQA team.

3.1.1.3 Construction Quality Assurance (CQA) Field Representative (FR)

The Construction Quality Assurance Field Representative (CQA FR) staff shall consist of qualified personnel working under the direct supervision of the CQA Manager and CQA Supervisor. The CQA Supervisor will be responsible for the day-to-day coordination and management of the CQA FRs.

The CQA FR is responsible for performing quality assurance in the field, and for performing observations of conformance with the Plans for Construction and Technical Specifications. The CQA FR will document the results of the required CQA observations and testing and inform responsible personnel about unsatisfactory items. The CQA FR is also responsible for documenting that corrective actions are taken to resolve the conditions. For defective work, the site-specific CQA FR will initiate a Nonconformance Report and submit the report to the CQA Supervisor.

The duties of the CQA FR are listed below:

- Daily observations of construction activities to verify conformance with project Plans for Construction and Technical Specifications;
- Observe on-site testing performed by CQC team members;
- Perform periodic on-site CQA testing to verify CQC procedures and test results;
- Coordinate required sampling with commercial laboratory for other quality control testing (as needed);
- Prepare and submit daily field report of observations, testing results, and photographs;
- Conduct periodic inspection of specific construction items;
- Instrumentation monitoring (as needed);
- Verification that testing is performed and that results meet the Technical Specifications;

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- Verification of Subcontractor's CQC surveying;
- Reporting of nonconformances; and
- Reporting of observed deficiencies.

3.1.2 Materials Testing

The CQA Team will perform selected CQA sampling and material testing to audit CQC procedures and test results. The material testing schedule is included in Attachment A. The schedule specifies the anticipated types and minimum number of tests for each material subject to testing and required frequency of testing. The CQA Manager or identified representative is responsible for reviewing the material testing and results and manufacturer's supplied information for conformance to the Technical Specifications.

3.2 QUALITY ASSURANCE DAILY FIELD OBSERVATIONS

3.2.1 General

The CQA Team shall review the Plans for Construction and Technical Specifications for each day's construction activities. After observation of the day's activities, they shall document whether the work that was observed has been done in accordance with the Plans for Construction and Technical Specifications. The CQA Team's observation of work serves as an audit function. It is not to be considered a verification that all work performed was in accordance with the Plans for Construction and Technical Specifications. That responsibility remains with the CQC Team.

Daily observations are to be documented in a CQA daily field report.

Any suspect conditions shall be promptly reported to the CM. Each observation shall be documented on the Daily Field Report form for inclusion with the project records.

3.2.2 Conformance Verification

Conformance verification shall consist of observing and documenting testing performed by the Subcontractor to ensure that the required tests and evaluation of materials and construction products are performed. The CQA Team shall confirm that testing is performed at frequencies specified in this QMP. The CQA Team shall perform periodic testing to verify the results of the Subcontractor. Additional or supplementary conformance testing may be added at the discretion of the CQA Manager. Results shall be reviewed by the CQA Manager to assess conformance with project requirements. Copies of all conformance results shall be included on a CQC testing log and with the project records.

The NCR process shall be used, as needed, to immediately report deficiencies, remediation required, and resolution to the Contractor and Engineer. NCRs may be submitted to the CQA Manager by the CQA Team or Subcontractor. The CM will maintain a log as a record of the non-conformances encountered and the final resolution. A detailed description of the NCR process is provided in Subsection 5.2.5.

4.0 QUALITY CONTROL

Construction Quality Control (CQC) and overall construction/material quality is the responsibility of the Subcontractor. CQC includes establishing procedures and work plans, performing observations,

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documenting construction processes and performance, and performing materials testing to demonstrate the quality of the constructed elements. Specific CQC items are as follows:

- Ensuring that the Subcontractor's work complies in all respects to the Plans for Construction, Technical Specifications, other contract documents, and any approved changes to the contract documents.
- Developing the CQC Plan.
- Developing work plans, procedures, and submittals related to the work.
- Preparing a daily log of observations and activities.
- Providing data collected during this project to the Contractor.
- Demonstrating the means and methods for complying with the Plans for Construction and the Technical Specifications.
- Performing construction staking and layout.
- Performing construction materials quality control testing and reporting.
- Maintaining an updated construction schedule with CQC/CQA hold point milestones represented.
- Maintaining calibrations on measuring and testing equipment.
- Providing final as-built surveys and drawings.
- Testing logs, timely submittal of required deliverables, reporting nonconforming conditions to the Engineer, and data management.

4.1 SURVEY REQUIREMENTS

The Subcontractor shall provide field layout services for the purposes of executing the work according to the contract documents. The work shall be laid out and constructed to the elevations shown in the Plans for Construction and in accordance with the Technical Specifications. Tolerances shall be as defined in the Technical Specifications.

Final as-built surveys and CQC surveys will be conducted by the Subcontractor and may be supplemented with surveys conducted by the Contractor or identified representative.

4.2 CQC EXECUTION AND PLAN PREPARATION

The Subcontractor shall prepare a CQC Plan meeting the requirements of this QMP and the project specifications. The CQC Plan must be approved by the Contractor, CM, and Engineer prior to the start of construction. The Subcontractor shall execute CQC activities in accordance with the approved CQC Plan.

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4.3 CQC PLAN REQUIREMENTS

The Subcontractor is responsible for establishing and maintaining a CQC Plan for the project to ensure that the project is executed, and items are installed in accordance with the Plans for Construction and Technical Specifications. The objective of the CQC Plan is to provide a framework where a quality product will be produced. The details of the CQC system will be described in the CQC Plan document, which will establish procedures to ensure uniformity and provide a standard by which comparisons can be made.

The CQC Plan shall include, at a minimum, the following to cover all operations, both on-site and off-site, including work by subcontractors and suppliers:

- Organizational Structure: Chart showing the CQC organizational structure, including line of authority.
- Personnel: Names and qualifications, in resume format, for each person in the QC organization.
- Duties, Responsibilities, and Authorities: Duties, responsibilities, and authorities of each person in the QC organization.
- Outside Organizations: List of outside organizations, such as consulting engineering firms, that will be employed by the Subcontractor and a description of services these firms will provide, including decision-making authority (if any).
- Scope of Work: Scope, including testing laboratory information and accreditations and materials testing schedule.
- Submittals: Procedures for reviewing, approving, and managing submittals. Include the name(s) of the person(s) in the QC organization authorized to prepare required submittals, and the initial submittal of the submittal register as specified in the section entitled "Submittal Procedures."
- Completing Rework Items: Procedures for addressing nonconformance, deficient, and rework items.
- Measuring and Testing Equipment: Copies of current certifications for monitoring and testing equipment.
- Documentation Procedures: Documentation procedures, including proposed report/forms formats.
- Training Requirements: Documentation of personnel trained in specifics of the CQC Plan.
- Work Plans and Quality Process Documents.

4.4 TESTING SCHEDULE

It is the responsibility of the Subcontractor to perform tests specified in the Technical Specifications and verify that control measures are adequate to provide a product that conforms to the requirements of the project documents. A material testing schedule outlining minimum CQC and CQA testing frequencies is included in Attachment A. The schedule specifies the anticipated types and minimum number of tests for each material subject to testing. The CQC Manager or identified representative is responsible for verifying

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that material testing and results conform to the Technical Specifications. The Subcontractor shall maintain a log of all CQC material test results.

4.5 SUBMITTALS AND REQUEST FOR INFORMATION

4.5.1 Submittals

Subcontractor submittals shall be provided to the CM, consistent with the requirements of the Technical Specifications. For material submittals, the Subcontractor shall review and certify that the material conforms to the Plans for Construction and Technical Specifications prior to submittal to the CM.

4.5.2 Request for Information

The Subcontractor shall communicate issues such as constructability, discrepancies in the plans, and requests for Engineer support during field implementation, etc., using the RFI form. RFIs shall be submitted by the Subcontractor to the CM. The CM routes the RFI to the Engineer. The Engineer will prepare a response to the RFI and submit it to the CM. The CM then sends the completed RFI response to the Subcontractor.

The CM, or representative, shall document each RFI in an RFI log and in the project records. The RFI log will be maintained by the CM, or representative, and will be reviewed at the weekly project progress meetings. The Subcontractor is encouraged to engage the Engineer and the Engineering Team prior to RFI submittal in efforts to streamline the RFI process. The Engineer has seven calendar days upon receipt of the RFI to provide a response to the CM. If a quicker response is required by the Subcontractor, this should be noted in the RFI and in the correspondence to the CM.

Any changes to the project that result from the RFI process shall be documented and communicated to the CQA Team and the Subcontractor. Communications shall include discussion of the issue that led to the RFI, the intent of the RFI response, and any resulting changes to the project.

It should be noted that, because of the nature of the project, the RFI process may not be suitable for some of the day-to-day adjustments that will be necessary. Instead, daily collaboration between the CM, Engineer, and Subcontractor field staff will be essential to successfully meet the intent of the project. Such daily adjustments shall be documented in both the CQC daily field report and the CQA daily field report.

4.6 MEETINGS

4.6.1 Orientation Meeting

An orientation meeting (i.e., kickoff meeting) shall be held before field implementation of the QMP. At a minimum, those present will include the CM, the Contractor, the Subcontractor's PM, the CQC Manager, the CQA Manager, CQA FR, IDEM, and others as needed. This meeting will include a review of the project document objectives, quality management processes, hold points, and special project requirements. The Subcontractor will prepare meeting minutes and distribute them for review. The Owner shall notify the IDEM permit manager 10 working days prior to the meeting.

Other kickoff meetings may be required before the start of discrete phases of the work.

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4.6.2 Daily Meetings

Daily plan-of-the-day (POD) meetings related to safety and construction activities for the day's work shall be conducted by the CM or designated representative. Attendees shall include the CQA FR and the CQC Manager. Other key participants from IKEC, the Engineer, and the Subcontractor will be included in these meetings as appropriate.

4.6.3 Weekly Meetings

The CM will hold weekly on-site meetings with the project team during active construction. Portions of these meetings will be allotted to discuss quality and engineering. Those present shall include the CM; CQA FR; and the CQC Manager and on-site representative. Other key participants from IKEC, the Engineer, Contractor, and the Subcontractor will be included in these meetings as appropriate.

The primary purpose of the weekly meetings shall be to confirm that all parties involved with field activities are familiar with the design, required procedures, and associated quality objectives, along with any issues (e.g., safety, environmental) related to field implementation. Topics to be addressed at this meeting shall include a review of the schedule, any outstanding RFIs, outstanding change orders, status of Subcontractor submittals, and quality issues. The CM, or representative, shall provide minutes of each meeting for inclusion in the project records.

4.6.4 Additional Meetings

Other on-site meetings will be organized to address site-specific issues that need quick resolution but are not conducive to the weekly or other regularly scheduled meetings or that require specific personnel to be present and to work through specific issues as they arise. Such meetings will be documented on the CQC and CQA daily field reports.

5.0 QUALITY MANAGEMENT DOCUMENTATION

5.1 PROJECT DOCUMENTATION

Project CQA documentation shall be obtained and maintained by the CQA Manager and copied to the Engineer during all phases of field implementation. Project CQC documentation shall be obtained and maintained by the CM or identified representative and copied to the Engineer. The Subcontractor is required to submit all data collected, both raw and processed, to the Contractor. Transfer of these data should be through Procore or similar file management platform.

Distribution of the project documentation shall be in accordance with Table 5-1.

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Table 5-1. Reporting Responsibility

Item	Originator	Primary Recipients	Secondary Recipients
Daily CQA Report	CQA FR	CQA Supervisor	CM, EOR, CQA Manager, Subcontractor
Daily CQC Report	CQC Team	CM	CQA Manager, EOR
Request for Information (RFI): Submittal	Subcontractor	CM	EOR CQA Manager
Request for Information (RFI): Approval and RFI Log	CM	EOR	CQA Supervisor CQA Manager
Observed Deficiency Log	CQA FR, CQA Supervisor	CM, CQA Manager	Subcontractor
Nonconformance Reports and Log	CQA Supervisor, CQA Manager	CM, EOR	Subcontractor
Subcontractor Submittals	Subcontractor	CM	EOR, CQA Supervisor
Subcontractor Submittal Approval and Log	Subcontractor	CM	EOR, CQA Manager
CQC Testing Results and Log	CQC Team	CM	CQA Manager, EOR
CQC Correspondence and Log	Subcontractor	CM	CQA Manager, EOR
Design Revisions and Log	EOR	CQA Manager	Subcontractor, CM
Subcontractor Daily Production Report	Subcontractor	CM	EOR
Weekly Meeting Minutes	CM	All Present	N/A
Construction Certification Report	CQA Manager	EOR	CM

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5.2 CQA DOCUMENTATION

The CQA documentation shall include, but is not limited to, the following:

- Daily field report;
- RFI responses;
- ODs;
- NCRs;
- CQA correspondence (i.e., memos, letters);
- Photographic documentation; and
- Plans for Construction and Technical Specifications revisions and log.

5.2.1 CQA Daily Field Report

The CQA Team shall maintain daily field reports to document daily observations, investigations, and analyses of the construction, as well as to document the progress of the work. These reports shall include photographic documentation, where applicable. The CM will provide daily tracking of quantities, consistent with the Subcontractor pay items (hard quantities, standby hours tracked separately by CM). This daily tracking will allow independent assessment of the Subcontractor quantities in the team's daily field report.

5.2.2 Photographs

The CQA Team shall maintain a photographic record of the field implementation, documenting the progress of project construction. For the purpose of construction, photographs will be taken as needed to document processes, procedures, and any deficiencies or nonconformance, and will provide a photographic record for inclusion in the project record. Each photograph shall have the date recorded, the name of the person taking the photo (if by someone other than the daily field report author), and the location of photographs. This information shall be noted in the daily field report or the photographic log.

5.2.3 Material Testing Reports

CQA material testing reports will be compiled and distributed to the project team. Daily summaries and off-site test CQA results will be included in the CQA Daily Field Report. Reports shall include CQA testing results for both laboratory and field testing.

5.2.4 Observed Deficiencies (ODs)

Observed Deficiencies (ODs) shall identify and document deficiencies in quality, workmanship, materials, equipment, or supplies and unauthorized deviations from Plans for Construction or the Technical Specifications. The OD log is used to track and rectify deficient events that do not need to be escalated to the NCR level. The following procedures shall be used to document ODs:

- When the Subcontractor, Engineer, or CM notices an observed deficiency, the CQA FR records it in the corresponding CQA daily report.

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- The CQA Supervisor will enter the OD into the Observations tool in Procore along with the following information: referenced daily report number, observed date, deficiency description, reference to the corresponding requirement (if applicable), hold point (if applicable), and responsible party.
- The CQA Team, CM, and the Subcontractor will work to establish a resolution and corresponding timeframe for the OD.
- Once a corrective action for the OD has been agreed upon by all involved parties, it will be implemented by the responsible party (if necessary).
- After the OD is verified to be resolved, the CQA FR will record it in the corresponding CQA daily report.
- The CQA Supervisor will enter the following information into the Observations tool in Procore: referenced daily report number, the resolved date, and corrective action description.
- The Observations tool in Procore will be maintained by the CQA Supervisor for tracking the status of OD events.

5.2.5 Nonconformance Reports

Nonconformance Reports (NCRs) shall identify, report, and document a nonconforming event in quality, workmanship, materials, equipment, or supplies and unauthorized deviations from Plans for Construction or the Technical Specifications. The following procedures shall be used to report nonconformance:

- When a nonconformance is observed, whether by the Engineer, Subcontractor, or CM, the CQA FR prepares an NCR that describes deficiencies noted (including time, actions, locations, etc.) and references to the corresponding requirements. Additionally, a list of corrective actions is identified for the Subcontractor to complete, or meet, to the satisfaction of the Engineer.
- The NCR form is then routed to the Engineer for assessment, with a disposition on the installed work/materials that need to be replaced, if any. The form is then routed to the CM for finalization.
- The CM then routes the NCR package to the Engineer for final review and signature and returns it to the CM, if acceptable. Once that process is complete, the CM issues the NCR to the Subcontractor.
- The Subcontractor provides a list of proposed corrective actions on the form and returns it to the CM for processing and issuance to the CQA Manager. If the proposed corrective action is acceptable, the CQA Manager signs the NCR and includes a list of required documentation to be provided by the Subcontractor to confirm completion of the corrective action. The NCR is then returned to the CM for processing and issued to the Engineer for final approval and signature. The form is then routed to the CM for issuance to the Subcontractor for implementation. If the proposed corrective action is not deemed acceptable, the CQA Manager returns the form to the CM with additional comments for issuance to the Subcontractor for review and revision.
- The process is repeated until the Subcontractor's proposed corrective actions are acceptable to the CQA Manager.
- On completion of the rework identified as deficient or nonconforming, the CQA Manager will conduct a reassessment of the items noted in the NCR. If the reworked items are found acceptable,

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it will be so noted on the NCR. If, however, the items are still not acceptable to the CQA Manager, the items will be rejected and must be reworked before it is resubmitted for further assessment.

- The CM and the CQA Manager will periodically review the status of NCRs and will work with the Subcontractor to establish a timetable for the final resolution of all deficiencies.
- A Nonconformance Log will be maintained by the CM for tracking the status of nonconforming items.

5.3 PLANS AND TECHNICAL SPECIFICATIONS REVISIONS

Periodically during construction, changes to the Plans for Construction and/or Technical Specifications may be required. These changes will be reviewed and drafted by the Engineer and approved by the Engineer prior to field implementation. Any deviations and changes to the Plans for Construction and Specifications require review and concurrence from the Contractor. The revisions will be included on a revision log and in the project records.

5.4 CQC DOCUMENTATION

The CQC documentation shall include, but not be limited to, the following:

- CQC Daily Field Report;
- Field Observation Logs and Test Data Sheets;
- Subcontractor Submittals and Shop Drawings;
- Material Conformance Test Results;
- Construction Problem and Solution Reports;
- Photographic Documentation;
- Design and/or Specification Modifications; and
- Meeting Minutes.

5.4.1 CQC Daily Report

The CQC report shall include a summary of work performed for the day; CQC tests performed; test results; and a “remarks” section that will contain pertinent information, including significant observations, problems encountered during field implementation, and delays encountered. The following CQC data is to be provided in the report:

- Date.
- Weather.
- Quantities of material received on-site with corresponding delivery tickets.

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- Quantities of materials used.
- List of CQC tests performed.
- Remarks section outlining any issues, delays, etc., that were encountered during the day.
- Updated drawings, mapping, and graphical representations of field work.
- Quantities of materials and/or debris to be hauled off-site and disposed of at locations not owned, operated, or maintained by the Owner, including proper chain of custody, required haul tickets, and scale tickets.
- Daily hold points.
- Photographs of daily construction activities.

In addition to this reporting, refer to the Technical Specifications for more detailed requirements.

ATTACHMENT A

Material Testing Schedule

**LRCP Closure
Quality Management Plan**

Materials Testing Schedule

MATERIAL	PROPERTY	TEST	VALUE	QC FREQUENCY	QA FREQUENCY
Concrete Cast-in-Place Concrete	Compressive Strength	ASTM C39	4,500 psi	1 / 50 CY placed or 1 / day whichever is more frequent	1 / 200 CY placed. Also review QC laboratory reports
	Entrained Air Content	ASTM C231	2.5 - 6.0 %	1 / delivery	Observe Only
	Slump	ASTM C143	3 ± 1	1 / delivery	Observe Only
CCR Fill	Standard Proctor	ASTM D698	laboratory test	1 / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Min. 85%, within 2% of optimum moisture content	5 / acre per lift	1 / acre per lift
Flexible Membrane Liner (FML) 40 mil LLDPE Geomembrane	Large scale direct shear testing	ASTM D5321	laboratory test	1 test per material interface	review laboratory reports provided by CQC Manager
	Seam Properties - Shear Strength	ASTM D6392	Fusion - 60 lbs/in , Extrusion - 60 lbs/in	Cut 1 sample / 500 linear feet of weld	1 / destructive test sample provided by CQC Manager
	Seam Properties - Peel Strength	ASTM D6392	Fusion - 50 lbs/in , Extrusion - 44 lbs/in	Cut 1 sample / 500 linear feet of weld	1 / destructive test sample provided by CQC Manager
	Trial Welds	ASTM D6392	Fusion: 50 lbs/in peel; 60 lbs/in shear Extrusion: 44 lbs/in peel; 60 lbs/in shear	2 / operator / machine / day (morning and mid-shift, max 4 hr work intervals)	Observation Only
	Vacuum Testing	ASTM D5641	Standard Practice for Geomembrane Seam Evaluation by Vacuum Chamber.	1 / extrusion weld and repair location	Observation Only
	Air Pressure Testing	ASTM D5820	Standard Practice for Pressurized Air Channel Evaluation of Dual Seamed Geomembranes	1 / wedge weld and repair location	Observation Only

**LRCP Closure
Quality Management Plan**

Materials Testing Schedule

MATERIAL	PROPERTY	TEST	VALUE	QC FREQUENCY	QA FREQUENCY
Soils Cover Soil	Soil Classification	ASTM D2487	CL, CH, MH, or ML, CL-ML, SC, or SM-SC according to the Unified Soil Classification System, or a combination of these groups	1 / source / change in material	Review laboratory reports
	Standard Proctor	ASTM D698	laboratory test	1 / source / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Min. 92%, within 2% of optimum moisture content	5 / acre / lift	1 / acre / lift
Vegetative Cover	Soil Classification	ASTM D5268	Soils used for vegetative cover shall conform to the requirements set forth in ASTM D5268, unless otherwise approved based on the soils ability to sustain vegetation	1 / source / change in material	Review laboratory reports
	Agronomic Testing	-	laboratory test	1 / 10 acres	Review laboratory reports
Clay Berm Soil	Soil Classification	ASTM D2487	CL, CH, MH, or ML, CL-ML, SC, or SM-SC according to the Unified Soil Classification System, or a combination of these groups	1 / source / change in material	Review laboratory reports
	Standard Proctor	ASTM D698	laboratory test	1 / source / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Min. 95%, within 2% of optimum moisture content	5 / acre / lift	1 / acre / lift
Anchor Trench Backfill	Standard Proctor	ASTM D698	laboratory test	1 / source / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Compact each layer of material with at least three passes of a vibratory plate compactor with in-place moisture within -2% to +2% of optimum moisture content	1 / 100 linear feet of trench	1 / 500 linear feet of trench

**LRCP Closure
Quality Management Plan**

Materials Testing Schedule

MATERIAL	PROPERTY	TEST	VALUE	QC FREQUENCY	QA FREQUENCY
Utility/Pipe Trench Backfill	Standard Proctor	ASTM D698	laboratory test	1 / source / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Min. 95%, within 2% of optimum moisture content	1 / 100 linear feet of trench	1 / 500 linear feet of trench
HDPE Pipe					
HDPE Gravity Pipe	Low Pressure Air Test	ASTM F1417	Standard Practice for Installation Acceptance of Plastic Non-pressure Sewer Lines Using Low-Pressure Air	1 / pipe run	Observation Only
Materials Delivered to Site	Defects	Visual Observation	no defects	1 / material delivery	1 / material delivery
	Conformance to Submittals	Visual Observation	conforms to plans	1 / material delivery	1 / material delivery

Notes:

(1) Testing frequency may be adjusted as directed by the CQA Manager

(2) This table does not include all required quality control testing. The Subcontractor shall be solely responsible for the proper implementation of its Quality Control Program.

APPENDIX K

Post Closure Plan

Post-closure Plan

CFR 257.104(d)

Landfill Runoff Collection Pond

Clifty Creek Station

Madison, Indiana

April 2021

Prepared by: Indiana-Kentucky Electric Corporation

3932 U.S. Route 23

Piketon, OH 45661



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1.0 OBJECTIVE

This report has been prepared to fulfill the requirements of 40 CFR 257.102(b) of the Coal Combustion Residuals (CCR) Rule to develop a Closure Plan for the Clifty Creek Station's Landfill Runoff Collection Pond (LRCP).

2.0 DESCRIPTION OF THE CCR UNIT

The Clifty Creek Station is located on the shore of the Ohio River near Madison, Indiana and consists of six coal-fired electric generating units; each nominally rated at 217 megawatts, that began producing electricity in 1955 to support the Department of Energy's (DOE's) Portsmouth Gaseous Diffusion Plant located near Piketon, Ohio.

The LRCP is an inactive surface impoundment located southwest of the station, adjacent to the CCR landfill. The LRCP is surrounded by steep hillsides to the east and west, the CCR landfill to the north, and a constructed dam to the south. The current water surface elevation is approximately 486 feet. The crest of the dam is at approximate elevation 505 feet. Currently, the facility functions as the stormwater and leachate collection pond for the CCR landfill, and discharges to the Ohio River through an NPDES-permitted outfall.

3.0 DESCRIPTION OF THE POST-CLOSURE PLAN 257.102(b)(1)(i)

[A description of the monitoring and maintenance activities required in paragraph (b) of this section for the CCR unit, and the frequency at which these activities will be performed]

3.1 Section 257.104(b)(1)

[Maintaining the integrity and effectiveness of the final cover system including making repairs to the final cover as necessary to correct the effects of the settlement, subsidence, erosion, or other events and preventing run-on and run-off from eroding or otherwise damaging the final cover.]

Inspections are performed for the items noted below. The inspection frequencies are scheduled to properly detect any issues so that repairs can be performed before significant harm occurs.

- **Embankment**: The waste embankment will be inspected for slides, settlement, subsidence, displacement, and cover condition (see below).
- **Final Cover Surface**: The Final Cover surface will be inspected for any ponding of water or flat areas. Due to the design contours required to achieve the final cap grade, special attention will be focused to ensure that no settlement, subsidence, erosion, depressions or flat areas exist and that no water is allowed to pond above the cap system. Condition of the vegetation will be observed for maintenance needs (i.e., gaps in vegetation, presence of undesirable trees or brush).

- Stormwater Management System: The stormwater management system, including channels, culverts, slope drains, etc., will be inspected for erosion, integrity of channel lining, ponding, and accumulated sediment.

Maintenance during the post-closure care period will be performed as discussed below following the facility inspections.

- Embankment: Embankments will be inspected for slides, settlement, subsidence, displacement, and cover condition. Any areas exhibiting any such conditions will be repaired by reworking, replacing and/or compacting the material to design grade/specifications.
- Erosion Damage Repair: Any areas exhibiting erosion will be repaired by reworking, replacing and/or compacting the material to design grade/specifications, and reseeding the area. Applications of additional fertilizer, selective herbicides, rodent control measures, etc. will be implemented as necessary. The selection of fertilizers and herbicides will strive to minimize their impact on groundwater. Follow-up monitoring of the repaired area will be conducted.
- Settlement, Subsidence, Displacement: Any areas at the closed site exhibiting evidence of settlement, subsidence, or displacement will be examined to determine the cause of the movement. If backfilling or placing additional fill material is needed to maintain the integrity of the closed structure, it will be performed in accordance with the site/closure specifications, including seeding. If the condition reoccurs or persists, or if the severity of the condition initially is judged to warrant it, a detailed investigation of the cause will be performed and remedial action will be performed. Repairs will be made as necessary. Follow-up monitoring of the area will be performed.
- Closure Cap Surface: Any areas that show signs of ponding water or flat contours will be observed and addressed. Due to the design contours required to achieve the final cap grade, special attention will be focused on the cap surface to promote drainage, re-seeded to support vegetative growth, and maintained to minimize the ponding of water.
- Stormwater Drainage System: The channel linings are specified for design velocities. Maintenance of the stormwater management system will consist of removing sediment build up and/or undesirable vegetation from the stormwater management system's channels, culverts, and sediment basins as required. Eroded areas will be repaired by back-filling and reseeding in accordance with the specifications. Damage to culverts will be repaired; structure replacement will be performed if needed.

3.2 SECTION 257.104(b)(3)

[Maintaining the groundwater monitoring system and monitoring the groundwater in accordance with the requirements of §§257.90 through 257.98.]

The groundwater monitoring system will be observed for the general integrity of the wells, well casings and well protective casings. Any damaged portions of the monitoring wells and/or their protective casings will be replaced in-kind.

Monitoring the groundwater will be in accordance with the groundwater monitoring plan for this facility and in accordance with the requirements of §§257.90 through 257.98.

4.0 POST-CLOSURE CONTACT 257.104 (d)(1)(ii)

[The name, address, telephone number and email address of the person or office to contact about the facility during the post-closure care period.]

The name, address, telephone number, and email address of the person to contact about the facility during the post-closure period will be provided upon notification of closure.

5.0 POST-CLOSURE PLANNED USE 257.104 (d)(1)(iii)

[A description of the planned uses of the property during the post-closure period. Post-closure use of the property shall not disturb the integrity of the final cover, liner(s), or any other component of the containment system, or the function of the monitoring systems unless necessary to comply with the requirements in this subpart...]

The post-closure use of the property will be undisturbed vacant land space. The only activities occurring on the closed CCR unit will be related to the Post-Closure care activities. All other activities will be prohibited.



Stantec Consulting Services Inc.
11687 Lebanon Road, Cincinnati OH 45241

October 11, 2016
File: 175534018
Revision 0

Indiana-Kentucky Electric Corporation
3932 U.S. Route 23
P.O. Box 468
Piketon, Ohio 45661

**RE: Closure and Post-Closure Plans
Landfill Runoff Collection Pond
EPA Final Coal Combustion Residuals (CCR) Rule
Clifty Creek Station
Madison, Jefferson County, Indiana**

1.0 PURPOSE

This letter documents Stantec's certification of the EPA Final CCR Rule closure and post-closure plans for the Indiana-Kentucky (IKEC) Clifty Creek Station's Landfill Runoff Collection Pond.

2.0 CLOSURE AND POST-CLOSURE PLAN

The closure plans describe the steps necessary to close the CCR units at any time during the life of the unit and is subject to the requirements described in 40 CFR 257.102(b). The post-closure plans describe the monitoring and maintenance activities to be performed during the post-closure period of the unit and is subject to the requirements of 40 CFR 257.104(d).

3.0 SUMMARY OF FINDINGS

The EPA Final CCR Rule closure and post-closure plans are conceptual and subject to the completion of all necessary environmental reviews. They are therefore subject to change at any time. The attached closure and post-closure plans demonstrate compliance with the requirements set forth in 40 CFR 257.102(b) and 257.104(d).

4.0 QUALIFIED PROFESSIONAL ENGINEER CERTIFICATION

I, Stan A. Harris, being a Professional Engineer in good standing in the State of Indiana, do hereby certify, to the best of my knowledge, information, and belief:

1. that the information contained in this certification is prepared in accordance with the accepted practice of engineering;
2. that the information contained herein is accurate as of the date of my signature below;



October 11, 2016
Page 2 of 2

**RE: Closure and Post-Closure Plans
Landfill Runoff Collection Pond
EPA Final Coal Combustion Residuals (CCR) Rule
Clifty Creek Station
Madison, Jefferson County, Indiana**

3. that the closure plan for the IKEC Clifty Creek Station's Landfill Runoff Collection Pond meets the requirements described in 40 CFR 257.102(b); and
4. that the post-closure plan for the IKEC Clifty Creek Station's Landfill Runoff Collection Pond meets the requirement of 40 CFR 257.104(d).

SIGNATURE 

DATE 10/11/16

ADDRESS: Stantec Consulting Services Inc.
11687 Lebanon Road
Cincinnati, OH 45241

TELEPHONE: (513) 842-8200

ATTACHMENT: Clifty Creek Landfill Runoff Collection Pond Closure and Post-Closure Plans



APPENDIX L

Closure and Post-Closure
Cost Estimate

**Opinion of Closure Costs
Landfill Runoff Collection Pond
Clifty Creek Plant
Indiana-Kentucky Electric Corporation
Madison, Jefferson County, Indiana**

Facility Name:	Clifty Creek Landfill Runoff Collection Pond
Facility Location:	Madison, Indiana
Facility County:	Jefferson
Total Waste Fill Acreage:	57.0 Acres
Total Grading Acreage:	63.0 Acres
Closure Year:	2020-2025
Acreage for Closure	57.0 Acres

(Based on MSW Landfill Closure Plan State Form 50391, Sections III and VI.)

III. LABOR, MATERIALS, & TESTING (Provide a listing of items necessary to close the facility. For items that will vary depending upon the number of acres to be closed, the quantities should be indicated on a per-acre basis.)

A. Item	B. Quantity	C. Units (per acre)
Geosynthetic materials (geomembrane, geotextile, geocomposite drainage layer)	57.0	\$ 88,761.60
Uncompacted 30-inch soil layer	57.0	\$ 35,717.53
6-inch vegetative soil layer	57.0	\$ 12,524.64
Vegetative cover	57.0	\$ 3,288.19
Surveying	57.0	\$ 850.00
Engineering certification	57.0	\$ 3,310.34
Additional items	lump sum	#REF!
Deed notation	lump sum	\$ 10,000.00

V. COST PER ACRE FOR FINAL COVER & VEGETATION

A. What Percent of Final Cover and Topsoil is Available from Areas that are Controlled, and will be Controlled through Post-Closure, by the Permittee?

1. % of final cover:	0%
2. Describe location of sources:	Offsite borrow sources are being assessed.
3. % of topsoil:	0%
4. Describe the location of sources:	Offsite borrow sources are being assessed.

B. Cost per Acre for Acquisition and Placement of 30-inch Soil Layer

1. Acquisition			
a. Quantity of soil needed per acre (cubic yard (yd ³)/acre)	4,033		
b. Excavation unit cost (\$/yd ³) (if obtained onsite)	N/A		
c. Purchase unit cost (\$/yd ³) (if obtained offsite)	\$ 0.50		
d. Delivery unit cost (\$/yd ³) (if obtained offsite)	\$ 5.50		
e. Acquisition cost (\$/acre)	\$ 24,198.00		[1a * (1c+1d)]
2. Placement and Compaction			
a. Placement/spreading unit cost	\$ 2.41		
b. Compaction unit cost (\$/yd ³)	\$ -		
c. Placement and compaction cost (\$/acre)	\$ 9,719.53		[1a * (2a+2b)]
3. Testing			
a. Soil classification (if soil source is of variable quality) (\$/acre)	\$ 500.00		
b. Survey control for cover thickness and proper slopes (\$/acre)	\$ 1,300.00		
c. Density testing (if planned) (\$/acre)	N/A		
d. Testing cost (\$/acre)	\$ 1,800.00		
4. TOTAL COST, SOIL COVER (\$/acre)	\$ 35,717.53		[1e + 2c + 3d]

C. Cost per Acre for Acquisition and Placement of Geosynthetic Materials			
1. Acquisition			
a. Quantity of material needed per acre (square yards, yd ²)		4,840	
b. Purchase and install geomembrane (\$/yd ²)	\$	5.67	
c. Purchase and install nonwoven geotextile (\$/yd ²)	\$	2.19	
d. Purchase and install composite drainage layer (\$/yd ²)	\$	7.38	
e. Delivery unit cost (\$/yd ²) (if applicable)	\$	-	
f. Acquisition cost (\$/acre)	\$	73,761.60	[1a * (1b+1c+1d)]
2. Placement			
Placement cost (\$/acre) (if applicable and not included in purchasing unit cost)	\$	-	
3. Testing and QA/QC			
a. Fingerprinting, destructive (shear and peel tests) & nondestructive seam test (\$/acre)	\$	10,000.00	
b. Other testing (\$/acre)	\$	5,000.00	
c. Testing cost (\$/acre) (if applicable)	\$	15,000.00	[3a + 3b]
4. TOTAL COST, GEOSYNTHETIC LAYERS (\$/acre)	\$	88,761.60	[1f + 2 +3c]
D. Cost per Acre for Acquisition & Placement of Topsoil			
1. Acquisition			
a. Quantity of topsoil needed per acre (yd ³ /acre)		807	
b. Excavation unit cost (\$/yd ³) (if obtained onsite)		N/A	
c. Purchase unit cost (\$/yd ³) (if obtained offsite)	\$	7.61	
d. Delivery unit cost (\$/yd ³) (if obtained offsite)	\$	5.50	
e. Acquisition cost (\$/yd ³)	\$	10,579.77	[1a * (1c+1d)]
2. Placement			
a. Spreading unit cost (\$/yd ³)	\$	2.41	
b. Placement cost (\$/acre)	\$	1,944.87	[1a * 2a]
3. Topsoil Cost (\$/acre)	\$	12,524.64	[1e + 2b]
F. Cost per Acre to Establish Vegetation			
1. Vegetation			
a. Seeding unit cost (\$/acre)			
b. Fertilization unit cost (\$/acre)			
c. Mulching unit cost (\$/acre)			
d. Vegetation Establishment Cost (\$/acre)	\$	3,288.19	[1a + 1b + 1c]
G. Cost per Acre to Certify Closure			
1. Registered Professional Engineer			
a. Initial review of closure plan (hours)		80	
b. Total number of inspections		30	
c. Inspection time required (hours/visit)		24	
d. Total inspection time (hours)		720	[1b * 1c]
e. Prepare final documentation (hours)		240	
f. Total engineer time (hours)		1,040	[1a + 1d + 1e]
g. Engineer unit labor cost (\$/hour)	\$	120.00	
h. Professional engineer cost (\$)	\$	124,800.00	[1f * 1g]
i. Area of site permitted for filling (acres)		37.7	
j. Closure Certification Cost (\$/acre)	\$	3,310.34	[1h/1i]

H. Other Costs per Acre for Final Cover and Vegetation			
1. Other Costs (\$/acre)		850	soil thickness survey
I. Total of Items B through F (must not be less than \$5,000/acre)			
	LRCP Closure	\$ 140,291.96	per acre
VI. OTHER CLOSURE COSTS (total facility basis, not per acre)			
A. Notation of Property Deed		\$ 10,000.00	
B. Other Costs - such as drainage feature, installation of gas vents, etc.			
	Activity	Cost	
	Ditch Lining (Unimat + Concrete)	\$ 2,000,000.00	
C. Total		\$ 2,010,000.00	[A + B]
VII. CLOSURE COST ESTIMATE		\$ 10,006,641.72	[(Acreage * VI) + VI.C]
10% Contingency (per IDEM)		\$ 1,000,664.17	
Total		\$ 11,007,305.89	
VIII. ADDITIONAL INFORMATION REQUIRED FOR FACILITIES PROVIDING FINANCIAL ASSURANCE ON AN INCREMENTAL BASIS			
A. Will Closure Financial Assurance be Provided on an Incremental Basis?		No	

**Opinion of Post-Closure Costs
Landfill Runoff Collection Pond
Clifty Creek Plant
Indiana-Kentucky Electric Corporation
Madison, Jefferson County, Indiana**

Facility Name:	Clifty Creek Landfill Runoff Collection Pond
Facility Location:	Madison, Indiana
Facility County:	Jefferson
Total Waste Fill Acreage:	57.0 Acres
Total Grading Acreage:	63.0 Acres
Closure Year:	2020-2025
Acreage for Closure	57.0 Acres

(Based on MSW Landfill Closure Plan State Form 50391, Section VI.)

A. Cost for Semi-Annual Inspections and Reports			
1. Inspection			
a. Number of inspections during post-closure period (semi-annual inspections for 30 years)		60	
b. Inspector time required (hours/insp)		30	
c. Inspector time labor cost (\$/hour)	\$	90.00	
d. Inspection cost (\$)	\$	162,000.00	[1a * 1b * 1c]
2. Report Preparation			
a. Number of reports during post-closure period		60	
b. Cost per report (\$)	\$	5,000.00	
c. Report cost	\$	300,000.00	[2a * 2b]
3. TOTAL COST, INSPECTIONS AND REPORTS (\$)	\$	462,000.00	[1d + 2c]
B. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control			
1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 10% of the cost per are calculated for final cover and vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).			
a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII.I of the closure plan)	\$	3,571.75	
b. Total permitted fill acreage		57	
c. Total Cost, Maintenance of Final Cover and Vegetation Cover	\$	203,589.92	[1a * 1b]
2. Vegetation Control Costs			
a. Mowing frequency (visits/30 years)		60	
b. Area to be mowed (acres/visit)		63	
c. Mowing unit cost (\$/acre)	\$	296.63	
d. Total mowing cost (\$)	\$	1,121,261.40	[2a * 2b * 2c]
e. Other (\$) - specify below (weed control for well access, etc.)	\$	-	
f. Vegetation Control Costs	\$	1,121,261.40	[2d + 2e]
3. TOTAL COST FOR MAINTENANCE OF FINAL COVER AND VEGETATION	\$	1,324,851.32	[1c + 2f]
C. Cost for Leachate Treatment and Disposal		N/A	
D. Cost for Leachate Collection System Monitoring and Maintenance		N/A	

E. Cost for Groundwater Water Monitoring and Well Maintenance			
1. Monitoring Well Maintenance Labor Cost			
a. Maintenance frequency (visits/30 years)		60	
b. Number of monitoring wells needing maintenance per visit		2	(estimated)
c. Maintenance time required (hours/well)		4	
d. Unit labor cost (\$/hour)	\$	81.00	
e. Monitoring well maintenance labor cost (\$)	\$	38,880.00	[1a * 1b * 1c * 1d]
2. Monitoring Well Parts and Sampling Equipment Replacement Cost			
a. Number of wells needing replacement during post-closure period		0	
b. Existing monitoring well abandonment unit cost (\$)	\$	-	
c. New monitoring well construction unit cost (\$)	\$	-	(drilling charged by foot)
d. Monitoring well replacement cost (\$)	\$	-	
e. Number of pumps/bailers needing replacement during post-closure period		5	
f. Pump/bailer unit cost (\$/pump)	\$	2,000.00	
g. Pump/bailer replacement cost (\$)	\$	10,000.00	[2e * 2f]
h. Monitoring Maintenance and Pump/bailer Replacement Cost (\$)	\$	48,880.00	[1e + 2d + 2g]
3. Cost for Groundwater Monitoring			
a. Number of required monitoring wells		10	
b. Monitoring frequency (semi-annual sampling for 30 years)		60	
c. Sampling cost (\$/well)	\$	1,100.00	
d. Laboratory testing cost (\$/well)	\$	400.00	
e. Statistical Analyses and Report (\$/well)	\$	300.00	
d. Groundwater Monitoring Cost (\$)	\$	1,080,000.00	[3a * 3b * (3c+3d+3e)]
4. TOTAL, GROUNDWATER MONITORING AND WELL MAINTENANCE COST	\$	1,128,880.00	[2h + 3d]
F. Cost for Methane Monitoring and Maintenance			
		N/A	
G. Cost for Drainage and Erosion Control Maintenance			
1. Drainage and erosion control maintenance frequency (visits/30 years)		60	
2. Cost for materials to repair per visit	\$	500.00	
3. Total material cost (\$)	\$	30,000.00	[1 * 2]
4. Maintenance time required per visit (hours)		10	
5. Unit labor cost	\$	140.00	
6. Total labor costs (\$)	\$	84,000.00	[1 * 4 * 5]
7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST	\$	114,000.00	[3 + 6]
H. Cost for Access Control and Benchmark Maintenance			
4. Fencing material cost (\$)	\$	-	Facility is fenced.
7. Total labor costs (\$)	\$	-	
8. Benchmark maintenance cost (if applicable (\$))	\$	5,000.00	
9. Other (\$)	\$	-	
10. TOTAL, ACCESS CONTROL/BENCHMARK MAINTENANCE COST	\$	5,000.00	[4 + 7 + 8 + 9]
I. Optional - Maintenance of dike(s) required for facilities constructed in floodplain/floodway			
J. Other costs - Costs not included in the above items should be listed here. They may include such items as access road maintenance, lift station power costs, etc. Please enter "N/A" if you do not have additional costs to place here.			
		N/A	
K. TOTAL POST-CLOSURE COST	\$	3,034,731.32	[A3 + B3 + C4 + D3 + E4 + F3 + G7 + H10 + I7 + J]
10% Contingency (per IDEM)	\$	303,473.13	
Total	\$	3,338,204.45	